Appendix A-E

Draft Existing Conditions Report Appendices

POUND RIDGE WASTEWATER TASK FORCE

Appendix A: Historical Wastewater Reports

Appendix for Wastewater Reports

1992 Proposed Scotts Corners Wastewater District, Folchetti & Associates Page 2

- 1. Comprehensive study and proposal for a Wastewater Treatment Plant (WWTP) for pre-treating wastewater and pumping to a Subsurface Disposal System (SSDS).
- 2. Two parcels in Scotts Corners area are under consideration for wastewater treatment plant location, Berman and Quade; neither is acceptable.
- 3. Town parcel Lot 86 Block 9820 Tax Map 19 in Town Park is a potential solution, but requires either 9,000 feet or 6,300 feet of pumping.

Sept., 1998, Malcolm Pirnie, Letter to Clay Fowler PR Planning Board, Pound Ridge Treated Wastewater Effluent Well Injection <u>Page 46</u>

1. Technically and regulatorially, a bad idea – see report.

Sept. 3, 1999 Malcolm Pirnie, Wastewater Disposal Evaluation, Letter to Clay Fowler PR Planning Board Page 52

- 1. MP was retained to evaluate and provide alternatives separate sewage treatment systems
 - i. Wastewater Treatment Plant, On site treatment, Pump to "Ball Fields".
 - ii. Upgrading selected ssts Needs more study
- 2. Includes Table of existing conditions

June 29, 2000, Malcolm Pirnie to Malcom Pirnie, Scotts Cornet Test Pits and Percolation Test Page 59

1. Summary of the test, but no results.

July 11, 2000, Malcolm Pirnie to Clay Fowler, Scotts Corners Wastewater Treatment System Page 68

- 1. This solution could be used to treat wastewater from only a particular area.
- 2. Results of test pits and percolation tests behind lots 60, 61 and 62, Block 9320

Sept.26, 2000 Malcolm Pirnie, Letter to Clay Fowler PR Planning Board, Scotts Corner Septic Evaluation – Scope of Work and Cost Estimate Page 75

- 1. Propose some kind of hybrid system, a combination of new leach fields (behind lots 59 through 63), maximizing the efficiency of the existing systems, and tying them all together.
- 2. Inventory water supply wells, Calculate water uage and wastewater discharge volume, Figure out individual septic system details; tank and field locations and size, Calculate hydraulic loading
- 3. Support creation of a community water supply

April 2002, Scotts Corners Potable Water and Wastewater Conceptual Investigation Letter from Folchetti & Associates to Joy Simpkins, Waste WaterWastewater <u>Page 80</u>

- 1. System investigation for Scotts Corners northwest parking lot.
- 2. Quad Parcel (9320-56) found unsuitable for SSDS.
- 3. Town Park site may be suitable for SSDS, would need variances.
- 4. Golf Course option for disposal through irrigation may be feasible.
- 5. Potable Water assessment, neither of two Stamford systems suitable.
- 6. Water service via Golf Course may be viable alternative. BHC was supportive (out dated).

1992 Feasibility Study Proposed Scotts Corners Wastewater District Prepared by Folchetti & Associates

Comprehensive study and proposal for a Wastewater Treatment Plant (WWTP) for pre-treating wastewater and pumping to a Subsurface Disposal System (SSDS). 5 Sites identified and tested with recommendations.

• Estimated Cost for plant and collection system is \$1,570,000.

• Wastewater flow of 24,700 gpd.

• Sq. Ft. has increased annual average of approx. 3.4%/year since 1974

(inflated because of size of Trinity Corners Shopping Center).

• Adjusted growth rate without TCSC is 0.75%/year.

• Estimated 2012 design flow is 28,000 gpd.

• Westchester County Health Department (WCHD) and Stamford Water

Company is willing to relax separation distances in case of subsurface discharge of treated effluent.

Technical notes extracted by TD on Folcetti study 1992; 3/6/2016

1.1.2 DEC regulations prohibit point discharges into AA streams, aprt of the Stamford Water Company

1.1.4 The estimated existing combined commercial and residential waste water flows in Scotts Corners is 24,700 gpd

1.1.5 The growth rate of Scotts Corners is 3.4% per year since 1974, but EXCLUDING Trinity Corners shopping Center is 0.75% per year.

1.1.8 Based upon the 0.75% growth rate the estimated design flow for 2012 is 28,000 gpd

1.1.9 The Quade and Berman parcels are unsuitable for discharge sites.

1.1.10 WCHD and Stamford Water might work with Pound Ridge to reduce separation distances in the case of subsurface discharge of treated effluent.

1.1.11 WCDH will not consider relaxation of standard application rates even though effluent is treated.1.2 Conclusions:

1.2.1 A wastewater treatment system with subsurface disposal of treated effluent will alleviate the existing sewage problems in the Scotts corner area.

1.2.2 Based on the nature of the soils, pretreatment with a conventional system, is recommended prior to subsurface discharge.

1.2.3 Two parcels in Scotts Corners area are under consideration for wastewater treatment plant location.

1.2.4 Base upon a reconnaissance and a soil test program, the Town Parcel Lot 66, Block 9820, Map 19 may be suitable for subsurface disposal of plant treated effluent.

1.2.5 The estimated cost in 1992 dollars is \$1,570,000 for a collection system and SSDS.

3.1 The Scotts Corners Commercial District is about 41.1 acres. The primary zone is 24.43 acres. Fig 3.1

3.1.1 Table 3-1; Building square footage

Commercial –	159,680
Residential –	13,222
Total	172,902

- 3.1.2 Table 3-2 Remaining developable square footage Commercial 62,193 Residential 67,699 Total 129,892
- 3.2.1 Estimated Existing Flows Based upon the DEC "Design Standards for wastewater Treatment Works 1988" Commercial flows 20,393 Residential flows 4,520 Total 24,643 (noted above)
- 3.2.2.1 Existing and Saturation flow projection results in unreasonable flows of 64,062.
- 3.2.2.2 Revised projection using dry and wet commercial results in a 2012 flow of 27,900 or 28,000. See text for projection methodology.

4.2 Design Loads

Table 4-1 Design loads for 28,000 gpd or 0.028 mgd based upon ten states standards

Suspended solids	240 mg/l	56.05 #/d
BOD5	220 mg/l	51.4 #/d
NH3-N	25 mg/l	5.8 #/d
Phosphorus	10 mg/l	2.3 #/d

4.3 Treatment Required

- 1. 3rd paragraph page 15 "the use of innovative/alternative using wetlands, land treatment, do not seem acceptable to DEC and WCHD".
- 2. Could pump the effluent to a different watershed
- 3. Subsurface discharge system (SSDS)

4.4 Collection System

8 inch gravity sewer pipe, and 4 inch force mains, and two pump stations, See fig 4-1 Ten States requires 4.0 factor so collection system would have to handle 120,000 gpd.

4.5 Treatment Alternatives

See report for treatment plant suggestions - Sequencing Batch Reactor (SBR) See Fig 4.2 for process flow diagram

 5.0 Three SSDS Treatment Site Alternatives Berman Parcel on Trinity Pass Rd.
 Quade Parcel behind the PR Fire Department Town Parcel Lot 86, Block 9820, Tax Map 19

5.0 Site Alternatives

Page 20 - Treatment requirements result in the need for 2 acres plus.

- 5.1 Berman parcel see text for discussion
- 5.2 Quade parcel see text for discussion Neither are acceptable
- 5.3 Town parcel

Did perc test and given this result and the area of land available it would work Would require 9,000 of force main.

Or with an easement this could be reduced to 6,300 feet

- 5.4 Conclusion is that the treatment plant should be in Scotts Corners and the treated effluent pumped to the Town Park.
- 6.1 Cost for it all is \$1,570,000 plus 20 year loan at 6%. O&M at \$38,000 per year.
- 6.5 Cost Allocation Alternatives
- 6.5.1 Scotts Corner alone a, assessed property value; b, metered use; c, prorating
- 6.5.2 Town wide allocation
- 6.5.2.1 Single tier Capital and operating costs borne town wide based upon flat fee or property value.
- 6.5.2.1 Double tier Captital costs town wide O&M covered by users
- 6.6 Alternative Financing SRF, FMHA, HUD,

WASTEWATER TREATMENT FEASIBILITY STUDY

SCOTTS CORNERS POUND RIDGE, NY

JUNE 1992

J. ROBERT FOLCHETTI & ASSOCIATES

ENVIRONMENTAL ENGINEERS P.O. BOX 374 BREWSTER, NY 10509 Som ERS

FEASIBILITY STUDY

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PROPOSED SCOTTS CORNERS WASTEWATER DISTRICT

POUND RIDGE, NEW YORK

May 1992

J. ROBERT FOLCHETTI & ASSOCIATES P. O. Box 374 Brewster, New York 10509 (914) 279-3346

Table of Contents

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Page

1.0								
	Reco	nmendations	•	•			•	
	1.1	Findings						
	1.2	Conclusions						2
	1.3	Recommendations						
2.0	Intro	oduction	•	•		•		3
3.0	Exis	ting Conditions and Projections			•			5
	3.1	Land Use						5
		3.1.1 Existing Use	•				•	6
		3.1.2 Future Use	•				•	7
		3.1.3 Demographic Data		•				8
	3.2	Flows						11
		3.2.1 Estimate of Existing Flows						11
		3.2.2 Projection of Future Flows						12
4.0	Treat	ment						
	4.1	Design Flows						14
	4.2	Design Loads						14
	4.3	Treatment Required						15
	4.4							
	4.5	Treatment Alternatives						18
5.0	Site	Alternatives						19
	5.1	Berman Parcel						20
	5.2	Quade Parcel						
	5.3							
	5.4	Treatment Plant Location						23
6.0	Estir	nated Project Cost						
	6.1	Estimated Capital Costs						
	6.2	Estimated Annualized Capital Cost.						
	6.3	Estimated Annual Operation and						
		Maintenance Costs						26
	6.4							
	6.5	Cost Allocation Alternatives						
		Alternative Financing Sources						
	6.7	New York State Revolving Fund Low						
		Interest Loan Milestones						29
7.0	Imple	ementation Task Schedule						
8.0		and Control Requirements						

TABLES

Table	3-1	Existing Land Use in Scotts Corners
Table	3-2	Remaining Developable Square Footage in Scotts
		Corners
Table	3-3	Change in Square Footage in Scotts Corners 1974-1990
Table	3-4	Percent Change 1990 to Saturation
Table	3-5	Projected 2012 Square Footage Increase at 0.75%/Year
		Growth
Table	3-6	Estimated Existing Flows in Scotts Corners
Table	3-7	Estimated Flow at Saturation
Table	3-8	Estimated 2012 Flow
Table	4-1	Design Loads
Table	5-1	Percolation Rates on Town Parcel
Table	5-2	Comparison of Pumping Raw Waste vs. Treated Effluent
Table	6-1	Estimated 1992 Construction Cost
Table	6-2	Estimated Annualized Capital Cost & Parameters
Table	6-3	Estimated Annual O&M Costs
Table	6-4	Estimated Total Annual Costs Under NYSDEC Loan
		Program

FIGURES

Figure 3-1 Figure 3-2	Proposed Scotts Corners Wastewater District Scotts Corners Wastewater District Commission
2	Questionnaire Responses
Figure 4-1	Conceptual Collection System Sketch
Figure 4-2	Process Flow Diagram
Figure 5-1	Deep Hole Test Locations Berman & Quade Parcels
Figure 5-2	Town Parcel Soil Test Location Sketch
Figure 5-3	
Figure 7-1	Implementation Task Schedule

5)

2)

APPENDICES

Appendix A	Trinity Corners Shopping Center SSDS Failure History Since 1980
Appendix B	Deep Hole Test Data
Appendix C	Extract From NYS Audit and Control Board Requirements
Appendix D	Extract From NYS Revolving Fund Intended Use Plan
	(Draft) 1992
Appendix E	Siano Violations
Appendix F	Westchester County Health Department Comments on
	Chromaglass System
Appendix G	State Revolving Fund Direct Loan Application Form
Appendix H	Farmers Home Administration Loan Application Form
Appendix I	Housing and Urban Development Loan Application Form

1.0 Summary of Findings, Conclusions and Recommendations

- 1.1 Findings
- 1.1.1 That the Scotts Corners Commercial District of Pound Ridge is primarily composed of low water use, retail establishments that are individually served by SSDS's and private wells.
- 1.1.2 That Scotts Corners lies in the watershed of the Stamford Water Company. All streams in this watershed are classified 'AA' Special. NYSDEC regulations prohibit point discharges into waters so classified.
- 1.1.3 That the Trinity Corners Shopping Center has a long history of SSDS failures. Samples taken from the shopping center storm drain in the third quarter, calendar year 1991, show elevated levels of fecal coliform and fecal streptococci. These failures have the potential to impact directly on the quality of water in the Stamford Water Company watershed.
- 1.1.4 That the estimated existing combined commercial and residential wastewater flow in Scotts Corners is 24,700 gpd.
- 1.1.5 That square footage in Scotts Corners has increased an annual average rate of approximately 3.4% per year since 1974.
- 1.1.6 That this growth rate is inflated due to the size of the Trinity Corners Shopping Center.
- 1.1.7 That the adjusted growth rate without Trinity Corners Shopping Center is 0.75% per year.

- 1 - 9

- 1.1.8 That based on this rate of growth, the estimated 2012 design year flow is 28,000 gpd.
- 1.1.9 That the Quade and Berman parcels are unsuitable for use as subsurface discharge sites.
- 1.1.10 That the Westchester County Health Department (WCHD) and Stamford Water Company both have stated that they are willing to work with the Town of Pound Ridge to resolve existing problems in terms of some relaxation of separation distances in the case of subsurface discharge of treated effluent.
- 1.1.11 WCHD will not consider relaxation of standard application rates, even though treated effluent would be applied.
- 1.1.12 That Scotts Corners will qualify for SRF status once the wastewater district formation process is commenced.

1.2 Conclusions

- 1.2.1 That a wastewater treatment system with subsurface disposal of treated effluent will alleviate the existing sewage problems in the Scotts Corners area.
- 1.2.2 That, based on the nature of the soils, pretreatment with a conventional system is recommended prior to subsurface discharge.
- 1.2.3 That two parcels in the Scotts Corners area are under consideration for wastewater treatment plant location.
- 1.2.4 That based on a reconnaissance and soil test program the Town parcel (Lot 86, Block 9820, Map 19) may be suitable for subsurface disposal of plant treated effluent.

1.2.5 That the estimated capital cost, in 1992 dollars, for the treatment plant, collection system and SSDS is approximately \$1,570,000.

1.3 Recommendations

- 1.3.1 That the conceptual cost estimate be evaluated by the Town.
- 1.3.2 That, if this conceptual estimate is acceptable, the Town of Pound Ridge proceed with the major tasks shown on Figure 7-1.
- 1.3.3 That the Town of Pound Ridge continue to explore innovative systems for subsurface disposal with the agencies.

2.0 Introduction

The Scotts Corners area of the Town of Pound Ridge is not presently served by a municipal sewer system. The structures in the area are served by a Sub-Surface Disposal System (SSDS) handling domestic and commercial wastewater flows. The geologic and hydrogeologic conditions of the area are not well suited for this type of treatment. These conditions have resulted in frequent failures. While all of these failures have not been documented, the Westchester County Health Department and Stamford Water Company as well as many residents and business owners are well aware of the problem.

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The notable exception to this lack of documentation is the Trinity Corners Shopping Center. Since its construction in the early 1970's, the SSDS for this facility has been subjected to a series of failures resulting in discharges of untreated sewage to the ground surface and drainage of the local watershed. The fact that this watershed serves the Stamford Water Company and the City of Stamford, Connecticut is cause for concern. Recent reclassification of surface waters in this area to 'AA Special' further complicates the situation. NYSDEC "Water Quality Regulations for Surface Waters and Ground Waters" (6 NYCRR Part 701.3.C.) prohibits discharge into waters SO classified.

As a result of these problems, the Pound Ridge Sewage Treatment Committee, through the Town Board of the Town of Pound Ridge, retained JRFA to study the feasibility of forming a Municipal Wastewater District and constructing a collection system and sewage treatment plant to serve the Scotts Corners area.

The following sections describe the present and projected future conditions, treatment options, estimated costs and other concerns for the Scotts Corners Wastewater District.

3.0 Existing Conditions and Projections

3.1 Land Use

Scotts Corners is situated in the southeast corner of the Town of Pound Ridge. It is bordered on the northeast by the Town of Lewisboro, on the west by the Town of Bedford and on the south by the City of Stamford, Connecticut.

The Scotts Corners Commercial District area encompasses approximately 41.1 acres. This is divided into primary and secondary zones (see Figure 3.1). The primary zone encompasses 24.43 acres and the secondary zone encompasses 16.68 acres.

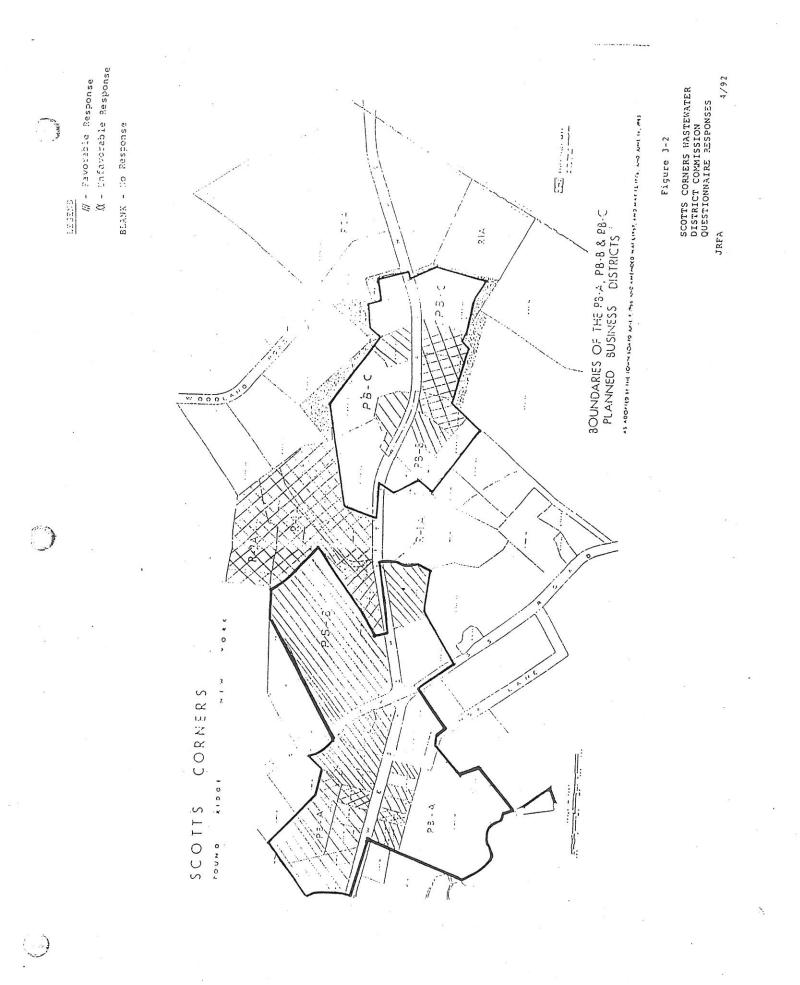
The zones were termed primary and secondary based on density and history. The primary zone has a history of SSDS failures, particularly at the Trinity Corners Shopping Center. It also has a higher population and use density. The secondary zone is primarily a low density, residential area and has virtually no documented history of SSDS failure. The Pound Ridge Sewage Treatment Committee issued a questionnaire to the owners/residents in both districts. Those responses are indicated in Figure 3-2. The secondary zone was eliminated from consideration for three reasons:

1. No substantial history of SSDS failures.

2. Low density population/water use.

Predominantly unfavorable response to the District
 Formation questionnaire. Over 50% of this zone

- 13 -



either responded in the negative or failed to respond at all.

Consequently the primary zone constitutes the proposed wastewater district. Should the future bring about a change in conditions, the secondary zone may petition to enter the proposed District.

3.1.1 Existing Use

3

F. P. Clark Associates 1990 Planning Study provided the basis for all land use and demographic data. A field survey was conducted to verify and update existing uses.

Land use within Scotts Corners is based on the zoning regulations of the Town of Pound Ridge. Each lot in planned business district A has a building envelope. Each lot in planned business district B has a maximum 2,500 square foot allowable building footprint size.

Table 3-1 illustrates the existing uses of the primary zone in Scotts Corners.

Table 3-1

Existing Land Use in Scotts Corners

Building Square Footage		Primary Zone
Commercial		159,680
Residential (Apartments)	54 54	13,222
TOTAL		172,902

-15⁶ -

There is presently a total of 172,902 square feet in existence in the primary district.

Presently, wastewater disposal in Scotts Corners is through use of individual SSDS's. There is some evidence on record documenting to a pattern of system failure in the area. Personal review of available records at the WCHD revealed approximately a dozen complaints on private SSDS's. In addition the Trinity Corners Shopping Center has experienced multiple failures and is presently pending court action with the WCHD. The Stamford Water Company has determined this system to be a detriment to their raw water quality. Pertinent information has been requested under the Freedom of Information Act from both the WCHD and Stamford Water Company and is included in Appendix A.

3.1.2 Future Use

The Pound Ridge Zoning Ordinance limits the maximum allowable floor space per lot. In PB-A each lot has a legislated building envelope; in PB-B each lot has a maximum 2,500 square foot building footprint size and a 'floor area ratio' that defines the amount of second story floor space allowed. Additionally, a percentage of this second floor space must be used for residential purposes. The zoning ordinance and the Clark Study define these numbers explicitly.

- 76 -

Table 3-2 illustrates the maximum remaining square footage developable in the proposed District.

Table 3-2

Remaining Developable Square Footage in Scotts Corners

	Primary Zone
Commercial	62,193
Residential	67,699
TOTAL	129,892

These values are influenced by several factors, as follows.

- First, the available commercial square footage in the primary zone does not include lot 1.9. This lot contains the shopping center and exceeds the maximum allowable F.A.R. per the zoning ordinance.
- Second, there was no allowance made in these figures for future residential square footage on this lot. Though the code permits second story residential use above the Shopping Center, the nature of the structure and its current use suggest that it is not appropriate for residential use. Therefore, residential potential for lot 1.9 was not included in the residential square footage depicted in Table 3-2.

3.1.3 Demographic Data

The Clark Study established several patterns between 1974 and 1990 regarding growth in Scotts Corners. These patterns reflect changes in existing square footage over

- 8 -

that period of time. Table 3-3 summarizes the Clark Study Findings.

Table 3-3

Ch	ange in Squar	e Footage in Sco	otts Corners	1974-1990
	Comm	ercial Use	Resi	dential Use
District	Total % Change	% Change/Year	Total % Change	% Change/Year
PB-A	+ 25.8%	+ 1.6%	- 8%	5%
PB-B	+245%	+15.3%	-49%	-3%
Average		+ 8.45%		-1.75%

These changes reflect some significant issues. First, the decrease in residential use in PB-A is a result of changeover to small commercial establishments in older buildings. The loss of residential square footage in PB-B is assumed to reflect demolition in conjunction with construction of Trinity Corners Shopping Center.

Second, the increase in commercial square footage is skewed due to the shopping center. This is assumed to be a one time, non repetitive event. The increase in commercial square footage in PB-A is assumed to be representative of reasonable commercial growth in the area.

Finally, U.S. Census Data, as kept by the Westchester County Planning Department, shows 5.7% population increase from 1970-1980 and 13.5% population increase between 1980-1990. Hence, growth from 1970-1990 is less than 1%

-18 -

per year. Since the annual population growth is less than the representative commercial growth (1.6%), it is assumed that commercial growth will decrease, until such time as future population growth causes an increase in commercial demand. Therefore it is reasonable to expect an annual commercial growth rate of 0.75% for a neighborhood shopping area under these conditions.

In addition to the Clark data, JRFA estimated the saturation density of the area. This saturation estimate is based on each zone achieving the maximum remaining square footage allowed by the current zoning ordinance. These figures were determined from the maximum square footage per lot allowed by the zoning ordinance. Existing square footage was subtracted from the maximum allowable. The difference, divided by the existing square footage, provides the percent change to saturation. Table 3 - 4summarizes these percent changes from 1990 to the saturation condition.

Table 3-4

Percent Change 1990 to Saturation

District		Commercial Use			Residential Use		
		Exist SF	Saturation SF	% Change		Saturation SF	% Change
PB-A	÷	81,239	143,522	+77	12,622	71,626	+470
PB-B*		78,441	58,225		600	11,604	+1834

The PB-B saturation SF is lower than the existing SF figures because lot 1.9 is over built. The saturation SF is taken from the Pound Ridge Zoning Ordinance.

With the exception of the commercial growth in PB-B, the averages shown are extraordinary for both commercial and residential change. These rates are clearly unreasonable.

Based on a 0.75% per year increase over the next 20 years, a potential growth projection may be made. Table 3-5 summarizes this potential growth.

The 2012 square footage will be used to calculate the design flows.

Table 3-5

Projected 2012 Square Footage Increase at 0.75%/Year Growth

	Commerc	ial Use	Residential Use		
District	1990 Existing	2012 Projected (Sq.Ft.)	1990 Existing (Sq.Ft.)	2012 Projected (Sg.Ft.)	
PB-A PB-B*	<u>(Sq.Ft.)</u> 81,081 76,132	93,243	12,262	14,101	

* PB-B commercial expansion predicted for lots 24 and 25 only. PB-B existing residential includes lot 24 only; the residential growth prediction is based on this figure only.

3.2 Flows

12

3.2.1 Estimate of Existing Flows

In the absence of metered water use data in Scotts Corners, the estimate of existing wastewater flows is based on the New York State Department of Environmental Conservation (DEC) Publication "Design Standards for Wastewater Treatment Works (1988)". This publication establishes average daily wastewater flows for a variety

- 201 -

of water users. The commercial use averages are based on a gallon per day (gpd) per square foot for dry users, or gpd per seat for wet users such as restaurants. Residential use averages are based on gpd per bedroom.

In general, the Scotts Corners area is occupied by dry use establishments. Per the DEC Standard, these are assigned a 0.10 gpd per square foot use factor. The most notable exception to this use is in restaurants and service stations. Ordinary restaurants are assigned a 35 gpd per seat use factor by DEC. Service stations are assigned a rate of 400 gpd per sanitary closet. Table 3-6 summarizes the estimated existing flows.

Table 3-6

Estimated Existing Flows in Scotts Corners GPD

	Commercial Flow	Residential Flow	Zone Total
Primary Zone	20,393	4,250	24,643

3.2.2 Projection of Future Flows

Two future flow projections have been made. The first is based on maximum expansion of the District, called saturation flow. The second is based on District expansion at a rate of 0.75 percent per year up to 2012, the design year of the plant.

3.2.2.1 The projection of saturation flow for Scotts Corners is based on the combination of existing flow and

- 12 -

additional flow resulting from saturation. Table 3-7 summarizes this projection.

Table 3-7

	Estimated Flow at Saturation GPD							
	Exist Comml +	Exist Resid	=	Total Exist	+	Proj Comml +	Proj Resid =	Total Proj
Pri Zone	20393	4250		24643		10919	28500	64062

Clearly projecting an increase in use and flows to this extent is unreasonable.

3.2.2.2 The design year flow projection for District expansion at a rate of 0.75 percent per year is based on the estimated increase in square footage by the year 2012 (the design year).

> The ratio of commercial wet users vs. total use, by square foot, was carried from the existing 1990 data to the projected 2012 figures. This ratio was established using the PB-A district as a standard, since the majority of the wet users in Scotts Corners are located in this District. The equation yielded approximately 6.3% wet use in the PB-A District. This percentage was applied to the <u>total</u> increase in projected square footage as a reasonable wet user increase at the design year.

> > - 122 -

Based on the Zoning Ordinance, F.A.R., and the Clark Study, the increase in residential square footage was assessed as two bedroom apartments.

Table 3-8 summarizes the projected flow increase at 0.75% growth per year.

Table 3-8

Estimated 2012 Flow GPD

Flow Class		<u> 1990 Exi</u> Sq.Ft.	isting <u>Flow</u>	<u>2012 Es</u> Sq.Ft.	timated Flow
Commercial, Dry		153,031	15,218	164,647	16,809
Commercial, Wet		6,649	5,175	7,415	6,435
Residential		12,262	4,250	14,101	4,940
TOTA	L		24,643		27,984

4.0 Treatment

4.1 Design Flows and Loads

4.1.1 Design Flows

Based on the established growth rate, existing zoning, Town Master Plan and discussions with the Pound Ridge Sewage Treatment Committee, an average day design year flow of 28,000 gpd is established. This flow assumes that the alleged infiltration problems at Trinity Corners Mall will be corrected.

4.2 Design Loads

As Scotts Corners does not have an existing treatment facility, a characteristic study to determine typical

- 14 - 23

wastewater quality parameters is not feasible. Existing literature and company experience were used to determine acceptable parameters that are in accordance with the Ten State Standards. Table 4-1 depicts these parameters.

Table 4-1

Design Loads

Parameter	Concentration	#/D @ .028 mgd
Suspended Solids	240 mg/l	56.05
BOD5	220 mg/l	51.4
NH3-N	25 mg/l	5.8
Phosphorus	10 mg/l	2.3

4.3 Treatment Required

The primary criteria for determining appropriate treatment in the Scotts Corners area is the 'AA' Special surface water classification. NYSDEC reclassified these waters at the request of the Stamford Water Company. Prohibition of point discharge into these waters applies to all surface waters in the Scotts Corners area.

Based on discussions with WCHD and NYSDEC this is interpreted as requiring subsurface discharge of treated wastewater effluent.

Consequently use of innovative/alternative systems utilizing wetlands, land treatment, etc. do not appear acceptable to the agencies. Two possible alternatives remain. One alternate is to pump effluent out of the

- 15 -

Stamford Water Company watershed and discharge into a different watershed where surface discharge is acceptable. Such an alternate would generate additional pumping and piping costs. Additionally, significant opposition can be expected from the residents and municipalities in whose watershed the proposed discharge may occur.

The second alternate is to use a Sub-Surface Discharge System (SSDS). The regulatory agencies have indicated that pretreatment is recommended prior to subsurface discharge. Subsurface discharge standards, in accordance with NYSDEC and WCHD regulations, will have to be met.

Pretreatment, in the form of a wastewater treatment plant (WWTP) is recommended for several reasons:

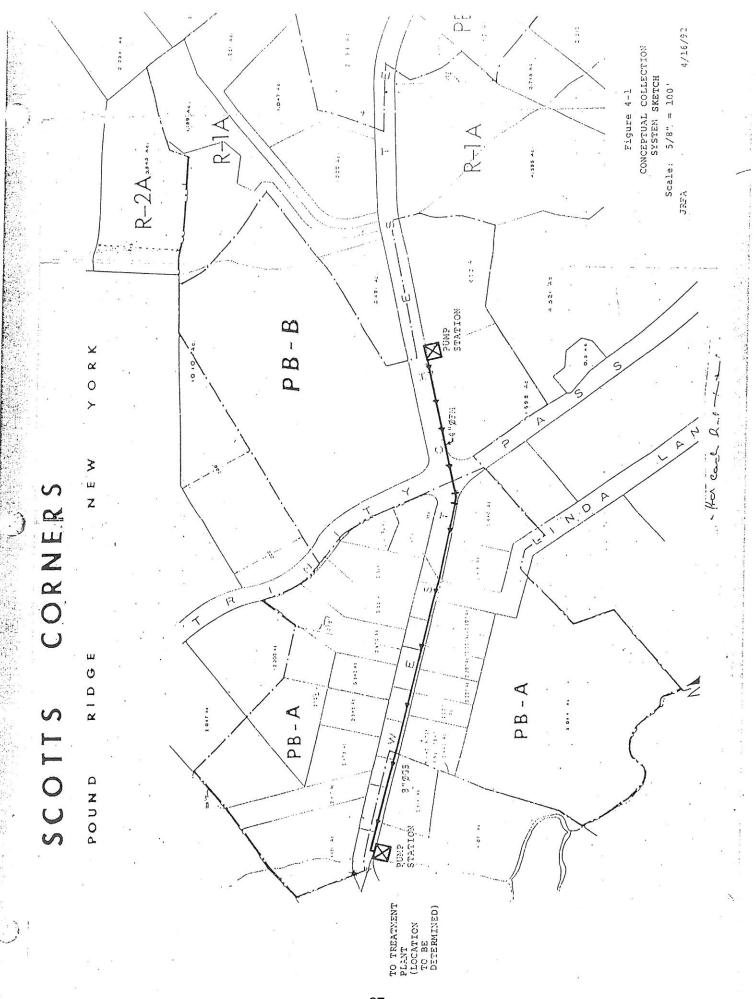
- WCHD and Stamford Water Company are willing to consider relaxation of required separation distances for pretreated waste. Given the wide distribution of rock and surface water in the area, and relatively shallow depth to ground water, this may prove to be a significant benefit in locating an acceptable subsurface disposal area.
- Pretreatment affords much more effective treatment of organic loads and solids than does subsurface discharge of septic tank effluent.
- Consequently pretreatment will markedly extend the life expectancy of an SSDS.

- 16 -25

- 4. Failure of a 30,000 gpd SSDS without pretreatment would present problems several orders of magnitude greater than those historically associated with the Trinity Corners Shopping Center.
- 5. Given the regulatory situation and geologic and hydrologic conditions in the Scotts Corners area, pretreated subsurface discharge is the most environmentally sound option.

4.4 Collection System

There are currently no base maps of the Scotts Corners area available. As a result, the collection system is currently a conceptual estimate based on field observations within the proposed District limits. Length of pipe run is based on a 1,000 scale USGS topographic map. All gravity sewer pipe is presently assumed to be 8 inch diameter minimum, and force mains 4 inch diameter. Two pump stations are assumed. The conceptual sketch is shown in Figure 4-1 utilizing a zoning map base. Based on the Ten State Standards, the Peak Hourly Flow factor is established at 4.0. This equates to a peak hourly flow rate of 0.12 mgd (120,000 gpd). The Peak Hourly Flow will be considered in the design of the collection system and plant as necessary. The concept is subject to change pending receipt of detailed topographic maps.



4.5 <u>Treatment Alternatives</u>

Several treatment processes were identified and evaluated. The Chromaglass Sequencing Batch Reactor (SBR) was considered. The SBR price is comparable to that of conventional treatment processes. WCHD is presently considering acceptability for use in Westchester County (see Appendix F). The evaluation was based on the following criteria.

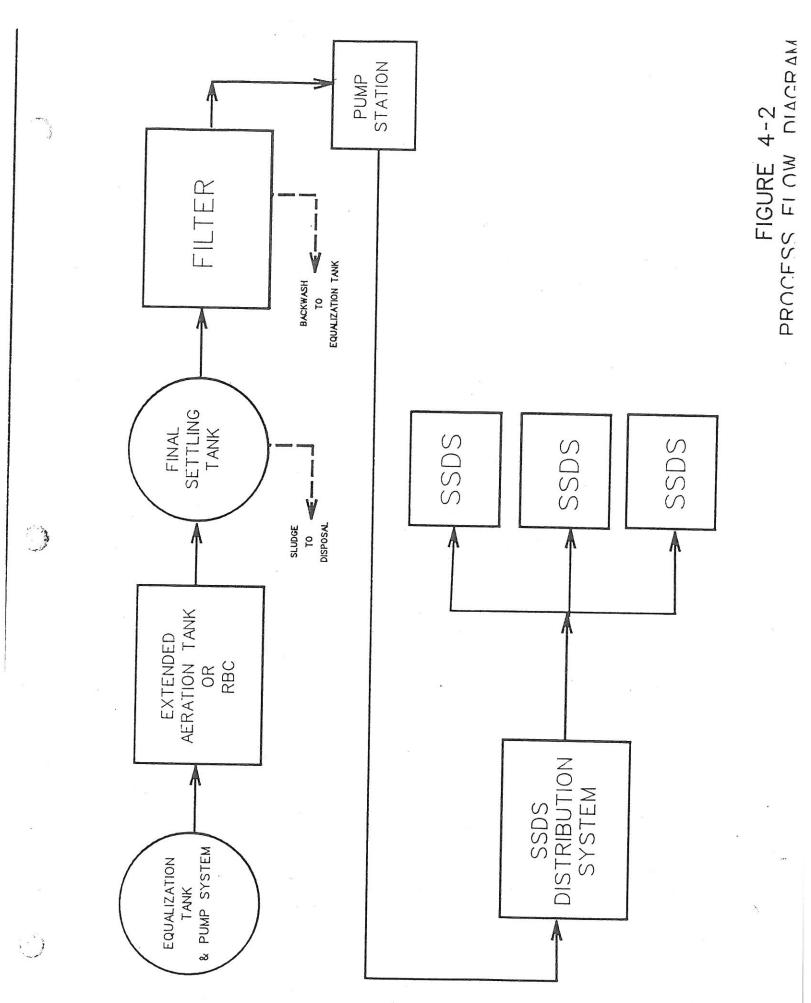
1. Process reliability.

2. Total annual costs.

- Process flexibility in meeting the increased flow volume over the design period.
- 4. Adaptability for future treatment requirements.
- 5. Site constraints including room and surroundings.
- 6. Regulatory agency considerations.

The processes considered are as follows: Alternate 1 - Extended aeration Alternate 2 - Rotating biological contactors

Both processes will be preceded by equalization and followed by filtration. This process train will produce a highly stabilized effluent which should result in maximizing the life of the subsurface disposal system. Figure 4-2, the process flow diagram, is a generic depiction of the process.



The final selection of effluent standards will be decided by NYSDEC during the State Pollution Discharge Elimination System process. This information will be the basis for study and analysis during the Facility Report Stage and will determine the final selection of a treatment process.

5.0 Site Alternatives

Three SSDS sites were proposed for testing to JRFA by the Pound Ridge Sewage Treatment Committee. One was the Berman parcel located on Trinity Pass Road. The second was the Quade parcel located behind the Pound Ridge Fire The third was the Town owned parcel, Lot 86, Block House. 9820, Tax Map 19. Among other sites considered was the Stamford Water Company (SWC) parcel bounded by Fancher Road and Westchester Avenue. They were requested to consider this as a possible site for either the treatment plant or the SSDS. After due consideration, SWC declined use of this parcel for either purpose.

In view of a design flow of 28,000 GPD, SSDS requirements are extensive. Size of the system is based on acceptable application rate of effluent to soil. The WCHD has stated that they will not relax application rate standards. Assuming a percolation rate of 30-45 minutes, the application rate will be .5 gallons/day/square foot. Assuming use of rectangular galleys, and factoring in separation between galleys and between laterals, yields a required minimum SSDS area of approximately 2 acres. Additional area would be required for buffers and pumping/distribution structures and expansion.

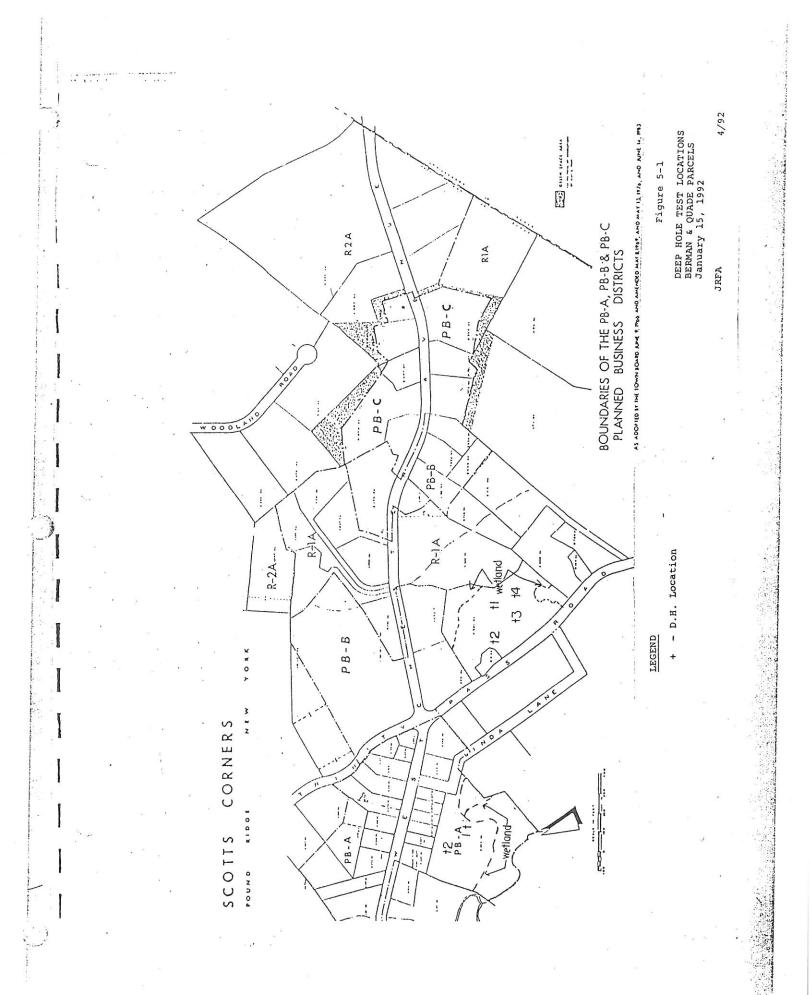
Deep hole tests were conducted on the Quade and Berman parcels on January 15, 1992. Deep hole tests were conducted on the Town parcel on May 5, 1992 and percolation tests were conducted on May 19, 1992. The Town of Pound Ridge Highway Department provided equipment and operators for this job. Location of the test pits on each lot is depicted in Figures 5-1 and 5-2. Appendix B depicts individual data for each hole.

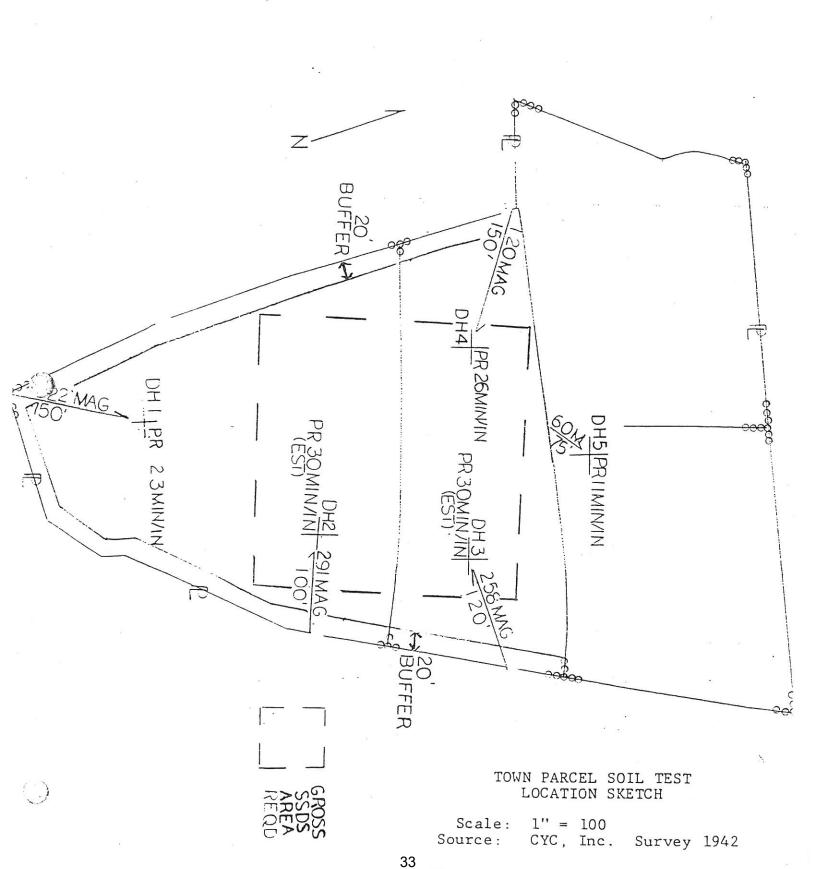
5.1 <u>Berman Parcel</u>. Four holes were dug on the Berman parcel. Two of them, numbers 2 and 3, encountered bedrock or boulders too large to move with a backhoe within 3 feet of the surface. Holes 1 and 4 both struck bedrock or rocks too large to move at seven feet. Both holes had high clay content in the A Horizon and sandy clay sand content in the B Horizon. Water flowed into Hole 1 at seven feet and into Hole 4 at five feet. General limitations for standard SSDS the Berman parcel include:

1. Tight soils.

- Bedrock or rocks too large to move with a standard backhoe.
- 3. Groundwater rose to within 4 feet of the surface.
- 4. Useable area is extremely limited due to proximity to wetlands, rock and groundwater.

- 20 -





3 JRFA

5/27/92

- Quade Parcel. Two holes were dug on the Quade parcel. Hole 1 hit bedrock or rocks too large to move at five feet and Hole 2 hit the same at six feet. Both holes had a sandy A Horizon. Hole 1 had high clay content in the B Horizon that made a good cast. Hole 2 had high sand content in the B Horizon that cast poorly. Water flowed into Hole 1 at five feet and into Hole 2 at four feet. General limitations for standard SSDS aboard the Quade parcel include:
 - Bedrock or rocks too large to move with a standard backhoe within 5 feet of the surface.
 - 2. Groundwater within 4 feet of the surface.
 - Useable area is extremely limited due to proximity to wetlands, rock and groundwater.

Both sites are unsuitable for standard SSDS. Even with pretreatment, waivers would still be necessary for rock, ground water and surface water/wetlands. Assuming approval of necessary waivers, both parcels are only large enough for the SSDS alone. There is not sufficient useable land on either parcel for the plant, SSDS and required expansion area.

5.3 <u>Town Parcel Lot 86</u>, <u>Block 9820</u>, <u>Tax Map 19</u>. Five holes were dug on the Town parcel. The most shallow hole was seven feet deep and the deepest hole was over nine feet deep. All holes had clayey sand in the A horizon. Hole 1 had clayey sand in the B horizon; holes 2, 3 and 5 had

- 34 -

5.2

sandy clay in the B horizon; hole 4 had coarse, sandy gravel in the B horizon. Holes 3, 4 and 5 all displayed a C horizon primarily composed of gravelly sand. Neither bedrock nor groundwater was encountered in any hole.

Percolation tests were conducted at all five locations in accordance with NYSDEC "Design Standards for Wastewater Treatment Works, 1988". All tests were conducted at a total depth of 4 feet. See Appendix B for water surface elevation in each hole. Perc holes 1, 4 and 5 were replenished after 1 inch of drop after each run. Holes 2 and 3 were filled and measured at the end of the test period without replenishment to test varying head conditions. Table 5-1 provides percolation data for each hole tested.

Γ	a	b	1	e	- 5	-	1

	Test #						
ж ,	1	2	. 3	4	Avg		
Hole 1	22:40	22:17	23:30	23:20	22:40		
Hole 2	30				30		
Hole 3	30				30		
Hole 4	22:49	25:00	26:30	26:50	26:40		
Hole 5	10:40	10:50	11:43		11		

Percolation Rates on Town Parcel, in min/inch

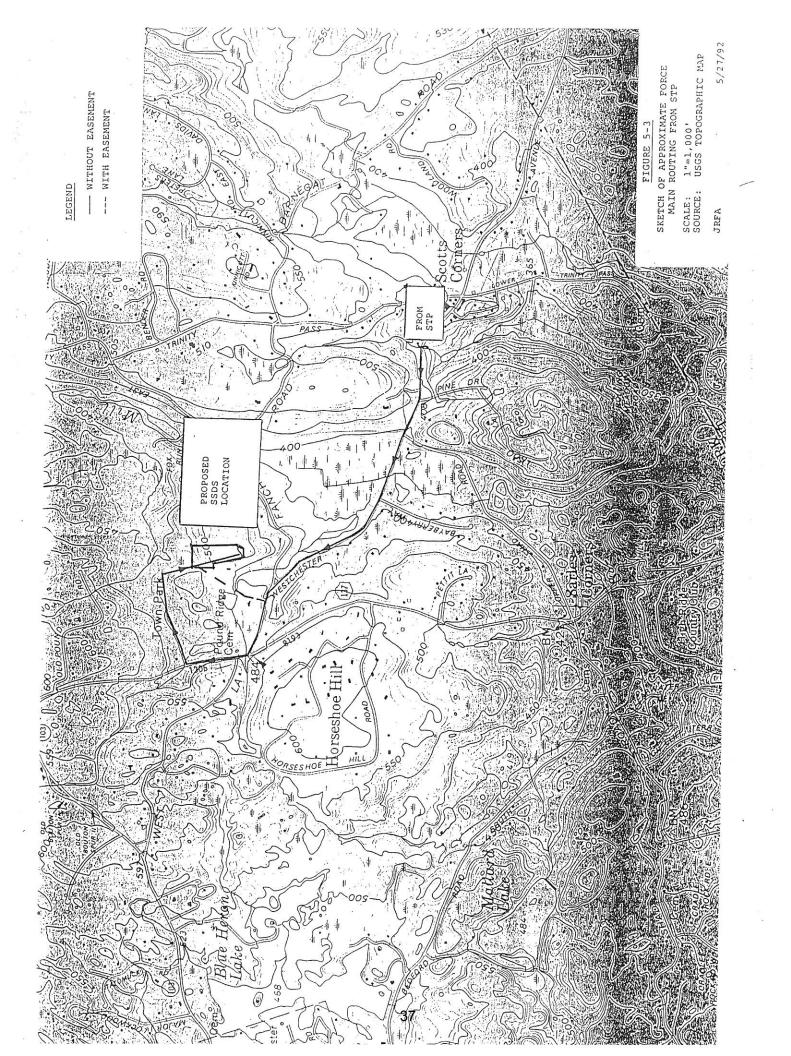
Based on the preliminary soil data, the Town parcel appears to be suitable for a SSDS location. It should be noted that this is data typically collected during SSDS tests for a single dwelling unit. As such it only provides preliminary information on the suitability of this site for subsurface discharge of the flows estimated from Scotts Corners. Detailed analysis of soils, hydrogeology and permeability are necessary to properly evaluate this site. In the absence of suitable base maps, it is estimated that approximately nine thousand feet of force main will be required from the STP to the SSDS. This force main will carry treated effluent only. Part of this cost is offset since there is no cost to be borne for the acquisition of this parcel for SSDS.

3

Utilization of this site will require a treated effluent pump station at the plant location.

Routing the force main along Westchester Avenue via the Town park to the SSDS site results in a 9,000 (\pm) foot run. Alternatively the Town may secure an easement as shown in the map with a resulting run of 6,300 \pm feet (see Figure 5-3).

5.4 <u>Treatment Plant Location</u>. All three sites discussed were proposed primarily as SSDS sites. Along with one other untested site in Scotts Corners, the Berman and Quade parcels could be suitable for locating the STP. The Town parcel could be suitable for both the STP and the SSDS. If the plant were located in Scotts Corners, the force main to the proposed SSDS location would carry highly



treated effluent instead of raw waste. A comparison of

these two options is depicted in Table 5-2.

Table 5-2

Comparison of Pumping Raw Waste vs. Highly Treated Effluent

Raw Waste

Treated Effluent

- Requires pretreatment in the form of comminution or grinding.
- Pretreatment will require some type of additional odor control.
- Requires separate Supplementary/Backup Power source.
- No additional treatment necessary.
- No additional odor control required.
- Uses STP Supplementary/Backup Power.
- Will most probably require smaller diameter pipe.
- Can use high efficiency pumps.
- Requires minimum 4-inch diameter pipe.
- Pumps used are low efficiency.

The advantages of pumping highly treated effluent are readily evident in this comparison. It is recommended that the STP be located in the Scotts Corners area.

6.0 Estimated Project Cost

6.1 Estimated Capital Costs

Capital cost estimates for the wastewater management facilities are based on the conceptual design, recent estimates from manufacturers and vendors, and prices for similar work. These estimates are subject to revision during the Facilities Report and the design phase.

The estimated 1992 construction costs for the plant and collection system are depicted on Table 6-1. It should be

noted that the cost of the land for the STP is NOT included.

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Table 6-1

Estimated 1992 Construction Cost

Item

		Cost
Site Work Site Preparation Earth Work Roads/Drainage	\$	48,000
Collection System Gravity Sewers Force Main & Pump Station	\$	450,000
Wastewater Treatment Facility Treatment Plant & Effluent P. Structures Electrical HVAC	\$.S.	360,000
Subsurface Disposal System	\$	400,000
Subtotal	\$1	,258,000
25% Conti	Ingency \$	315,000
Total	\$1	,570,000

6.2

Estimated Annualized Capital Costs

At present, the New York State Revolving Fund interest rate is approximately five percent (5%). It is assumed that the rate will rise between the submission of this report and final SRF approval of the project. A six percent (6%) loan rate is therefore assumed.

Table 6-2 summarizes the annualized capital costs and the parameters observed to determine them.

Table 6-2

Estimated Annualized Capital Cost & Parameters

Costs

Total Estimated Capital Cost	\$1	,570,000
Annualized Capital Cost	\$	137,000

Parameters

Eligibility	00 Percent of Proposed Facilities	
Interest Rate	5.0 Percent	
Loan Term	20 Years	

6.3 Estimated Annual Operation and Maintenance Costs

In addition to the capital cost of construction, the District will incur additional costs for operation and maintenance. These costs are listed in Table 6-3.

Table 6-3

Estimated	Annual	0&M	Costs
Electric			\$26,000
Labor			\$10,000
Maintenance			\$ 1,500
Т	OTAL		\$37,500

6.4 Estimated Total Annual Costs

Estimated total annual costs to the Scotts Corners Wastewater District are summarized in Table 6-4.

Table 6-4

Estimate Total Annual Costs Under NYSDEC Loan Program

Annualized	Annual	Total
Capital Cost	O&M Cost	Annual Cost
\$137,000	\$37,500	\$174,500

6.5 Cost Allocation Alternatives

The total annual costs presented above are an estimate, based on the application of an assumed SRF interest rate. These costs may be allocated among those who benefit in several ways.

- 6.5.1 <u>Scotts Corners District Only</u>. This alternative provides a single tier allocation among the users in the district. The entire annual cost is borne by the Scotts Corners district property owners. It may be allocated based on assessed property value, metered water use or pro-rating.
- 6.5.2 <u>Town Wide Allocation</u>. The Scotts Corners area, represents the major commercial center in the Town of Pound Ridge. Hence, one could reasonably conclude that the entire Town would benefit from maintaining the area in a viable condition. Using this rationale, either a single or double tier system may be considered.
- 6.5.2.1 <u>Single Tier Allocation</u>. This alternative provides for allocation of the entire capital and operating cost to the property owners on a town wide basis. It may be allocated

based on a flat fee, assessed property value or other acceptable formula.

6.5.2.2 <u>Double Tier Allocation</u>. This alternative provides for allocation of the capital cost only to the property owners on a town wide basis. Those property owners served by the system would be allocated a second tier of payment to cover the O&M costs. This system may be allocated by flat fee, assessed property value, metered water use or other acceptable formula or combination of formulas.

Therefore, although double tier allocation is feasible, the Town might consider continuing to pursue available grants and other financing sources that would minimize the cost to the taxpayer.

- 6.6 <u>Alternative Financing Sources</u>. There are several additional sources of financing at the State and Federal levels. These additional sources typically apply to municipalities experiencing economic hardship. Basic qualifications for each are discussed below.
- 6.6.1 <u>New York State Revolving Fund (SRF)</u>. In addition to their loan via sale of bonds, the SRF offers direct loans to two thirds or one third of the market rate, and also at zero percent interest. The Environmental Facilities Corporation, administrators of the SRF, assesses municipal

- 248 -

need based on individual application. The specific form is included as Appendix G.

- 6.6.2 <u>Farmers Home Administration (FMHA)</u>. FMHA offers loans and grants to poor rural communities. Telephone conversations between JRFA and Mr. Roy Wittich highlight the following requirements.
 - Loans are offered to communities that are not qualified under any other program (i.e., SRF) at comparable interest rates.
 - Grants are available to communities where the mean income is below the State poverty level.

The FMHA Application Form is included as Appendix H.

- 6.6.3 <u>Housing and Urban Development (HUD)</u>. HUD administers a Small Cities Community Development Block Grant Program. Telephone conversations between JRFA and Robert Guadagno indicate that fifty percent or more of the community population must consist of low to moderate income persons in order to quality for the program. The actual value of low-moderate income varies by County within the State. The HUD Application Form is included as Appendix I.
- 6.7 <u>New York State Revolving Fund Low Interest Loan Milestones</u> JRFA contacted Mr. Michael Sheehan, Construction Management Division, NYSDEC. The following sequence of milestones is in accordance with his recommendations.

- 243 -

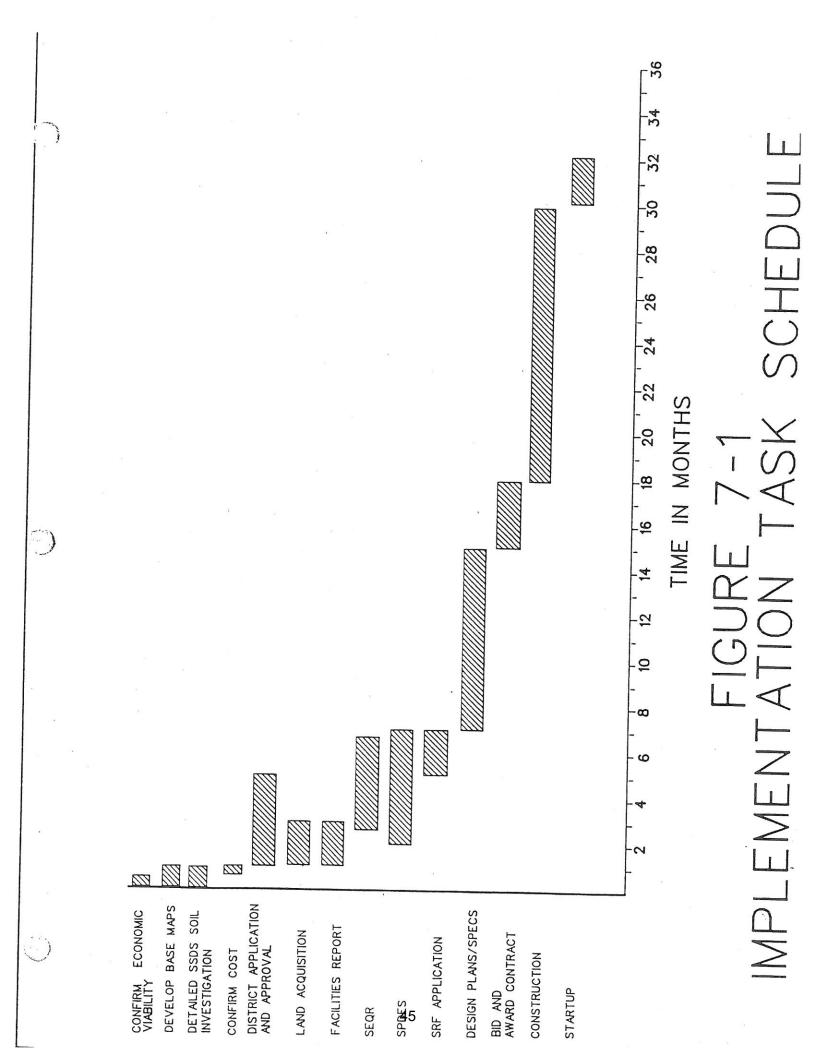
- 6.7.1 Establish Municipal Wastewater Treatment District.
- 6.7.2 Collect Water Quality data from Stamford Water Company.
- 6.7.3 Complete Facility/Engineering Report.
- 6.7.4 Meet with DEC for scoring of application.
- 6.7.5 See Appendix D for SRF Requirements.

7.0 Implementation Task Schedule

Figure 7-1 provides a graphic explanation of the various major tasks and estimated times associated with their completion.

8.0 Audit and Control Requirements

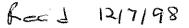
The New York State Board of Audit and Control reviews all applications for establishment of new sewer districts. Application must be made within ten days of adopting a resolution approving the establishment of the district. If the Town proposes to finance all or part of the cost it must prove that the formation of the district serves the public interest and that the cost does not pose an undue burden on the taxpayers. As recently revised, Part 85, Chapter III, 2 NYCRR outlines the application process in detail and is included in Appendix C.



Malcolm Pirnie Dec. 2, 1998 Pound Ridge Treated Wastewater Effluent Well Injection Letter to Clay Fowler PR Planning Board Summary

MP provides project approach and estimated rage of costs to prepare a permit application in support of deep well injection of wastewater for the Pound Ridge commercial area.

- Needs USEPA permit and NYS DEC has "no regulatory mechanism for such permits".
- EPA's primary concern is that the wastewater effluent will be injected into a potable water aqueduct.
- There are no such wells in Westchester (1998).
- Would have to inject into crystalline bedrock at 30,000 gpd (20.8 gpm).
- There are technical issues with keeping the well open, and need sufficient fractures in the rock
- Proposal includes breakdown of costs totaling \$100,000





MALCOLM PIRNIE, INC. ENVIRONMENTAL ENGINEERS, SCIENTISTS & PLANNERS

December 2, 1998

Mr. Clay Fowler Planning Board Chairman Town House 179 Westchester Avenue Pound Ridge, NY 10576

Re: Pound Ridge Treated Wastewater Effluent Well Injection Project Approach and Estimated Range of Costs

Dear Mr. Fowler:

Malcolm Pirnie, Inc. is pleased to provide you with this project approach and estimated range of costs to prepare a permit application in support of deep well injection of wastewater for the Pound Ridge commercial area. The injection of the treated effluent from the commercial area will require a Class V Underground Injection Control (UIC) permit administered by the United States Environmental Protection Agency (USEPA). Additionally, the State Pollutant Discharge Elimination System (SPDES) permit, administered by the New York State Department of Environmental Conservation (NYSDEC), would have to be modified, with the effluent limits being consistent with GA (groundwater) effluent standards.

1. BACKGROUND INFORMATION

We have had numerous discussions with Joe Marcogliese of the NYSDEC regarding effluent injection for similar projects. Mr. Marcogliese has stated that the NYSDEC would follow the lead of the USEPA regarding the permitting of the injection well, as the NYSDEC has no regulatory mechanism for such permits. Mr. Marcogliese has stated that the NYSDEC would modify an existing SPDES permit pending the issuance of a UIC permit by the USEPA.

We have also had numerous past discussions with Carol Lynes of the USEPA. Carol Lynes is in charge of administering UIC permits within the Westchester County area. The EPA's primary concern is that the wastewater effluent will be injected into a potable aquifer. Ms. Lynes has previously stated that the USEPA is in the process of modifying the UIC application requirements to include a specific classification for domestic effluent injection wells. While the "official" modified application requirements are not yet available, we have discussed what those additional requirements will be, and the USEPA has stated that they will send us written confirmation of the additional requirements. Ms. Lynes stated that because there are no existing effluent injection projects in Westchester County, the information and procedures will be reviewed carefully. Overall, the USEPA is not opposed



Mr. Clay Fowler Town House December 2, 1998 Page 2

to the concept of injecting wastewater as a disposal method. Some of the additional requirements mentioned by Ms. Lynes include maximum injection pressure calculations, geophysical logs, preliminary injection well design, breakthrough analyses, and monitoring and inspection plans.

2. TECHNICAL DISCUSSION

As with any deep well injection system, the hydraulic characteristics of the receiving geologic formation and the integrity of the overlying formations are the determining factors in evaluating the feasibility and ultimate success of the system. The geology in the vicinity of Pound Ridge, and northern Westchester County as a whole, typically consists of thin overburden deposits (predominantly glacial till) underlain by crystalline bedrock. The overburden is not of sufficient thickness or permeability to allow for the injection of wastewater at a rate of 30,000 gpd (equivalent to 20.8 gpm). Therefore, the receiving geologic unit will be in the underlying bedrock.

Groundwater occurs in bedrock in fractures and fissures. The degree to which groundwater can be transmitted through bedrock is dependent on the number and size of the fractures and the extent and interconnection of the fracture system. Therefore, it is necessary to locate a fracture system of sufficient size and areal extent to be able to effectively receive the injected wastewater.

As an injection medium, bedrock can be favorable over unconsolidated deposits because the borehole is open: in other words, there is no well screen or gravel pack. A common problem with injection systems is the fouling of well screens due to bacterial encrustation, chemical precipitation, high entrance velocities (due to the size of the screen openings), gas entrainment and the clogging of screens by particulate matter. These problems can be reduced with injection wells in bedrock, depending on the size of the fracture openings. Chemical precipitation may still occur along fracture surfaces, depending on the chemical and thermal characteristics of the entrance water compared to the receiving groundwater.

Because identifying prolific fractures is critical to the success of the deep well injection system, we would propose to conduct a geophysical investigation of the site utilizing the Very Low Frequency (VLF) system. Using the VLF will help optimize the siting of promising injection well locations. The VLF system receives frequencies that are transmitted through the earth's mantle (predominantly signals generated by the U.S. Navy). Depending on the strength and orientation of the received signals, more favorable fractures can be identified and differentiated from less favorable fractures. This will reduce the overall cost of the system because the VLF reduces the amount of "guess work" involved in siting a well. Furthermore, identifying and mapping site fractures will assist in determining recharge areas and potential hydraulic connection between the injection system and surrounding withdrawal systems. This information is required for the UIC permit application.



Mr. Clay Fowler Town House December 2, 1998 Page 3

3. PROJECT APPROACH AND ESTIMATED COST RANGES

In support of the UIC permit application data requirements, and determining the actual feasibility of deep well injection of wastewater, we would propose to complete the following tasks.

A. <u>Review Regional Data</u>

We will review existing, available data on the hydraulic characteristics of the region, including recharge/discharge areas, depth to groundwater, and identify other groundwater users surrounding the site, including distance to the site, and the type and depth of each well. This information is needed to develop a framework of conditions and uses around the site. The estimated range of costs to complete this task is \$2,500 to \$3,500.

B. Site Visit/Geologic Mapping

We will conduct a site visit to map the geologic structure (strike and dip) of the bedrock surface expressions on outcrops (if they exist) and conduct a fracture trace analysis. We will then prepare a map showing the orientation of geologic features on the site relative to the surrounding area. The estimated cost range to complete this task is \$2,000 to \$3,000.

C. <u>Geophysical Survey</u>

We will conduct a geophysical survey using the VLF instrument to locate waterbearing fractures on the site. We will then interpret the data and, in conjunction with the field mapping, we will map fracture orientation and fracture depths on the site. This information will be used to locate injection wells and assist in the hydraulic isolation interpretation between the injection points and withdrawal points. The estimated cost range to complete this task is \$5,500 to \$8,500, depending on the size of the survey area.

D. <u>Well Installation</u>

Based upon the data obtained in Tasks A through D above, we will make an assessment as to the viability of an injection well at the site. If the geologic structure is not favorable for injection, no additional work would be completed. If the geology is favorable, we will locate an injection well site and drill an injection test well to a maximum depth of 600 feet, upon consultation with the Town. The injection test well will be designed to transmit water at a discrete depth interval different than that of surrounding withdrawal wells. A maximum of three monitoring wells (total footage of 900 feet) will be installed at different depth intervals to monitor the



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Mr. Clay Fowler Town House December 2, 1998 Page 4

mounding effect of the injected water during the injection pilot test (see Task E below) as required by the UIC permit application. The estimated Malcolm Pirnie labor cost range to complete this task is \$10,000 to \$15,000. The estimated drilling subcontractor cost range is \$15,000 to \$20,000 (depending on the actual depths drilled).

E. Injection Well Pilot Test

Upon completion of the well installation, we will conduct an injection well pilot test. The maximum injection rate will be determined by first completing a step-test, whereby the injection rate is gradually increased until the back pressure is stabilized. After the step-test is completed, the pilot injection test will be run at the optimal rate for seven days. A period of seven days is necessary to allow for stabilization of the induced hydraulic mound and pressures to determine the area of influence created by the injection. We would need a supply of water for the injection test.

During the injection test, we will monitor water levels in the newly installed wells and up to five off-site wells continuously (24 hours a day) for the seven days. Additional information to be gathered will include injection flow rate, injection pressure, back pressure, water temperature and pH. Upon completion of the injection test, we will analyze and interpret the data and make a determination of the viability of the geologic formation to assimilate the injected water. This analysis will include a geochemical compatibility analysis of the injected wastewater and the receiving groundwater. The estimated Malcolm Pirnie labor cost range to complete this task is \$25,000 to \$35,000. The estimated subcontractor cost range is \$13,000 to \$18,000.

F. Injection Well Preliminary Design/Monitoring Program

We will prepare a preliminary design of the injection well and the monitoring program to be put a place once the UIC permit is issued. The monitoring program will be a very important component of our permit application package, as the USEPA is concerned about breakthrough and subsequent monitoring activities. The preliminary design will include injection pressure calculations, a schematic design and piping diagram. The estimated cost range to complete this task is \$10,000 to \$15,000.

G. Engineering Report and Permit Application

We will prepare a detailed engineering report in support of the UIC permit application, and complete the UIC permit application for submission to the USEPA. The estimate cost range to complete this task is \$8,000 to \$10,000.



Mr. Clay Fowler Town House December 2, 1998 Page 5

As we discussed on the telephone, the UIC permit application is a complicated process, particularly since the USEPA has little experience with domestic wastewater injection. The estimated cost ranges presented in this letter account for a heightened level of effort to provide the USEPA with technically sound and scientifically valid data in support of the UIC permit application. We would be happy to discuss our overall approach with you at your request.

We appreciate the opportunity to provide you with this information and look forward to assisting the Town of Pound Ridge on this project.

Please give me a call at 201-529-4700 if you have any questions.

Very truly yours,

MALCOLM PIRNIE, INC.

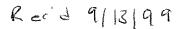
Michael van der Heijden, CGWP Associate

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Malcolm Pirnie Sept. 3, 1999 Wastewater Disposal Evaluation Letter to Clay Fowler PR Planning Board Summary

MP was retained to evaluate and provide alternatives separate sewage treatment systems (ssts) in Scotts Corners (SC), preliminary findings.

- Interviews determined that Block 9454 (SC Market), Lot 6 (Moonstruck) and Lot 7 (Albano electric) were experiencing recurring failures
- Lots 13, 14, 15 have cesspools with issues
- Summary of findings is in Table 1
- An estimate of water usage was made using data form the PR Business Association (Rosalie Roth) divided by the area of the buildings for a rate of 0.142 gallons/square foot / day. This was applied to properties that did not have water usage rates. Adding data from WCDH resulted in a water usage rate of 27,000 gpd. Only present usage included.
- Solutions proposed are:
 - Combined system for all users
 - Upgrading selected ssts
- Combined System
 - Wastewater Treatment Plant Previous study determined that it is a viable engineering solution; but the capital and operating costs render it not economically viable.
 - On-site septic and pump to "Ball Fields". Use ball fields as leach fields, versus disposal for treated effluent, would seem viable. Need septic tank maintenance. Would also have to address ball field underdrains.
 - On-site Treatment and Disposal, need 2.5 acres. Would have to negotiate waivers with DOH for reserve capacity or somehow spread the loading rate over 24 hours rather than business hours. Would result in restrictions to future development and might result in deed restrictions. Could truck effluent off site.
- Upgrade selected ssts's.
 - Upgrade selected ssts, for example Chubby's Lot 64, or Dinardo's Lot 60
 - Needs more study.
- See table 1 for existing Conditions Assessment





MALCOLM PIRNIE, INC. INDEPENDENT ENVIRONMENTAL ENGINEERS, SCIENTISTS & CONSULTANTS

September 3, 1999



Mr. Clay Fowler, Chairman Pound Ridge Planning Board Town House 179 Westchester Avenue Pound Ridge, New York 1576-1743

Re: Scotts Corners, Pound Ridge, New York Wastewater Disposal Evaluation

Dear Mr. Fowler:

Malcolm Pirnie, Inc. has been retained to evaluate and provide alternatives to the existing separate sewage treatment systems (SSTS) in Scotts Corners. It is our understanding that some of the existing SSTS in the Scotts Corners commercial business area have had reoccurring problems, and that previous studies have been conducted regarding sewage treatment alternatives in an effort to remedy these problems. The following paragraphs describe our preliminary findings and recommendations of this study.

A field visit was conducted to assess existing separate sewage treatment systems (SSTS) for individual property lots in the Scotts Corner commercial business area (Figure 1) on August 11, 1999. The field assessment included visual observations of the SSTS and their hydrologic setting and interviews with occupants of each building to determine previous problems with their systems. Data was also collected from the Westchester County Health Department and the Pound Ridge Building Department on the existing SSTS designs and capacities. Water usage rates were obtained from Rosalie Roth (a member of the Pound Ridge Business Association) and by incorporating an estimated water usage rate that was determined from this data. The square footage of each building was obtained from the Scotts Corners Planning Study written by Frederick P. Clark Associates in October 1990. Some businesses in Scotts Corners date back to the late 1930's, making it difficult to find information on their SSTS at this time.

Interviews with the occupants of each building revealed that the SSTS that serve Scotts Corners Market, Moonstruck and Albano Electric (Block 9454, Lots 6 and 7 respectively)



Mr. Clay Fowler, Chairman Town House September 3, 1999 Page 2

were the only businesses that appear to experience recurring failures. Previous failures of these systems may be related to stormwater runoff patterns, a high groundwater table, poor soil conditions or the proximity to shallow bedrock. For example, the location of the absorption trenches for Scotts Corners Market are located on top of a hill adjacent to the existing parking lot. Rock outcroppings visible in the area may indicate that the shallow underlying rock could be creating a type of "bathtub" condition where stormwater runoff percolates through the ground and accumulates on top of the rock surface. The Moonstruck and Albano systems are downhill of a wooded area that directs stormwater runoff into the absorption field area. The true cause for system failures can be better understood once subsurface investigations are conducted.

The presence of a cesspool that serves the Country Shopper, Antiques and Tools and an adjacent lot (Block 9454, Lots 13, 14 and 15 respectively) was reported during the site visit. The cesspool may be in poor physical condition and have insufficient capacity based on the age of the system. Building occupants stated that the system could be in excess of 100 years old. Further investigations regarding the condition and capacity of the cesspool should be conducted as the project approaches more detailed stages.

Access hatches with holes on the lid were also noticed in various locations during the site visit as shown on Table 1. Therefore, runoff infiltrates into the pumping pit or septic tank increasing the volume of flow into the system. The existing hatches should be replaced with new watertight hatches to minimize infiltration. This condition should be also be addressed as the project approaches more detailed stages.

Obtaining water usage rates is a critical parameter in designing the size of new or upgrading existing SSTS. An estimated water usage rate was determined by taking the sum of the water usage rates provided by Rosalie Roth divided by the sum of the square footage of the respective buildings. The resulting water usage rate factor of 0.142 gallons/square foot/day was utilized for estimating water usage rates at properties where Rosalie Roth did not provide data. Multiplying this factor by the square footage of an existing building yields an estimated water usage rate for that building. By combining data from Rosalie Roth and the Westchester County Department of Health the estimated water usage rate for the Scotts Corners commercial district is approximately 27,000 gallons per day (Table 1). It should be noted that this water usage rate estimate only includes existing buildings and does not factor in future expansion of existing stores or the construction of new structures. Future building expansion and the increase in water usage must be considered as the project approaches more detailed stages.

Based on the current estimated water usage rate of 27,000 gpd for the entire commercial district, a variety of possible solutions are available. Potential solutions have been

Mr. Clay Fowler, Chairman Town House

MALC

September 3, 1999 Page 3

subdivided into two general scenarios: 1.) using a combined system for all commercial uses or 2.) upgrading selected SSTS and continue using individual systems.

1. Combined System

- a. <u>Wastewater Treatment Plant</u>: A study regarding the viability of a WWTP with a subsurface effluent discharge to serve the commercial area has been previously completed. While a WWTP is a viable engineering solution, both initial capital and operation and maintenance costs (O&M) make this option economically not viable.
- b. <u>On-site Septic and Pump Station to Ball Fields</u>: The previously completed WWTP study proposed an on-site WWTP with the effluent being discharged at the ball fields located approximately 1.5 miles to the north. If the ball fields have sufficient area and capacity (which they appear to have), they could be used as a leach field, rather than disposal fields for treated effluent. This would involve having an on-site septic tank and a pump station to convey wastewater to leach fields located at the ball fields. This option would result in lower capital costs (no WWTP) and lower O&M costs. The only O&M would be associated with periodic septic tank cleaning and pump station maintenance. It should be noted that existing drainage patterns would have to be investigated during the next study phase. For instance, underdrains for the ballfield would have to be removed if they were discovered during further investigations.
- c. <u>On-Site Treatment and Disposal</u>: There are several possible solutions under this option that involve a combination of reserve capacity reduction and flow equalization. First, based on a flow of approximately 27,000 gpd and a percolation rate of between 30 and 45 minutes, a leach field area of approximately 2.5 acres would be required. The two acres does account for 100 percent reserved capacity as required by the Westchester County DOH. It may be possible to negotiate with the DOH a waiver for the reserve capacity. The downside to this option is a likely deed restriction on types of commercial use and expansion.

Second, if the flow rate were to be equalized, whereby the loading rate to the leach field would be dosed over a 24 hour period rather than normal working hours, the leach field size may be reduced by one-half. Incorporating equalization could reduce the required leach field area to approximately 1.5 acres. Combining reserve capacity reduction and flow equalization could reduce the leach field area to approximately 0.75 acres. Obviously, reducing reserve capacity is more of a business, as opposed to an engineering, decision. The balance between the need to occupy existing commercial space and the need for future expansion would have to be taken into account under this design scenario.



Mr. Clay Fowler, Chairman Town House September 3, 1999 Page 4

Flows above the 27,000 gpd design flow of the SSTS would require an additional method of sewage disposal. Trucking the sewage off-site is the most economical method (approximately \$500 per 1000 gallon truck tank full) of sewage disposal for the flows above what the new system has been designed for.

2. Upgrading Selected SSTS

A second scenario is to construct a smaller system to treat wastewater from only a particular area or from existing SSTSs that experience recurring failures. Possible locations for this "pocket" treatment system could include the overgrown area behind Chubby's Hardware (Block 9320, Lot 64) or the parking lot behind what is known as DiNardo's (Block 9320, Lot 60). Existing absorption trenches and leaching pits would have to be removed from any area where a new system was installed. It should be noted that a majority of the existing SSTS are currently operating without problems. Most commercial properties are land-locked by other buildings or natural features such as bedrock outcroppings or wetlands, preventing significant expansion. Some properties currently want to expand, such as the Scotts Corners Market, but are not able to because of the lack of treatment capacity.

It should be noted that these potential solutions are based on limited site-specific subsurface characterization and that other critical issues may be encountered when detailed soil investigations are conducted. We recommend that additional investigations be performed, including soil sampling and percolation tests, prior to proceeding with design and construction of a new treatment system.

If you have any questions please call me at 201-529-4700.

Very truly yours,

MALCOLM PIRNIE, INC.

Michael van der Heijden, CGWP Associate

c: D. Berman K. Matscherz, MPI D. Sweeten, MPI



P:3541003/28C_RPT.doc

TOWN OF POUND RIDGE SCOTTS CORNERS WASTEWATER TREATMENT STUDY EXISTING CONDITIONS ASSESSMENT

STORE NAME	OWNER	BLOCK	LOT	LAND USE	WATER USAGE (gpd)	BLDG. SIZE (sf)	SEPTIC TANK CAPACITY (gallons)	OVERFLOW TANK CAPACITY (gallons)	GREASE TRAP CAPACITY (gallons)	METHOD OF WASTEWATER CONVEYANCE	METHOD OF SEWAGE TREATMENT DISPOSAL	INFLOW POTENTIAL NOTED
THE DELI	DALE METZGER	9320	59	COMMERCIAL	575	4,050	SEE NOTE 5)	SEE NOTE 5)	NONE	GRAVITY FLOW		YES
DI NARDO'S	QUADE & ROTH INC.	9320	60	COMMERCIAL/RESIDENTIAL	4425 ⁴⁾	4,050	3,000	SIPHON TANK	750	GRAVITY FLOW	- 6.5' DIA. x 6' DEEP SEEPAGE PITS & 8 - 6.5' DIA. x 9' DEEP SEEPAGE PITS	
FASHION COIFFURES	CLEMONS TRUST	9320	61	COMMERCIAL/RESIDENTIAL	2700 ⁴⁾	4,050	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	NO
P.R. CLEANERS	DeGRAFF TRUST	9320	62	COMMERCIAL/RESIDENTIAL	1000 4)	4,860	1,850	NONE	NONE	GRAVITY FLOW	660 L.F. ABSORPTION TRENCHES	YES
P.R. TRAVEL	TRINITY LANE LTD.	9320	63	COMMERCIAL	1000 4)	4,050	1,000	NONE	NONE	GRAVITY FLOW	134 L.F. ABSORPTION TRENCHES	YES
CHUBBY'S	JOE DIPIETRO	9320	64	COMMERCIAL/RESIDENTIAL	1,035	7,290	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	NO
P.R. SHELL	SHELL OIL CO.	9320	65	COMMERCIAL	1,440	10,140	600	NONE	NONE	SUBMERSIBLE PUMP	TWO LEACHING PITS	NO
REX REALTY	NORMAN	9454	5	COMMERCIAL	1,150	8,100	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	NO
MOONSTRUCK	LOUIS MEDICO	9454	6	COMMERCIAL	820 ⁴⁾	3,103	1,200	1,200	1,000	550 GAL. PUMP PIT W/ SUBMERSIBLE PUMP		NO
ALBANO ELECTRIC	ALBANO REALTY	9454	7	COMMERCIAL	1,371	9,657	900	1,200	NONE	550 GAL. PUMP PIT W/ SUBMERSIBLE PUMP	200 L.F. ABSORPTION TRENCHES	1
HOULIHAN'S	BARING-GOLD	9454	8	COMMERCIAL	1,035	7.290	1,000	NONE	NONE	GRAVITY FLOW	132 LF. ABSORPTION TRENCHES	NO NO
TEXACO	CAPAZZO	9454	9	COMMERCIAL	1,035	7,290	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	Ì
FLEET BANK	ATEM ENTERPRISES	9454	10	COMMERCIAL	750 ⁴⁾	6,480	1,200	1,000	NONE	550 GAL. PUMP PIT W/ SUBMERSIBLE PUMP		NO
WINE CONNECTION	GATEWAY	9454	11	COMMERCIAL	855 4)	2,140	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	NO
ONE HR. PHOTO	VAZANNA	9454	12	COMMERCIAL/RESIDENTIAL	1,725	12,150	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	SEE NOTE 5)	YES
COUNTRY SHOPPER		9454	13	COMMERCIAL	605	4,260	CESSPOOL	NONE	NONE	GRAVITY FLOW	SEE NOTE 5)	YES
VACANT LOT		9454	14	COMMERCIAL	284	2,000	CESSPOOL	NONE	NONE	GRAVITY FLOW	SEE NOTE 5)	NO
ANTIQUES AND TOOLS		9454	15	COMMERCIAL/RESIDENTIAL	605	4,260	CESSPOOL	NONE	NONE	GRAVITY FLOW	SEE NOTE 5)	NO
COTTS CORNERS MARKET	APS REALTY TRUST	9455	PB-8/1.9	COMMERCIAL	1800 4)	58,225	2,700	NONE	NONE	2500 GAL. PUMP PIT W/ SUBMERSIBLE PUMP	1.043 L.F. ABSORPTION TRENCHES	NO
BARNWELL #1	DAVID BERMAN	9455	PB-B/25	COMMERCIAL	800 4	10,318	2,000	NONE	NONE	GRAVITY FLOW	LEACHING GALLERY (40'L x 5'W x 6'H)	NO
BARNWELL #2	DAVID BERMAN	9455	PB-B/24	COMMERCIAL	800 ⁴⁾	10,070					LEACHING GALLERT (40 L X 5 W X 6 H)	NO
BLDG. #1 & #2	DAVID BERMAN	9455	PB-B/24	COMMERCIAL/RESIDENTIAL			1,000	1,000	NONE	GRAVITY FLOW	ABSORPTION TRENCHES	
BLDG. #3	DAVID BERMAN	9455	PB-B/24	COMMERCIAL/RESIDENTIAL			SEE NOTE 5)	NONE	NONE	GRAVITY FLOW	TWO 50' LONG x 3' WIDE ABSORPTION TRENCHES	NO
BLDG, #4	DAVID BERMAN	9455	PB-B/24	COMMERCIAL			1,000	1,000	NONE	GRAVITY FLOW	ABSORPTION TRENCHES	NO
BEAUTY SPA	CONO ENTERPRISES, LTD	9456	5A	COMMERCIAL	1000 4)	4.257	1,000	1,000			200 L.F. ABSORPTION TRENCHES & 3 - 8' DIA. x 5' DEEP SEEPAGE PITS	NO
				TOTAL FLOW (gpd) -	26,810	ł				New Select One The Wy SOUNDAGEE FONIFI	200 L.C. HOSONETION THENCHES & 3 - 8' DIALX 5' DEEP SEEPAGE PITS	NO

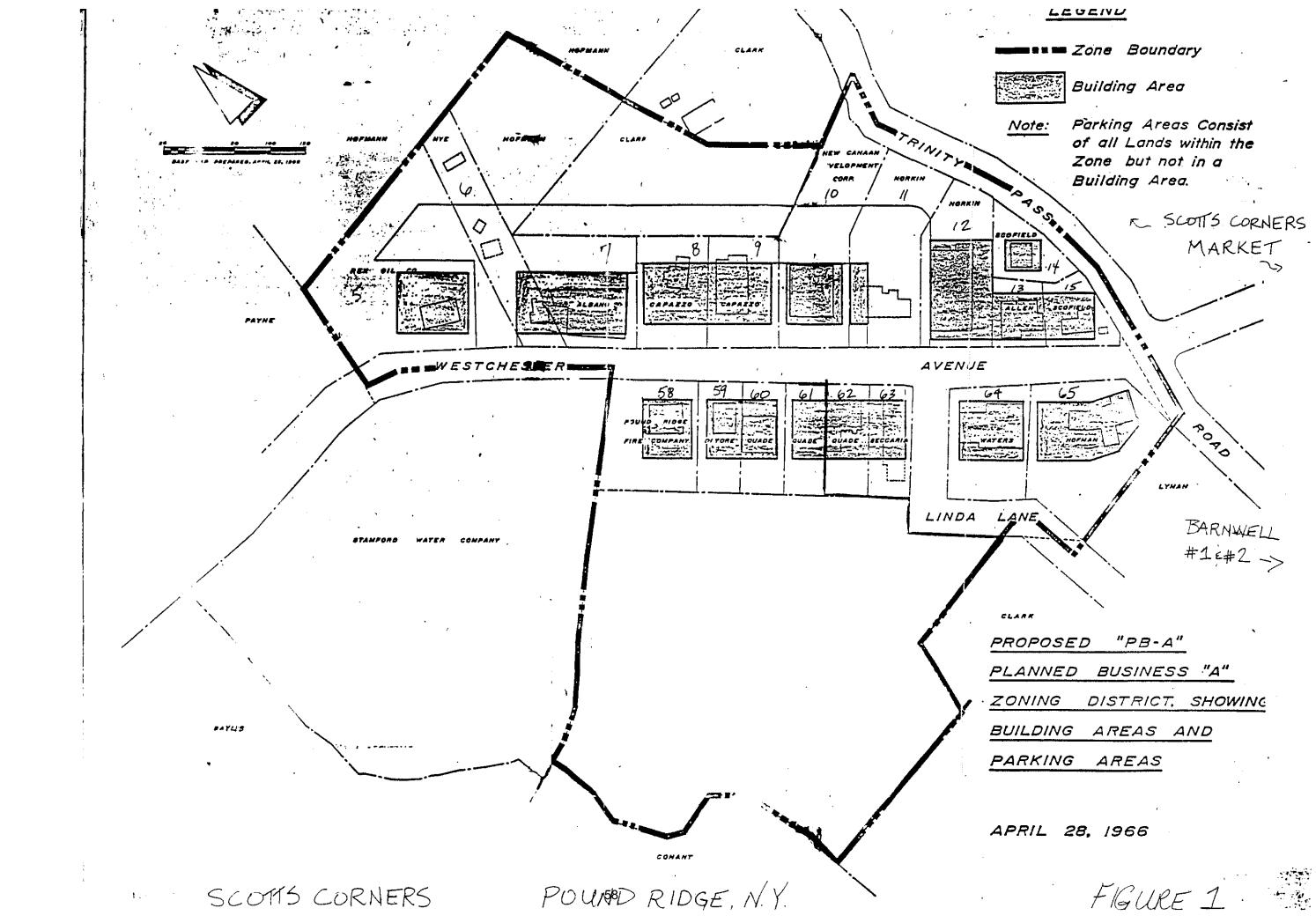
NOTES:

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ter d

SCOTTS CORNERS MARKET INCLUDES THE POST OFFICE NEXT DOOR
 BARNWELL #2 IS COMPRISED OF THE 4 BLDGS. LISTED BELOW.
 UNDER CURRENT ZONING REGS. THIS BLDG. IS OVER
 THE MAXIMUM DEVELOPMENT POTENTIAL.
 INFORMATION PROVIDED BY ROSALIE ROTH.
 INFORMATION IS NOT READILY AVAILABLE AT THIS TIME.





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June 29, 2000 Malcolm Pirnie to Malcom Pirnie Scotts Cornet Test Pits and Percolation Test Summary of the test, but no results. One page description – Lots 58 to 65 Photos

MALCOLM PIRNIE

INTEROFFICE CORRESPONDENCE

To: M. van der Heijden, NNJ M. Morgante, WHI **Date:** June 29, 2000

Copy: Project Files, 3541003

From: John Ifkovits, NNJ

Re: Scotts Corner Test Pits and Percolation Tests

On June 22, 2000, I oversaw the excavation of 3 deep test pits and conducted 4 percolation tests at Scotts Corner, Pound Ridge New York. Present for the test pits were Mike Morgante of MPI WHI, ED Deleney of The Westchester County Department of Health (WCDOH), and Marion Papas, WCDOH. AC&S Excavating of Pound Ridge supplied a Cat 426B backhoe and operator for the test pits.

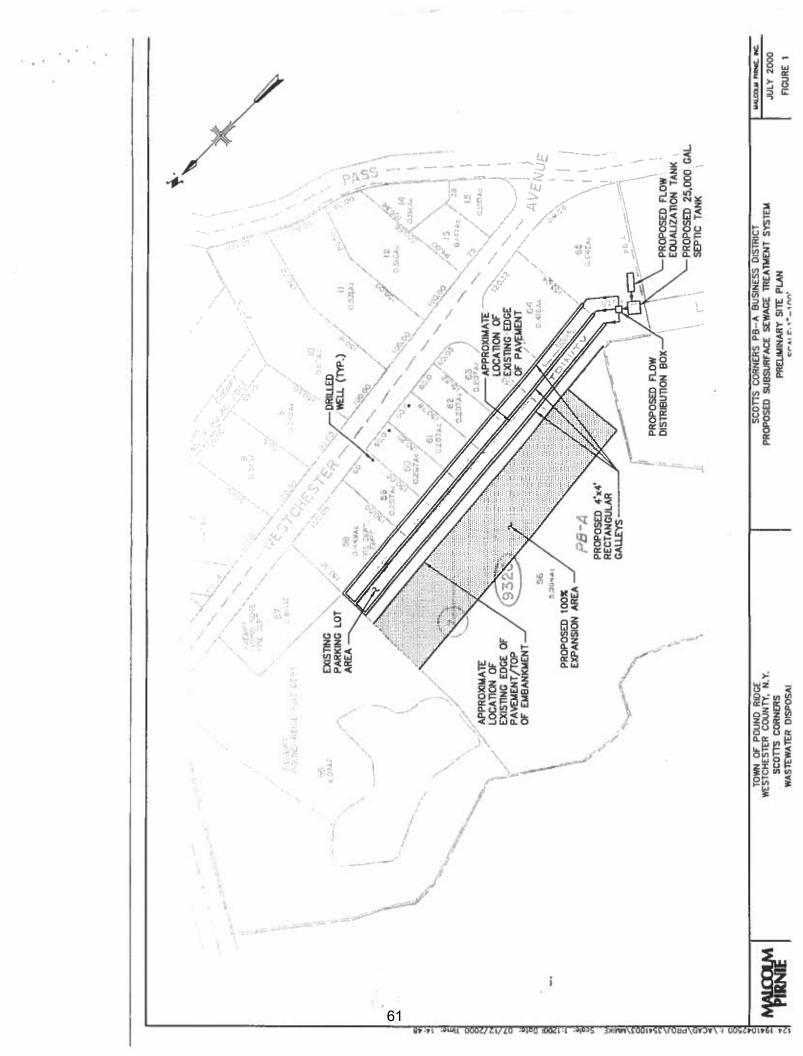
Mike departed the site during the excavation of the second test pit. Ed was content with three test pits. Additional test pits were planned for the area of the parking lot near the firehouse however Ed said they would not be necessary because the WCDOH has extensive records of the soils in this location. Underground utilities and the existing galleys were also located in this area. Ed and Marion departed the site after the third test pit. Test pit details are included on the attached test pit logs.

Four locations were chosen for percolation tests. There was one percolation test associated with each test pit and a fourth percolation test in the wooded lowland south east of the parking lot. The locations of the first three tests are shown on the test pit logs. Holes were dug as per requirements of WCDOH Bulletin SD-22. The percolation tests were not observed by WCDOH.

The holes were filled with water and allowed to pre-soak. The tests were run by filling the hole to a depth of ten inches and observing the time required for the water to drop three inches. The tests were run three times and are shown on their corresponding test pit logs. Percolation test P-4 filled with ground water and failed to drain.

The test pits were backfilled with the backhoe and compacted with the backhoe bucket. It will be necessary to arrange for the locations to be paved over.

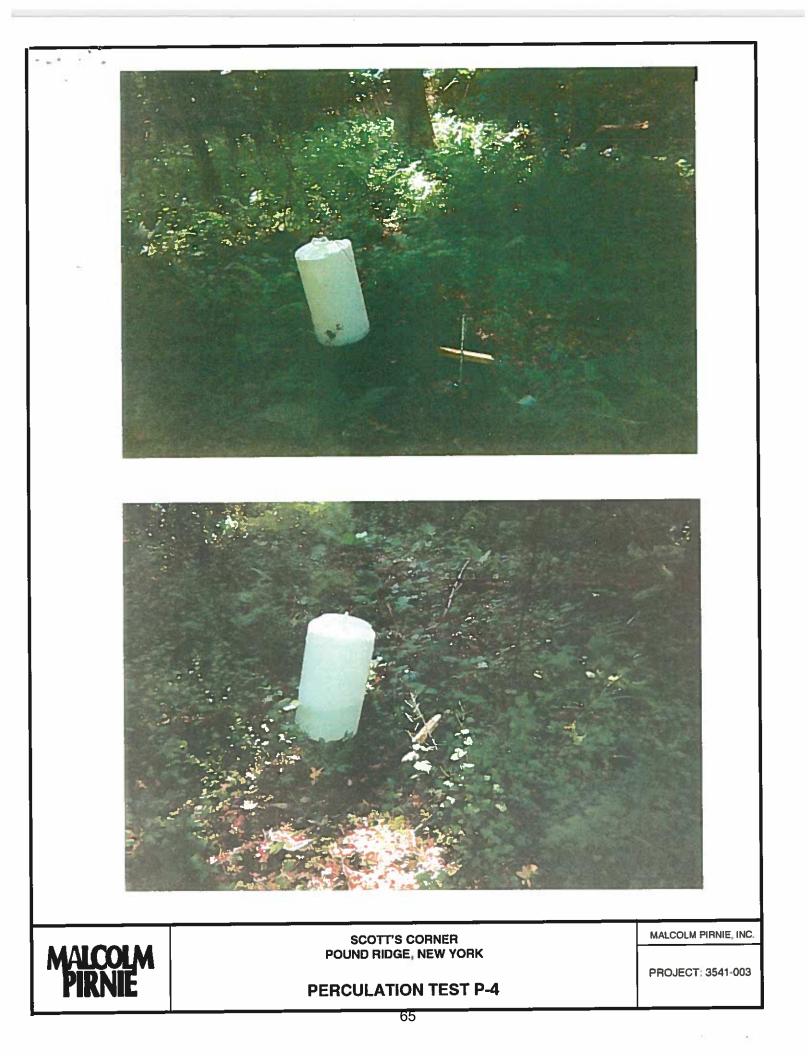
jji Attachments 3541003















July 11, 2000

Malcolm Pirnie to Clay Fowler

Scotts Corners Wastewater Treatment System

Results of test pits and percolation tests behind lots 60, 61 and 62, Block 9320

Summary

The report seems to be proposing a new SSTS in this area.

Existing SSTS behind Lots 58 through 65

- The existing leaching pits and adsorption trenches are 190 and 180 feet form public water supply wells
- Assumptions are made in order to support the proposed SSTS in this area.
- Would need a relaxation of the separation to public water supply wells

Proposed SSTS:

- See graphic for solution
- System could treat 24,000 gpd
- Details: 1800 feet of galleys, 24 foot on center, application rate of 0.6 gallons/day/ft2 from a perc rate of 24 minutes/inch (worst case) over 14,400 ft2 = 24,000 gpd

This does not address either the present or future flows of 28,000 from the 1992 Folcetti study This solution could be used to treat wastewater from only a particular area. Future study is recommended.

INCLUDES LOGS OF TEST PITS



FILE COPY

MALCOLM PIRNIE, INC. INDEPENDENT ENVIRONMENTAL ENGINEERS, SCIENTISTS & CONSULTANTS

DRAFT

July 11, 2000

Mr. Clay Fowler Pound Ridge Planning Board Town House 179 Westchester Avenue Pound Ridge, New York 1576-1743

Re: Scotts Corners Wastewater Treatment System

Dear Mr. Fowler:

Malcolm Pirnie, Inc. conducted a subsurface investigation in the parking lot area behind lots 60, 61, & 62 of Block 9320 in the PB-A Business District as discussed in the attached interoffice correspondence. The test pits that were excavated and the percolation tests that were performed found the existing soils suitable for subsurface wastewater disposal. The following paragraphs discuss the proposed subsurface sewage treatment system (SSTS) for Scotts Corners.

Preliminary Design of a New SSTS

Existing SSTS Behind Lots 58 through 65

The existing public water supply wells and surface water are located a minimum of 100feet away from septic tanks. The required separation distance from public water supply wells to absorption fields is 200-feet according to New York State Department of Environmental Conservation (NYSDEC) regulations. The existing leaching pits and absorption trenches are located approximately 190 and 180-feet, respectively, from the public water supply wells. In order to maximize the flow that the proposed subsurface sewage treatment system (SSTS) can accept the following is assumed:

- Existing separation distance of 100-feet between septic tanks and surface water and public water supply wells is maintained.
- Relaxation of the separation distance from the public water supply well and the proposed subsurface disposal fields to 100-feet minimum.

These requirements are subject to the review and acceptance by the Westchester County Department of Health (WCDOH) and the NYSDEC.



Mr. Clay Fowler Town House July 11, 2000 Page 2

Proposed SSTS

The SSTS will incorporate an influent equalization tank, a septic tank for settling and treatment and 4-foot high by 4-foot wide concrete rectangular galleys for subsurface disposal of the wastewater. It should be noted that various subsurface wastewater disposal alternatives such as leaching pits, concrete tri-galleys and rectangular galleys were investigated. There are other proprietary subsurface disposal methods that may provide more flow capacity which can be further investigated during detailed design. However, from this preliminary investigation it was determined that rectangular concrete galleys provide the greatest wastewater discharge capacity for this project.

The SSTS will be located behind lots 58 through 65 of Block 9320 as shown on Figure 1. Flow from the various businesses would be pumped to the influent equalization tank where the wastewater would be dosed to the septic tank and associated disposal fields. The total existing parking area behind lots 58 through 65 was utilized in the design of the SSTS to maximize the flow capacity. The 100% expansion area will be located southwest of the existing parking lot.

Approximately 1,800 linear feet (LF) of galleys spaced 24-feet on center can be installed in the existing parking lot area from lots 58 through 64. Each linear foot of rectangular galley corresponds to 8 square feet (ft^2) of subsurface disposal area. A wastewater application rate of 0.6 gallons/day/ ft^2 was determined from a percolation rate of 24 minutes/inch (worst case) based on field tests. Using this application rate and a subsurface disposal area of 14,400 ft², the maximum capacity of the subsurface disposal fields based on the assumptions that have been made is approximately 24,000 gallons/day (gpd).

Existing Wastewater Flows in the PB-A District

The total flow from the existing PB-A district is approximately 27,000 as found in Table 1 of the letter dated September 3, 1999 that was sent to you. The projected flow in the year 2012 based on the Feasibility Study prepared by J. Robert Folchetti & Associates in May 1992 is approximately 28,000 gpd.

Conclusions/Recommendations

The 24,000 gpd capacity of the proposed SSTS is based on preliminary information and may increase during the detailed design stage. However, the proposed system may not be capable of current and future treatment of the total flow from the PB-A business district. Furthermore, if the assumptions that have been made are not determined feasible upon review by the regulatory agencies, a similar system with less capacity can still be



Mr. Clay Fowler Town House July 11, 2000 Page 3

constructed in the same location. It would therefore be recommended to treat wastewater from only a particular area and from existing SSTS that experience recurring failures as previously discussed in Scenario No. 2 in the letter dated September 3, 1999. This proposed SSTS could accept flow from the previously identified businesses that have recurring failures of existing SSTS and the businesses from lots 58 through 65 of block 9320.

Upon the review and acceptance of the preliminary design by the regulatory agencies, the next phase of the project would require the following:

- A detailed investigation of existing and projected flows and current and future building use to accurately determine the design flow for the proposed SSTS.
- A site survey to establish the boundaries of the entire area from lots 58 through 65 in order to commence the detailed design of the subsurface disposal system.

If you have any questions or concerns, please do not hesitate to call me at 914-641-2658.

Very truly yours,

MALCOLM PIRNIE, INC. Zeral

Michael van der Heijden Associate

mam Enclosures

- Cc: E. Delaney, WCDOH
 - D. Berman, Scotts Corners Business Association
 - R. Roth, Scotts Corners Business Association
 - M. Morgante, MPI

MALCOLM PIRNIE

TEST PIT LOG

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	M PIRNIE						T PIT LOG					
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Malcolm Pirnie Sept.26, 2000

Scotts Corner Septic Evaluation – Scope of Work and Cost Estimate Letter to Clay Fowler PR Planning Board

Summary

- WCDOH will not relax the separation distance from existing water supply wells (200ft)
- Could consolidate the wells with a new one.
- WCDOH said they would not use the worst case percolation rate to determine hydraulic loading, but would allow a "reasonable" rate to be used.
- WCDOH states that the reserve capacity of the individual systems could be used to create an aggregate reserve capacity of multiple locations.
- Propose some kind of hybrid system, a combination of new leach fields (behind lots 59 through 63), maximizing the efficiency of the existing systems, and tying them all together.
- Need to:
 - 1. Inventory water supply wells
 - 2. Calculate water uage and wastewater discharge volume
 - 3. Figure out individual septic system details; tank and field locations and size.
 - 4. Calculate hydraulic loading
- Create base map
- Determine water usage, purchase and install meters.
- Inspect existing septic sysytems
- Calculate loading of existing systems
- Support creation of a community water supply
- Prepare modified preliminary design
- Total Cost is \$30,000.
- Create new leach field behind Lots 60 through 62 is needed.



FILE (OPI

MALCOLM PIRNIE, INC. ENVIRONMENTAL ENGINEERS, SCIENTISTS & PLANNERS

September 26, 2000

Mr. Clay Fowler, Chairman Town of Pound Ridge Planning Board Town House 179 Westchester Avenue Pound Ridge, NY 10576-1743

Re: Scotts Corner Septic Evaluation Scope of Work and Cost Estimate

Dear Mr. Fowler:

As you know, we have been making some progress in developing a solution to the Scotts Corners septic situation. The deep tests and percolation tests conducted beneath the parking lot yielded favorable results. In our draft letter to you dated July 11, 2000, we identified a possible disposal scenario based on some assumptions that would require certain waivers from the Westchester County Department of Health (WCDOH). One such waiver included the reduction in separation distances from the water supply wells to the septic system. The WCDOH required separation distance is 200 linear feet.

On September 6, 2000, we had a conversation with Ed Delaney of the WCDOH. Mr. Delaney stated that they are willing to be flexible, but they will not relax the 200 foot separation distance requirement. As you know, a significant number of the commercial properties have their own water supply well. The wells are located throughout the Scotts Corners commercial area and, therefore, maintaining the 200 foot separation distance reduces the amount of space available for leach fields. One strategy the Town may wish to consider is to consolidate all of the individual wells into one community system comprised of one or two wells. This consolidation would allow for greater flexibility in the placement and sizing of the systems. There may be grants or low interest loans available for the construction of a community well system. We have included a task in this letter to assist the Town in identifying and obtaining such grants or low interest loans. It should be noted that creation of a community well system is not a requirement for us to develop a solution for the existing failing systems. It will, however, impact the amount of excess capacity available for expansion.

During our conversation with Ed Delaney, we discussed two other issues: 1. Utilizing a "reasonable" percolation rate, rather than a worst case; and 2. Use of individual system reserve fields as an overall system reserve.

ONE INTERNATIONAL BOULEVARD BOX 601 MARWAH, NJ 07430-0601 201-525-4700 fax 201-520-1415 http://www.purple.com



Mr. Clay Fowler Town of Pound Ridge Planning Board September 26, 2000 Page 2

With respect to the first issue, the WCDOH typically will use the worst percolation test rate to devise the hydraulic loading-rate. Upon inquiry, Mr. Delaney stated that the WCDOH would not require Scotts Corner to utilize the worst percolation test, but rather would accept a "reasonable" rate that represents all the percolation tests. This will allow for an increased hydraulic loading rate and decreased overall area.

With respect to the second issue, we asked Mr. Delaney if it would be possible to use the reserve capacity of the existing systems as an aggregate reserve to service all of Scotts Corners. This strategy would allow us to maximize the capacity of overall system by "patching" together the individual systems. Mr. Delaney stated that he would not be adverse to such an approach, and that the overall reserve capacity of the "system" could be comprised of multiple locations.

At this juncture, it appears that some form of hybrid system will ultimately work for Scotts Corners. Specifically, a hybrid system would consist of using a combination of new leach field areas (e.g., the area behind lots 59 through 63), maximizing the efficiency of the existing systems, and tying the system together.

We are at the point where we need to start understanding some of the detailed engineering components of the existing systems so that we can make more definitive decisions on what will or will not work. Specifically, we need to understand the following:

- 1. How many water supply wells exist and where are they located?
- 2. What is the actual water usage for each user in Scotts Corners and what is the actual wastewater discharge volume?
- 3. What is the actual design of each septic system, including septic tank location, septic field locations, and septic field size?
- 4. Based on items 1 through 3 above, what is the actual capacity (e.g., hydraulic loading capacity) of each septic system?

Without the information mentioned above, it will be difficult for us to make decisions to move the engineering design forward. Therefore, we recommend the following steps to obtain more detailed and specific information.

1. Create a base map showing all of Scotts Corners, each septic system, and well locations. As part of a pervious task, we have compiled maps showing the locations of the majority of the septic systems. However, these maps are of

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2.

Mr. Clay Fowler Town of Pound Ridge Planning Board September 26, 2000 Page 3

varying scales and only show individual properties. Because the final system may be comprised of a combination of new and existing leach fields, we need one base map that shows all the septic systems on which we can layout the design. To create a base map, we would scan the existing drawings into a computer and compile the pieces together at a uniform scale. The estimated cost to create a base map is \$ 4,500.

Determine actual water usage. While the water use at Scotts Corners has been estimated in the past by us and others, it is important to know the actual use to determine the adequacy of the existing septic system and to aid in the sizing of a new system. Such information would also be valuable to justify specific design parameters with the WCDOH, and in the siting of a community water supply well, if necessary. This can be easily accomplished by placing water meters at each of the buildings. Water meters cost around \$100.00 each. Water use can be monitored and recorded by each tenant on a weekly basis.

For this task we would assist Scotts Corners in the purchase and placement of the water meters and the tabulation of the data. The estimated Malcolm Pirnie labor for this task is \$ 2,000. The estimated cost for the water meters is \$2,500, assuming there are 25 meters required at a cost of \$100 each.

- 3. Existing Septic System Inspection. As part of a pervious task, we identified the type of septic system for each of the buildings at Scotts Corners. It is now time to do a detailed inspection of each of the septic systems to determine the size of the tanks, linear feet of leach field pipes, leach field size, reserve field size, and overall condition. This information will be important in determining the adequacy of the existing systems to accommodate the existing hydraulic loads, and whether or not each system could accommodate additional loads. The inspection information will also be used to update the base map. The estimated cost for us to inspect each system is \$ 4,000.
- 4. Calculate Hydraulic Loading Capacity of Existing Septic Systems. Based on the results of the septic system inspection, we will calculate the hydraulic loading capacity of each of the existing septic systems. This will provide us with insight as to whether or not the existing systems are adequate to accommodate the existing loads and whether or not they can accommodate additional loads. We will also identify/recommend modifications (if applicable) that could enhance the capacity of each system. The estimated cost to complete this task is \$ 2,500.



Mr. Clay Fowler Town of Pound Ridge Planning Board September 26, 2000 Page 4

5. Community Water Supply System Grant. We will assist Scotts Corners to identify possible sources of funding (grants or low interest loans) to install a community water system. We will also provide assistance in preparing application materials if an appropriate source of funding is located. It should be noted that the installation of a community water supply system is not critical to moving forward, however, if a community system could be put in place, there would be increased flexibility and opportunities for leach field placement and design. The estimated cost to complete this task is \$ 1,500.

6. Prepare a Modified Preliminary Design. Once Tasks 1 through 5 are completed, we will have sufficient information to modify the design for the new leach field area, optimize the capacity of the existing systems, and prepare a preliminary design tying the system together. The design would be shown on the base map and would be the basis for discussions with the WCDOH. We have also budgeted for two meetings as part of this task. The estimated cost to complete this task is \$12,500.

The total cost to complete this next phase of work as discussed in Tasks 1 through 6 above is \$ 29,500 (including the cost for the water meters).

We believe that the approach discussed in this letter has the best opportunity to succeed. Based on our pervious work, we are confident that, at a minimum, the new leach field area (behind lost 60 through 62) would be sufficient to alleviate the existing failing system problem. The goal now is to attempt to optimize the existing systems so that additional capacity may be incorporated into an integrated system comprised of new and existing septic system components.

We appreciate the opportunity to provide you with this Scope of Work and Cost Estimate. If you have any questions, please call me at 201-529-4700.

Very truly yours,

MALCOLM PIRNIE, INC.

Michael van der Heijden Associate

jtC 1999-0577-739 P:\URG\Market\Lown of pound ridge proposal.doc 2002 <u>Summary form List of Reports</u>

April 2002

Scotts Corners Potable Water and Wastewater Conceptual Investigation Letter from Folchetti & Associates to Joy Simpkins

Waste WaterWastewater System investigation for Scotts Corners northwest parking lot.

WCDOH would not relax separation distances to wells.

- Quad Parcel (9320-56) found unsuitable for SSDS.
- Town Park site may be suitable for SSDS.
- Golf Course option for disposal through irrigation may be feasible.
- Potable Water
- 2 systems provide water to Stamford, CT: Stamford System & Laurel System.
- Laurel System supplies N. Stamford. It is a closed system and providing 60,000 gpd to Scott's Corners would require a system upgrade.

• Stamford System is questionable because it does not have a large margin of safety.

• Water service via Golf Course may be viable alternative. BHC was supportive.

Additional technical notes extracted by TD on the 2002 letter 3/20/2016

Parking Lot SSDS for Lots 56 (Quade)and 58 through 65 (Block 9320) is not feasible

- Need to be able to treat 50,000 gpd
- For 60, 61, and 62, preliminary design indicated a 4X4 galley SSDS could handle 24,000 gpd. But this requires relaxation of separation distance from public water supply well from 200 to 100 feet.
- Also sizing of the above will only handle 8,640 gpd.
- Then need to use Lot 56.
 - But test pits in 1992 determined it was not feasible due to groundwater within 4 feet of the surface, bedrock or boulder within 5 feet of the surface, and proximity to wetlands.

Park Athletic Facilities Lots 8, 86, 152, Block 9820.

- Even with potentially favorable soils, the area needed would require variances from regulatory agencies in terms of application rates and reserve areas.
- Perc testing was not done as a drought condition caused WCDH to suspend soils testing.
- For 86: 5 holes were done for perc tests, 11 to 27 min/inch
- 5.4 acres available with 150' setback from wetlands, and 20 foot offset from wetlands
- For a flow of 50,000gpd, @100% reserve area, need 6.7 acres using a 4X4 galley system on 14 foot centers.
- For a credit for treated effluent, allowing a 25% increase in loading, and a 50% reduction in reserve area, the area required for a 4X4 galley would be 4.02 acres.

- Slopes are OK at 4 to 20%, with 20% allowable by 1988 DEC design standards.
- BUT 4X4 not allowed on 20% slope areas; need tri-gallies
- For a flow of 50,000gpd, @100% reserve area, need 9.2 acres using trigallies on 12' centers.
- For a credit for treated effluent, allowing a 25% increase in loading, and a 50% reduction in reserve area, the area required for tri-gallies would be 5.5 acres.

Wastewater treatment through Golf Course Irrigation

- See discussion seems unlikely though a permit was issued to a golf course in Orange County.
- Not sure here, but as the golf course has at this point been built, it may not be an option

Potable Water

- Trinity Reservoir, part of the Stamford system. The Laurel also provides raw water to the water treatment plant on Interlaken Rd. in Stamford.
- Potable water is distributed via the Stamford system and the Laurel System
- The possibility of providing Scotts Corners with 60,000 gpd would require system upgrades and storage facilities.
- The Stamford System is fully utilized.

Ground Water Resources

• Long story made short is that drilling two test wells on BHC land at a rate of 60,000 is too risky and might ultimately result in the migration of the MTBE plume.

Water Service via the "Proposed" Golf Course.

- The golf course raw water storage tank might be operated by BHC and might have an allocation for Scotts Corners.
- Use of this water for potable purposes would have to involve the CT DEP, Dept of Public Health, and Dept, of Public Utility Control.
- This report precedes the golf course development so an update would be needed.

J. ROBERT FOLCHETTI & ASSOCIATES, L.L.C.

CIVIL / ENVIRONMENTAL ENGINEERS

247 ROUTE 100 PINEWOOD BUS. CTR. SOMERS, NY 10589 (914) 232-2500 (914) 232-6827 (FAX)

40 RAILROAD AVENUE MONTGOMERY, NY 12549 (845) 457-5318 (845) 457-9392 (FAX)

FAX TRANSMITTAL SHEET

TO: Honorable Joyce Simpkins

FROM: Robert M. Trzepacz, P.E.

FAX #: 914-764-0102

DATE: April 12, 2002

RE: Please find the attached correspondence, as we discussed on April 4, 2002. Originals will follow in the mail. Should you have any questions, please do not hesitate to contact me. Thank you.

NO. OF PAGES (including cover sheet):

7

PLEASE DELIVER THIS DOCUMENT IMMEDIATELY TO ADDRESSEE. PLEASE TELEPHONE US AT (914) 232-2500 IF ANY DOCUMENT IS ILLEGIBLE OR IF ALL PAGES ARE NOT RECEIVED.

J. ROBERT FOLCHETTI & ASSOCIATES, L.L.C. CIVIL / ENVIRONMENTAL ENGINEERS

April 11, 2002

Honorable Joyce Simpkins Pound Ridge Town House 179 Westchester Avenue Pound Ridge, New York 10576

SUBJECT: SCOTT'S CORNERS POTABLE WATER AND WASTEWATER CONCEPTUAL INVESTIGATION

Dear Ms. Simpkins:

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We would like to take this opportunity to advise you of the progress of our investigation. Alternatives are presented in two sections: wastewater and potable water.

1. WASTEWATER SYSTEM

1.1 PARKING LOT SSDS FEASIBILITY (LOTS 56 AND 58 THROUGH 65 BLOCK 9320)

Review of previous soil testing data and other data provided indicates that construction of a subsurface disposal system to treat an average daily flow of 50,000 gpd is not feasible given the following considerations:

- Subsurface investigation of lots 60, 61 and 62 of block 9320 and preliminary design of a subsurface disposal system (SSDS), reported July 11, 2000, concluded the maximum capacity of a 4x4 galley SSDS system at this location to be approximately 24,000 gpd.
- System capacity of 24,000 gpd required relaxation of required separation distances from the public water supply well from 200' to 100 feet.
- Telephone conversations with E. Delaney, WCDOH indicate that relaxation of the separation distance will not be permitted at this time.

247 ROUTE 100
 Pinewood Business Center
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40 RAILROAD AVENUE
 MONTGOMERY, NY 12549
 845-457-5318
 Fax 845-457-9392

Honorable Joy Simpkins Scotts Corners Potable Water and Wastewater Conceptual Investigation April 11, 2002 Page 2

We disagree with the system sizing provided in the July 11, 2000 report. Our calculations indicate the galley system proposed (1,800 lf of 4x4 gallies) is capable of treating approximately 8,640 gpd based on 1988 NYSDEC regulatory standards and the percolation rate information provided.

Use of lot 56, Block 9320, would be required to provide for flows greater than 8,640 gpd and was proposed for reserve absorption area under the July 11, 2000 layout.

- Test pits excavated January 15,1992 on Lot 56, Section 9320 determined:
 - Bedrock or rocks too large to move with a standard backhoe were found within 5 feet of the surface
 - Groundwater was within 4 feet of the surface
 - Useable area extremely limited due to proximity to wetlands, rock and groundwater
- The "Quade" parcel (TM# 9320-56) was found to be unsuitable for use as a subsurface discharge site in the <u>Wastewater Treatment Feasibility Study</u>, <u>Scotts Corners, Pound Ridge, NY</u>, prepared by J. Robert Folchetti & Associates, May 1992

1.2 PARK ATHLETIC FACILITIES (LOTS 8, 86, 152 BLOCK 9820)

Preliminary investigation of this parcel based on available information indicates soils characteristics and ground slopes are conducive to subsurface disposal. Gross system area requirements necessitate allowances from regulatory agencies for discharge of treated effluent in the form of an application rate credit and reduction of reserve area. Even with the credits use of tri-gallies is marginal and use of a 4x4 galley system would require further discussion with WCDOH. Subsurface disposal credit for treated effluent is not provided by WCDOH or NYSDEC. Discussions with WCDOH suggest that treated effluent credit may be considered by the regulatory agencies if it were requested

To advance this alternative, additional soils tests are required to determine depth of impervious surfaces, soils identification and groundwater depth. Percolation testing will be required for estimation of hydraulic conductivities in accordance with regulatory standards for preliminary system design. Testing is not possible at this time as Westchester County Department of Health has suspended soils testing since December 2001 due to the temporal drought conditions.

Honorable Joy Simpkins Scotts Corners Potable Water and Wastewater Conceptual Investigation April 11, 2002 Page 3

This assessment was based on the following factors:

The "Town" parcel (TM# 9820-86) may be suitable for use as a subsurface discharge site in the <u>Wastewater Treatment Feasibility Study. Scotts Corners.</u> <u>Pound Ridge, NY</u>, prepared by J. Robert Folchetti & Associates, May 1992, based on reconnaissance soils tests.

Reconnaissance soil tests conducted May 1992 provided the following results:

- 5 deep holes excavated overall depth 7-9 ft.
- No groundwater encountered
- No bedrock encountered
- Clayey sand soils in upper horizons, 3 holes had lower horizons of gravelly sand
- Percolation tests were conducted at all 5 locations at a depth of 4 ft. Rates ranged from 11 to 27 min/ inch
- Gross lot area available for SSDS is approximately 5.4 acres which maintains a 150' setback from wetlands designated by Marc Beroz, January 29, 2002 and a 20 offset from lot lines
- Gross area required for 4x4 galley system to treat an average flow of 50,000 gpd (including 100% reserve area) is approximately 6.7 acres. Galley spacing 14 ft. on center as stated by WCDOH
- Should a credit be permitted for treated effluent allowing a 25% increase in loading and 50% reduction in reserve area, the required area for 4x4 gallies would be approximately 4.02 acres (including 50% reserve area)
- Site slopes vary from 4% to 20%. NYSDEC 1988 Design Standards stipulate maximum slopes for SSDS are 20%.
- WCDOH indicated 4x4 gallies may not be permitted for use on slopes approaching 20%, tri-gallies are recommended

Gross area required for a tri-galley system to treat an average flow of 50,000 gpd (including 100% reserve area) is approximately 9.2 acres. Galley spacing 12 ft. on center as stated by WCDOH

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Honorable Joy Simpkins Scotts Comers Potable Water and Wastewater Conceptual Investigation April 11, 2002 Page 4

Should a credit be permitted for treated effluent allowing a 25% increase in loading and 50% reduction in reserve area, the required area for tri-gallies would be approximately 5.5 acres (including 50% reserve area)

WASTEWATER TREATMENT THROUGH GOLF COURSE IRRIGATION

Representatives from the NYSDEC, WCDOH and the Golf Course Developer were contacted and were presented with the concept of treated effluent disposal through irrigation. The NYSDEC representative indicated that a golf course in Orange County, NY was issued a discharge permit for this type of system. Both regulators felt that this could be a feasible alternative for Scotts Corners though more information would be required for further discussion.

NYSDEC stated they would obtain a copy of the Orange County discharge permit for our use and indicated that tertiary treatment would probably be required, most likely in the form of sand filtration. WCDOH comments focused on application timing, rain events, public exposure and storage requirements. Legal council for the developer stated they would be open to discussion of this matter, however would not want this to adversely affect the status of their application currently before the Town Planning Board.

Development of this alternative would require equalization in addition to a conventional means of discharge since land application will be weather dependent and largely seasonal. Connecticut Department of Environmental Protection prohibits surface water discharge within a water supply basin, relaxation of this requirement may be necessary. CT-DEP stated that they would not be inclined to allow a surface water discharge within a water supply basin.

2. POTABLE WATER

TRINITY RESERVOIR 2.1

According to Bridgeport Hydraulic Company representatives the Trinity Reservoir is one of the contributing reservoirs to the Stamford System which provides potable water treatment plant on Interlaken Road in Stamford, CT. The Laurel reservoir also supplies raw water to this treatment facility. Potable water is distributed from the plant via two systems, the Stamford System and Laurel System.

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Honorable Joy Simpkins Scotts Corners Potable Water and Wastewater Conceptual Investigation April 11, 2002 Page 5

The Laurel system which supplies North Stamford is a closed system which is a pressurized zone with no hydro-pneumatic or atmospheric storage. Providing 60,000 gpd for the Scott's Corners Area would require equipment upgrades to the existing Laurel System and it is likely that a means of storage would have to be required given the increased service area and usage.

Bridgeport Hydraulic Company indicated that the Stamford System does not have a large margin of safety and it is questionable if sufficient supply exists to provide 60,000 gpd to Pound Ridge. Mandatory usage restrictions are in effect in Stamford at this time. Treated water from the Interlaken Road Plant is supplemented by a small well and purchased water from BHC's Main System for distribution to its Connecticut customers. Given the Town's agreement regarding rights to raw water resources, BHC felt further investigations were required to determine the most appropriate means to provide potable water to this area.

2.2 GROUNDWATER RESOURCES

A Hydrological report prepared for BHC investigating groundwater resources within Pound Ridge was reviewed. 35 commercial/ residential wells were identified as contaminated with varying levels of MTBE. Subsequently, a fracture trace analysis and geological investigation were conducted by the author of the report. The conclusion of the assessments recommended drilling of test wells in two parcels owned by BHC which were identified as stratified drift geologic formations with fracture trace lineaments present in the underlying rock strata. Test wells would be required to determine if the stratified drift formations are thick enough to sustain the average daily water demand or if wells developed in bedrock could sustain the required demand.

The average daily water demand identified in the report was 15,000 gpd (5.5 mgal/year). The desired yield at this time is 60,000 gpd (21.9 mgal/year), which represents an increase of 400%. While the previous investigation stated that the proposed wells would be located far enough away from Scott's Corners and that there would be "no concern of interference or impact from the contaminant plume" it is our opinion that this alternate represents the highest risk solution. Expenditures for drilling, soils analysis, pump testing and water quality analysis are required to determine: if the formations are capable of providing and sustaining the required flows, if the raw water meets required water quality standards and what effect the withdrawal may have on transport of the contaminant plume. There is no assurance that this alternative will meet the Town's needs after completing the tasks required to advance this alternative.

Honorable Joy Simpkins Scotts Corners Potable Water and Wastewater Conceptual Investigation April 11, 2002 Page 6

2.3 WATER SERVICE VIA PROPOSED GOLF COURSE

BHC has indicated that the storage tank being contemplated for the Golf Course will be owned and operated by BHC for its Connecticut customers. Peter Galant, BHC, believed that the proposed tank size is adequate to serve the Scott's Corners Area originally contemplated for service, but could not recall the specific allocation for Scott's Corners at that time. Furthermore BHC stated that regulatory approvals from Connecticut would be required, specifically from the Department of Public Health, Department of Environmental Protection and the Department of Public Utility Control. Representatives from the developer of the proposed Golf Course have indicated that they would be open to discussions regarding this issue. Bridgeport hydraulic was supportive of this alternative.

Following your review of this letter we would like to meet and discuss project with respect to:

- Coordinating potable water supplies and wastewater treatment provisions
- Available wastewater options
- Scheduling a meeting with Town representatives, BHC, WCDOH, NYSDEC, CT DEP, CT DOH and representatives from the proposed golf course to advance selected alternatives. This appears to be the most promising of the water supply options, although the quantity of water which BHC is prepared to furnish is unconfirmed at this time.

Given the WCDOH moratorium on soils testing, lengthy response times from regulators and availability of regulators due to vacations, it is not possible to complete the report by the scheduled date of May 3, 2002. We would like to discuss revisions to the project schedule in light of the Town's needs and availability of information. We believe we can conclude the potable water section of the report following a meeting with the Connecticut and New York Regulatory Authorities. Please contact me at your convenience to determine a suitable time to meet.

Very truly yours,

Robert M. Trzepacz, P.E.

cc:

C. Fowler G. Warshauer K. Taft J. R. Folchetti File

POUND RIDGE WASTEWATER TASK FORCE

Appendix B: Historical Potable Water Reports

Appendix for Potable Water

1973 Dec. 21 Pound Ridge and Stamford Water Company contract to relocate Eastwoods Page 2 road and to construct reservoir, dams and dikes in the vicinity of the present Siscowit Reservoir. Of note it seems that the Town of Pound Ridge has the "right, privilege and priority to draw water from the completed Project" for Town purposes, residential and commercial use.

1997 April 15, Maps associated with BHC providing potable water to Scott's Corners. Page 12

1997 May 12, Feasibility Study for providing Scott's Corners potable water from 3 - 500 foot deep wells on BHC property on Westchester Avenue. A conceptual cost estimate is included that totals \$1.1 million.

1997 June 5 Four conceptual estimates to provide potable water to Scott's Corners, twoPage 22interconnect and two groundwater wells. Costs range from \$800K to \$1.5M. A map of the areas to beserved is included.

1998 Dec. 9, Agreement between BHC and Shell to prepare bid ready documents and an
estimate for water main to Scotts Corners. The permitting process has not been addressed. It also
contains a list of properties to be served.Page 26

1999 Nov. 23: Letter indicating the New Canaan would oppose any street openings that arePage 36required for the BHC Pound Ridge Water Supply Project.Page 36

June 22 1999: Letter from BHC to Keane and Beane regarding an estimate for the work for a pipeline at the cost of \$2.1 million and a ground water source for Pound Ridge from wells at \$500,000. The proposed well location is in a wetland. Permissions and permitting not addressed.

June 14, 1999 Letter from New Canaan selectman to Joy Simpkins, vague denial (of project?). May 4, 1999 Meeting Notes from New Canaan and Pound Ridge representatives resulting in denial of project based upon New Canaan not issuing permits. A water line might cause cause "downzoning" of that area of New Canaan. This superseded discussions about repaving costs which were also discussed. Sept. 8, 1999 Letter from BHC to Joy Simpkins regarding rights of BHC to put pipelines in New Canaan roads and a comment on water rates.

2000 September, Malcom Pirnie study proposal discusses regulatory issues <u>Wastewater Appendix</u> with potable water wells and the possibility of combining them into a Community Water Supply, but wants to start over with a wastewater study.

2002 April, Folchetti study also discusses potable water solutions, <u>Wastewater Appendix</u> water from Stamford, drilling wells and getting water from the golf course.

1973 Dec. 21 Pound Ridge and Stamford Water Company contract to relocate Eastwoods road and to construct reservoir, dams and dikes in the vicinity of the present Siscowit Reservoir. Of note it seems that the Town of Pound Ridge has the "right, privilege and priority to draw water from the completed Project" for Town purposes, residential and commercial use.

AGREEMENT made the 21st day of December 1973 by and between

THE TOWN OF POUND RIDGE, a municipal corporation having its office and place of business at Westchester Avenue (no street number), Pound Ridge, New York

FILE

hereinafter referred to as the Town, and

Construction

STAMFORD WATER COMPANY, a corporation specially chartered by the General Assembly of the State of Connecticut and having offices for the transaction of business in the City of Stamford, County of Fairfield and State of Connecticut,

hereinafter referred to as Water Company:

WITNESSETH:

WHEREAS, Water Company heretofore made application to divert water and to construct a reservoir, dams, dikes and appurtenances in, on or in the vicinity of its present Siscowit Reservoir, also known as Mead's Pond ("the Project") on premises owned by Water Company in the Town, and

WHEREAS, Water Company has heretofore made application to the Town for permission to relocate portions of Eastwoods Road, as part of the Project, as shown on the plans, drawings and specifications filed with the Town, and has requested that the Town approve the proposed relocation of said road in accordance with maps and surveys filed with the Town, and

WHEREAS, the Town did refer the aforesaid application to the Planning Board for consideration, study and recommendations, and did simultaneously refer the aforesaid application to the Water Control Commission for recommendation and report, and the aforesaid reports and recom-

mendations having been received and filed, and the aforesaid Planning Board, Water Control Commission and Town Board having heretofore agreed to the issuance of a single permit for construction and operation of the Project and road relocation subject only to the execution of this agreement, and

WHEREAS, Water Company has obtained approvals from other State and County boards, departments, bureaus and agencies having jurisdiction.

NOW, THEREFORE, in consideration of the mutual covenants and agreements hereinafter set forth, the parties covenant and agree as follows:

1. Water Company shall at its own expense construct the Project or portions thereof, substantially as shown on the aforementioned maps and operate the Project in accordance with all applicable laws, rules and regulations of the Town of Pound Ridge, the County of Westchester, the States of New York and Connecticut and the United States Government, and all departments, boards, bureaus and agencies thereof.

2. Water Company shall pay all fees of the Town in connection with the Project, including building permit fees and reasonable fees of the Town Engineer or inspectors, and shall obtain any additional permits necessary for the construction and completion of the Project.

3. During the course of construction of the Project Water Company shall comply with such reasonable safety precautions and regulations as the Town or any authorized officer or department thereof, including, but not

4 -2-

limited to, the Town Engineer, Town Police Department, Town Building Inspector and Town Fire Marshal may promulgate for the protection of the residents of the Town and other members of the public using or being in the vicinity of the construction project or the existing or proposed roads in the immediate vicinity of such project.

4. The Project shall be constructed to establish the high-water mark in the reservoir at maximum elevation of no more than 485 feet above sea level as shown on the map annexed hereto as Exhibit A.

5. The dam and dike (and any future enlargements thereof acceptable to the Town) shall be constructed in accordance with drawings and specifications to be finally approved by the State of New York and furnished to the Town and annexed hereto as Exhibit B.

6. The portions of Eastwoods Road to be surrendered by the Town and deeded to the Water Company shall be as shown on the survey annexed hereto as Exhibit C.

7. The relocated portions of Eastwoods Road to be located on premises presently owned by Water Company and as shown on the survey annexed hereto as Exhibit D shall be, except as hereinafter provided, deeded in fee simple absolute, free and clear of all encumbrances, (except those acceptable to counsel for the Town), to the Town for dedication as a portion of the public highway system of the Town.

8. The following requirements shall be applicable during the course of construction unless temporarily waived from time to time by the Town Engineer:

-3-

(a) Two-directional vehicular traffic shall be maintained on Eastwoods Road.

(b) Temporary detour routes shall be subject to the approval of the Town Engineer with respect to location and minimum quality of road surface and drainage.

(c) Vehicular right of way shall be maintained by such signs, signal devices or flagmen as may be reasonably required by the Town Engineer, at the expense of the Water Company or its Contractor.

Night lighting and barricading on detour routes and/or roads under construction shall be as reasonably required from time to time by the Town Engineer, at the expense of the Water Company or its Contractor.

(d) The Water Company shall be responsible for road sprinkling and dust control in accordance with reasonable regulations to be issued by the Town Engineer.

(e) A schedule of materials to be hauled away from the reservoir site on Town roads shall be filed with the Town Engineer before any such hauling, such schedules to include the location and approximate quantity of materials to be hauled and the proposed routes to be followed.

(f) Transportation of materials and construction equipment to and from the site shall be via a portion of Eastwoods Road and other roads approved by the Town Engineer but shall not include:

> Old Church Lane north of Old Mill Road; SiscowitRoad outside the construction area;

Eastwoods Road west of Old Church Lane; Hack Green Road; Conant Valley Road; Barnegat Road; Trinity Pass Road.

(g) Excavation and trucking shall be restricted to the hours of 7:00 a.m. to 5:00 p.m. and shall be prohibited on Sundays and legal holidays except for emergencies.

(h) Blasting operations shall be conducted pursuant to permit from the Town Engineer or Building Inspector who shall condition such permit as he may deem necessary for the protection of adjoining structures.

(i) The Town Engineer and Building Inspector shall have the right at all times to enter the premises and inspect conditions to determine possible violations of the provisions of this agreement.

6 -4-

9. Downward slope protection shall be provided along the relocated Eastwoods Road, prior to public use, such protection (except on the dam) to consist of large boulders situate five to six feet apart, with intermediate tree plantings. At the option of the Water Company stone walls may be substituted for boulder and tree protection.

10. Guardrail or barrier protection along the highway on dike or dam or in the immediate vicinity thereof shall be installed as approved by the Town Engineer, and such protection shall meet all Standards of New York State.

11. Minimum flow of 50,000 gallons per day (except during extreme drought conditions) shall be provided via release from dam or dike to stream flowing from western side of Project to Laurel Reservoir via Scotts Corners.

12. The relocated Eastwoods Road shall be constructed in accordance with all applicable road construction regulations as of the date hereof, except as otherwise previously approved and except as modified by width and other limitations in connection with the dam construction.

13. The Town shall, as provided by applicable decisions and permits of the New York State Water Resources Commission, have the right, privilege and priority to draw water from the completed Project (but not before the reservoir is filled) for Town purposes and residential and commercial use and shall pay reasonable charges therefor. In the event that the Town shall exercise its right to draw water after completion of the Project, the Town shall be and hereby is permitted to construct a suitable pump house on . Water Company property, and all required appurtenant apparatus, water pipes and electrical lines as may be required all

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at the sole expense of the Town and subject to reasonable written approval of the Water Company prior to construction. For that limited purpose the Town shall be deemed to have a perpetual easement over Water Company property within and on which to locate the pump house, pipes and other equipment; provided, however, that Water Company shall concur in the location of any pump house, pumping equipment, pipe or electric lines and related facilities and the required easement area.

14. Upon completion of the Project one standard dry hydrant shall be installed at such elevation and location and of such type and having such fittings as may be directed by the Fire Chief of the Town Fire District at Water Company's expense and shall become the property of the Town Fire District. If such dry hydrant shall be situate on Water Company property, the Town, for fire fighting purposes, shall be deemed to have an easement over a strip of land sufficient in width for access from the nearest highway to the dry hydrant by fire fighting equipment. Maintenance of the hydrant and of the access to it from the nearest public highway shall, after installation of the dry hydrant, be the responsibility of and at the expense of the Town Fire District.

15. Water Company shall save the Town harmless from the claims of any person resulting from damage to person or property in connection with any and all construction work on the Project, and attributable to negligence on the part of the Water Company, and from all expenses resulting therefrom (including reasonable attorneys' fees) incurred by the Town in resisting any claim against the Town. In

8 -6-

furtherance of the foregoing, Water Company or its contractors or agents when construction of the Project starts shall procure and maintain, or cause to be procured and maintained, public liability insurance in the limit set by the Water Company for the liability insurance to be carried by its contractor in connection with the aforementioned construction work, such insurance to name the Town as an additional named insured. Water Company or its contractors or agents shall furnish the Town with a certificate of such insurance, which certificate shall provide that the policy for such insurance shall not, while the Project is under construction, be cancelled without at least ten days prior notice to the Town or amended so as to adversely affect the interest of the Town

16. When the relocated portions of Eastwoods Road are satisfactorily completed the Town will, upon inspection of the relocated portions of said road by its appropriate officers and inspectors and certification by such officers and inspectors of the condition thereof, accept the same as a public highway (except for that portion on the dam which will be accepted as a perpetual easement for highway purposes) upon the delivery to it of a bargain and sale deed of the same (as a highway in part and an easement in part) with covenants against grantor's acts, in proper statutory form for recording in New York and with any required documentary stamps) affixed thereto at Water Company's expense, together with a policy of title insurance issued by a member company of the New York Board of Title Underwriters insuring to the Town in the sum of ten (10) thousand dollars that title to the same is vested in the Town free and clear of all liens and encumbrances, and free and clear of all mechanic's and

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-7-

similar liens. Water Company shall pay the charges of recording such deed and the premium on such policy of title insurance.

17. The aforementioned deed shall contain such utility and drainage easements over adjoining property of Water Company in the immediate vicinity of Eastwoods Road as the Planning Board of the Town shall require.

18. Simultaneously with the delivery of the aforementioned deed, the Town shall convey to Water Company by quit claim deed those portions of Eastwoods Road which have been relocated by Water Company and which are no longer needed by the Town as part of its public highway system. Water Company shall pay for all documentary stamps and other charges and taxes, if any, in connection with the delivery of such deed.

19. Water Company agrees that it or its contractors or agents shall, prior to relocation of Eastwoods Road, file a performance bond to assure the satisfactory completion of the relocated road. Said bond shall be approved as to form and surety by the attorney for the Town Board of the Town of Pound Ridge. The said bond shall continue in full force and effect until the road has been satisfactorily completed and accepted by the Town of Pound Ridge. The bond shall be in the amount of \$500,000.00.

20. The deed to be delivered to the Town shall include fee title to all relocated portions of Eastwoods Road, except that portion on the dam as to which it shall include a perpetual easement for highway purposes as to the entirety of the width of the same and title to the guardrails. It shall be the obligation of the Town to plow and to maintain, repair and repave the portion of the road

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on the dam in the same manner as any other public highway within said town. Upon the installation of guardrails or barriers along the roadway on the dam (as hereinabove provided), the obligation to maintain, repair and replace same shall be that of the Town. Except for the foregoing obligations of maintenance and repair the Town shall have no obligation to undertake any repairs whatsoever to the dam. All necessary repairs, earth replacement, sealing or grading of the dam and all repairs to valves and continuous flow equipment shall be the responsibility of Water Company. Repair of dam by Water Company shall be conducted in such manner that traffic will be permitted to use the road over the dam during repair and maintenance work to the fullest extent possible. The Town agrees that it will, at the request of Water Company, erect signs on or in the vicinity of the dam prohibiting parking or standing of vehicles.

21. The Water Company agrees, within twelve months from the date hereof, and in any event prior to the release of any performance bonds as provided for herein, to eliminate the erosion problem at the spillway of the Mill River dam.

IN WITNESS WHEREOF, the parties hereto have signed this agreement the day and year first above written.

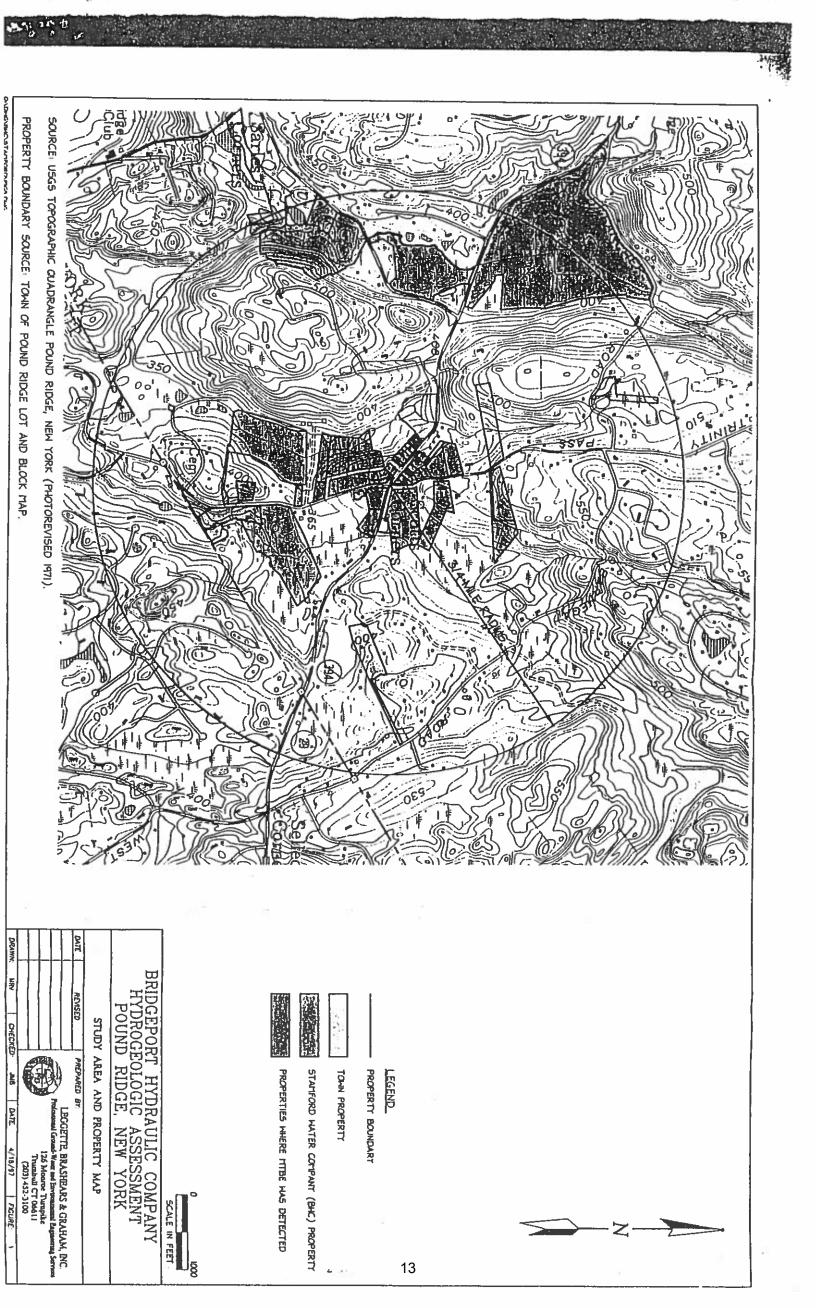
STAMFORD WATER COMPANY By <u>L'Sauford Rais</u> Its President

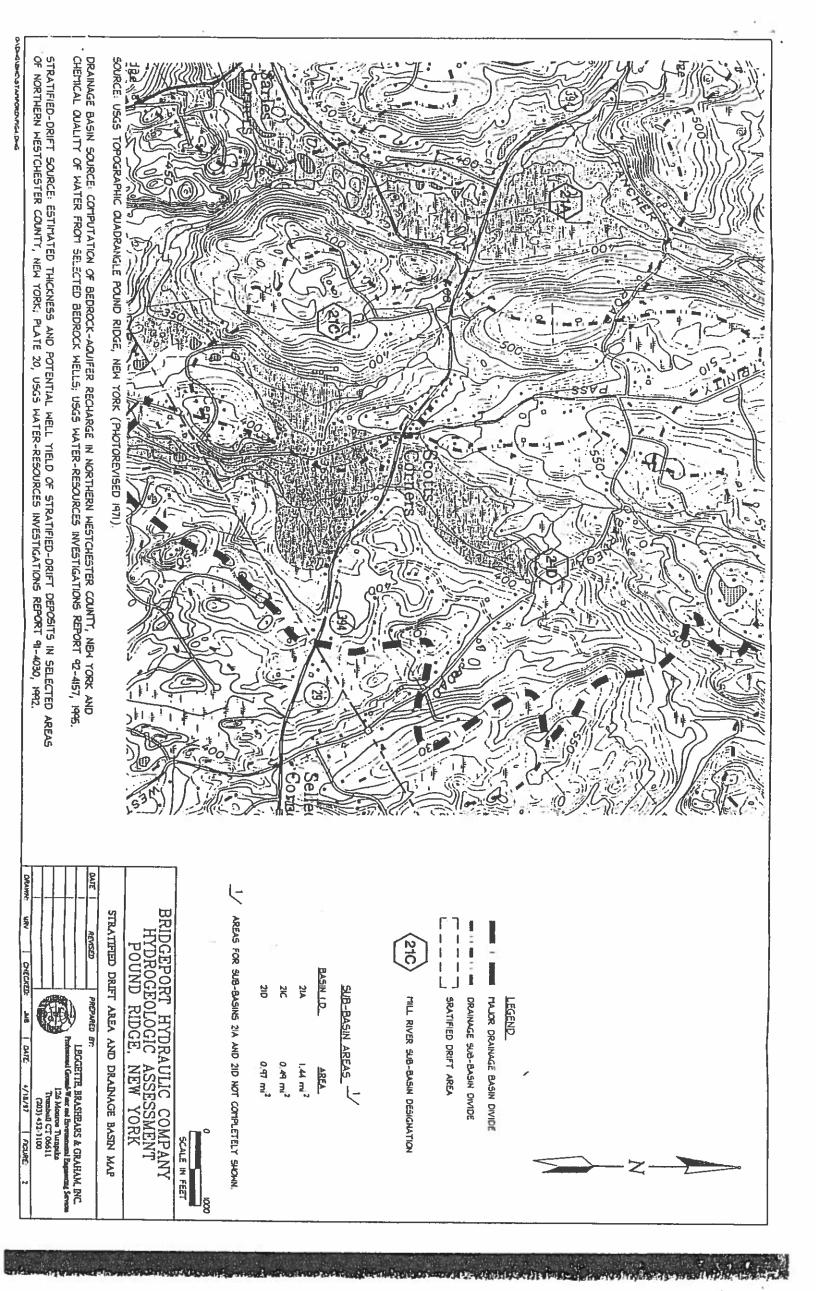
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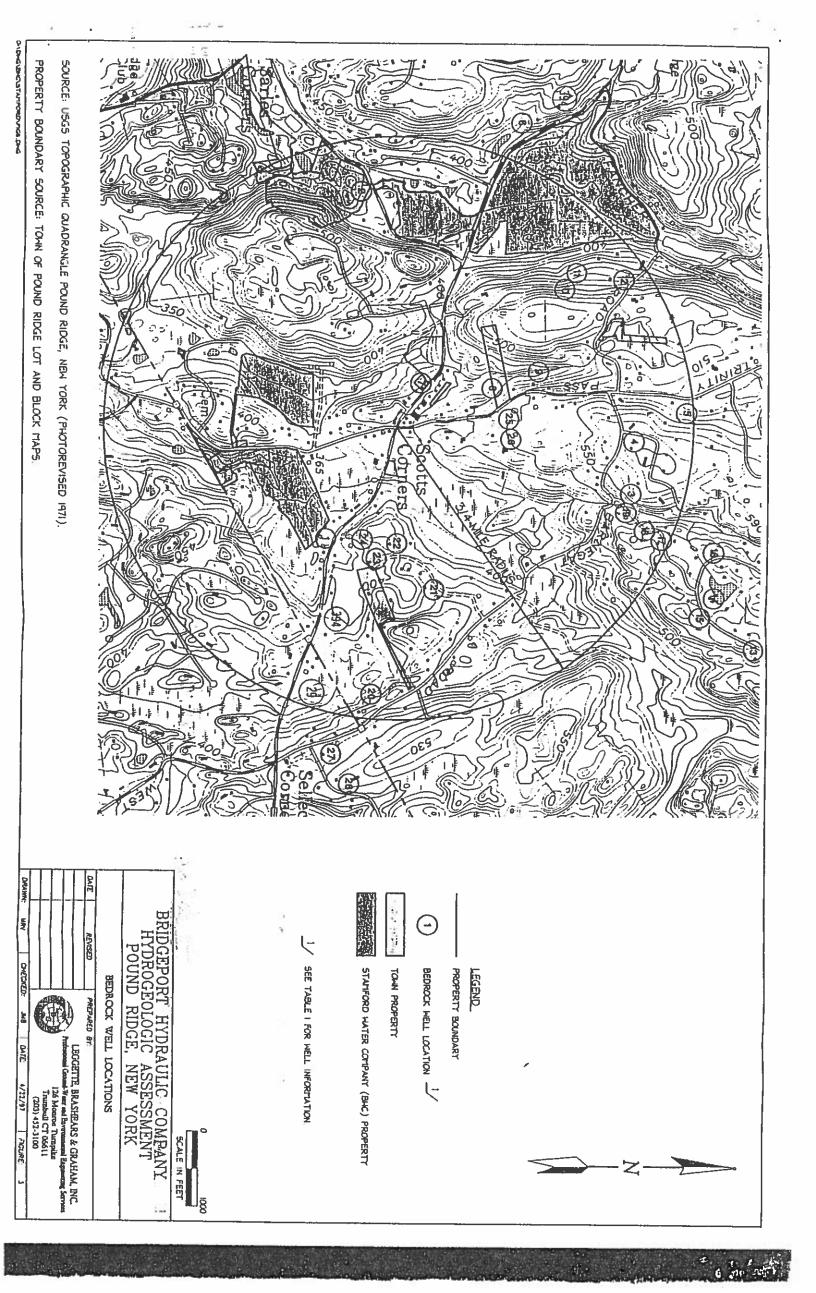
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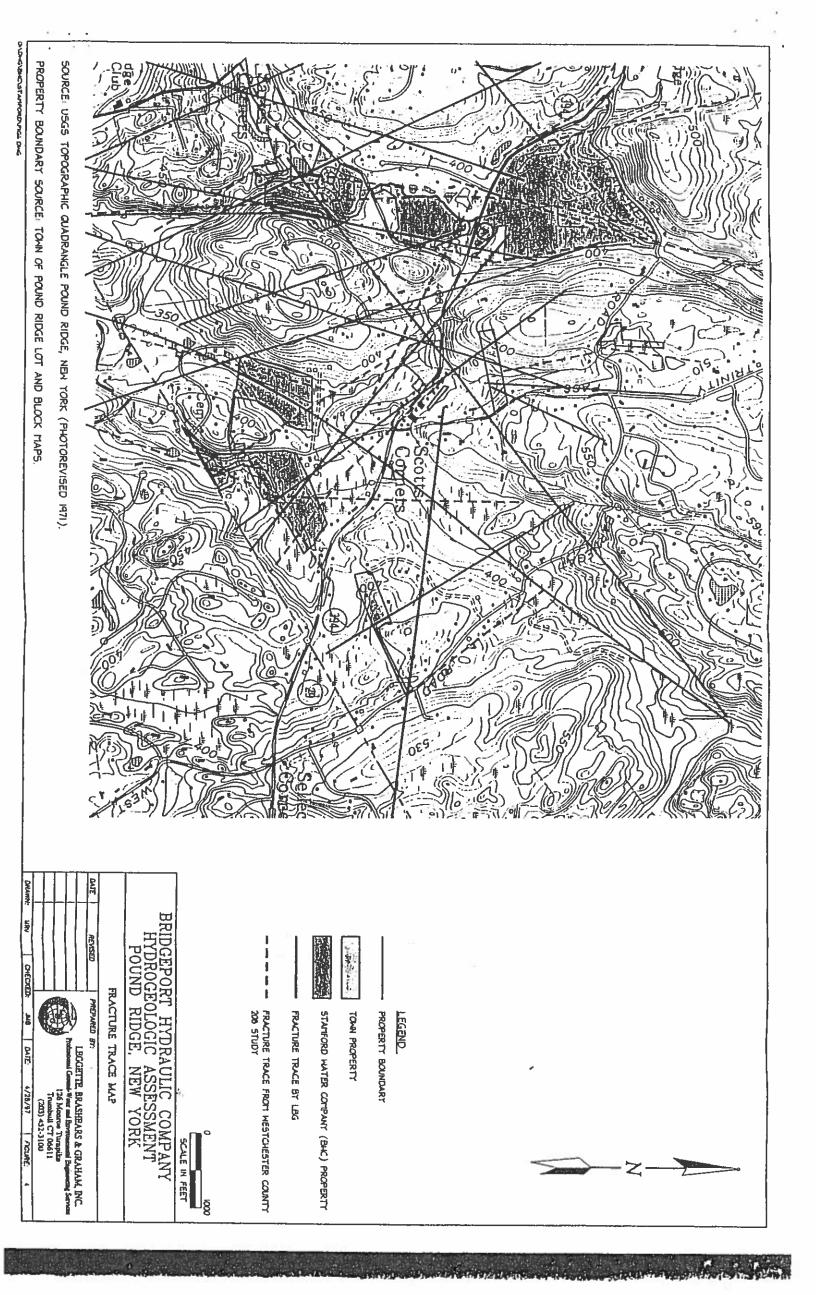
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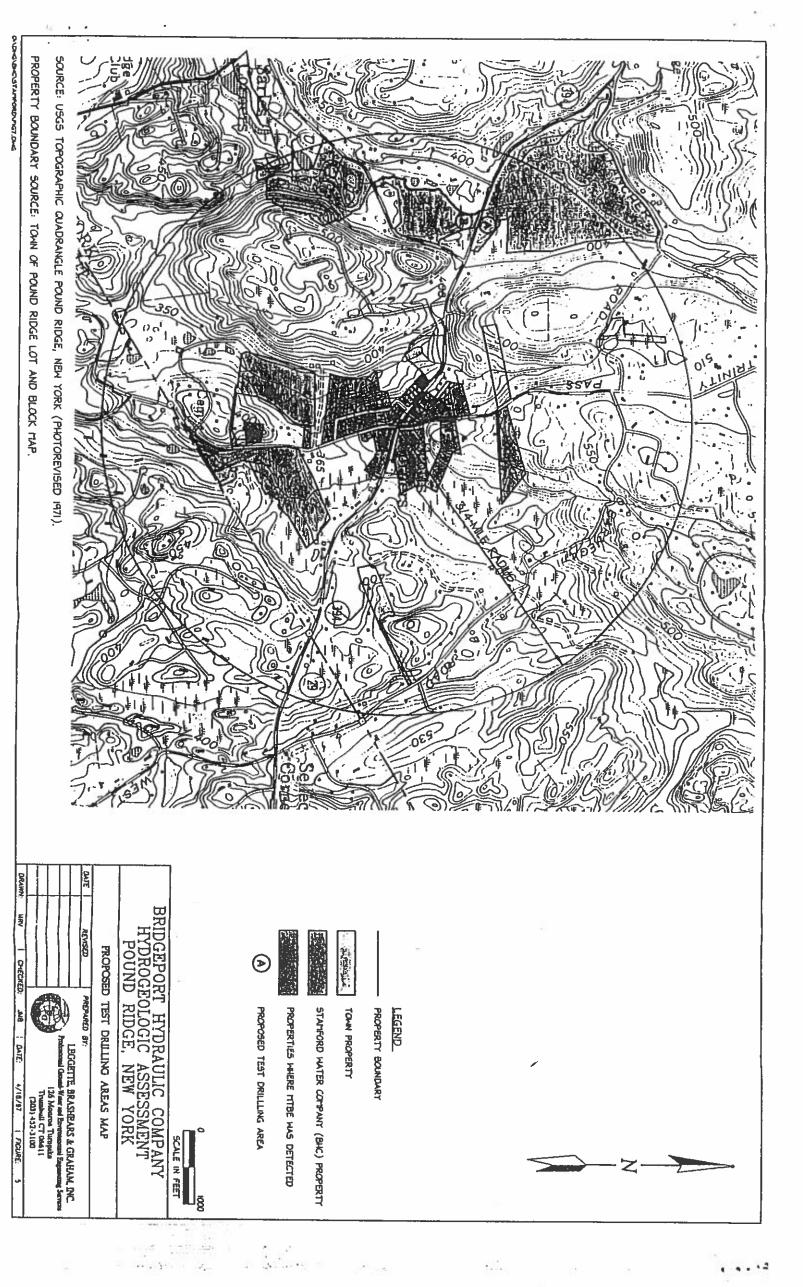
1997 April 15, Maps associated with BHC providing potable water to Scott's Corners.



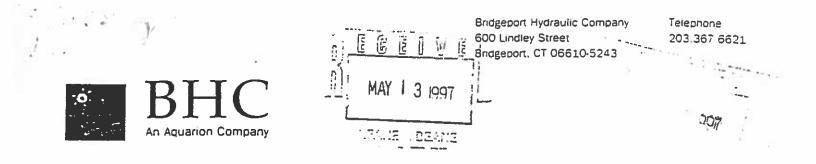








1997 May 12, Feasibility Study for providing Scott's Corners potable water from 3 500 foot deep wells on BHC property on Westchester Avenue. A conceptual cost estimate is included that totals \$1.1 million.



May 12, 1997

Mr. David McNeil Environmental Engineer Shell Oil Products Company 30 Jericho Executive Plaza Suite 500 West Jericho, NY 11753

Re: Feasibility Study for Groundwater Supply - Pound Ridge, NY

Dear Mr. McNeil:

Enclosed is a draft report prepared for BHC by Leggette, Brashears & Graham, Inc. (LBG) entitled "Hydrogeologic Assessment For the Area of Scott's Corners - Town of Pound Ridge, New York". The report presents the results of an analysis of property owned by the Town and BHC for potential development of a ground water supply to serve the area of Pound Ridge affected by MTBE contamination. It concludes that the best site for additional investigation is on property owned by BHC along Westchester Avenue, approximately 3,100 feet from Scott's Corners. Although other parcels with similar yield potential were identified, they were ruled out due to their proximity to lots that have contaminated wells. If it is necessary to pursue these sites further, the feasibility of central treatment could be investigated.

A conceptual cost estimate for providing a central water supply system to the Scott's Corners area from the proposed well site is summarized in the attached Table. This system would provide drinking water to all properties where MTBE has been detected, as identified on Figure 1 of the enclosed draft report. It should be noted that the extent of this system is greater than that previously estimated by BHC for interconnection to the Stamford system. A discussion of the specific properties to be served by the proposed water system would be helpful before finalizing the report. Perhaps this can be done in conjunction with reviewing Shell's comments.

If, after finalizing this report, Shell is interested in pursuing a ground water supply for Scott's Corners the recommended next step would be to finalize the location of, install, and test the water quality and production capacity of the necessary wells. As summarized in the attached table, three wells, with a maximum depth of 500 feet, can be installed, tested and permitted for approximately \$75,000. The final cost would vary depending on number and depth of wells, sitework necessary to access wells and unforeseen permitting issues. A report summarizing well production capacities and water quality characteristics could be provided before final permitting.

Page 2 Mr. David McNeil May 12, 1997

If you have any questions regarding the report, or would like to discuss next steps, please feel free to call me at (203) 337-5903.

Sincerely, 1 J

Peter B. Galant, P.E. Director of Engineering

- cc: J. Suttile
 - G. Thornhill
 - R. O'Rourke, Esq.

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- B. Aurelius, Esq.
- B. Conlon, Esq.

FEASIBILITY STUDY FOR GROUND WATER SUPPLY SCOTT'S CORNERS - POUND RIDGE, NEW YORK <u>CONCEPTUAL COST ESTIMATE</u>

DESCRIPTION	ESTIMATE
Install Wells (3 wells @ 500 ft. max.)	\$15,000
Test, Permit and Complete Wells	\$60,000
Well & Treatment Structures and Associated Sitework	\$200,000
Pipeline - Westchester Ave Well site to Trinity Pass (3,100 ft)	\$320,000
Pipeline - Westchester Ave East of Trinity Pass (500 ft)	\$47,500
Pipeline - Trinity Pass - South of Westchester Ave. (2,000 ft)	\$150,000
Pipeline - Trinity Pass - North of Westchester Ave. (1,500 ft)	\$142,500
Pipeline - Trinity Lane (950 ft)	\$67,000
Service Connections and Lines to House (35)	\$130,000
TOTAL:	\$1,132,000

Notes:

- 1. All estimates are considered reconnaissance grade (± 30%)
- 2. Scope of work based on draft report "Hydrogeologic Assessment For the Area of Scott's Corners Town of Pound Ridge, NY". May 1997.
- 3. Service connections exclude interior plumbing and well abandonment.
- 4. Treatment includes chlorination only.
- 5. Fire protection not included.
- 6. Legal fees for establishing a water company pursuant to NY State Transportation Corporation Law and NY Public Service Commission requirements are excluded.

1997 June 5 Four conceptual estimates to provide potable water to Scott's Corners, two interconnect and two groundwater wells. Costs range from \$800K to \$1.5M. A map of the areas to be served is included.

10102131	INU II:40 FAX 203 337 5839	BBC ENG DEPT
. •:		BHC Company 600 Lindley Street Bridgeport, CT 06610-5243

Telephone 203.337-5910

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Facsimile

To	David McNeil - Shell Oil	<u>From</u>	Peter Galant	
	Cesare Manfredi - DEC	Dete	June 4, 1997	
	<u> </u>	Time		
Fex No.		Telephone	(203) 337-5903	<u></u>
Number	of pages including this sheet3	Fax No.	(203) 337-5839	

The following is the additional cost information which you requested. Cost estimates are provided for the following four scenarios:

- Ground water supply with distribution system to serve the 35 properties described on Figure 1 of the draft report "Hydrogeologic Assessment For the Area of Scotts Corners - Town of Pound Ridge, NY" May 1997 as having MTBE detected (properties shown in green).
- Interconnection to Stamford and distribution system and service lines to 35 homes described above. Note that limited pressure would be available to homes at elevation greater than approximately 450 ft. and that booster pumps may be required.
- 3. Ground water supply with distribution system to serve the 13 properties designated as moderate and high level MTBE concentrations on the following map (source unknown).
- 4. Interconnection to Stamford and distribution system and service lines to 13 properties described above.

I hope that this additional information is helpful in making an "apples to apples" comparison of the ground water and interconnection supply alternatives. As requested by Shell, I will prepare an order of magnitude estimate of the time frame to implement these two alternatives.

If you have any additional questions, please feel free to call me.

G. Thornhill J. Suttile R. O'Rourke

cc:

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SCOTT'S CORNERS WATER SUPPLY SYSTEM CONCEPTUAL COST ESTIMATES

Scenario 1: GROUNDWATER SUPPLY (Detected MTBE Homes)

DESCRIPTION	LENGTH	ESTIMATE
Install 3 New Wells		\$15,000
Test, Permit and Install Pumps in Above Wells		\$60,000
Well & Treatment Structures, sitework, hydro., etc.		\$200,000
Westchester Ave Well Site to Trinky Pass	3,100	\$320,000
Westchester Ave East of Trinity Pass	500	\$47,500
Trinity Pass - South of Westchester Ave.	2.000	\$150,000
Trinity Pass - North of Westchester Ave.	1,500	\$142,500
Trinity Lane	950	\$67,000
Service Connections (35)		\$130,000
Service Connectoria (00)	TOTAL	\$1,132,000

Scenario 2: INTERCONNECTION (Detected MTBE Homes)

DESCRIPTION		ESTIMATE
Laurel Rd. to Ponus St. to Trinity Pass to Westch. Ave.	11,100	\$1,050,000
Trinity Lane	950	\$87,000
Trinity Pass - North of Westchester Ave.	1,500	\$142,500
Westchester Ave East of Trinity Pass	500	\$47,500
Westchester Ave West of Trinkty Pass to Fire House	720	\$85,000
Services (35)		\$130,000
	TOTAL	\$1,522,000

Scenario 3: GROUNDWATER SUPPLY (Moderate/High_MTBE Homes)

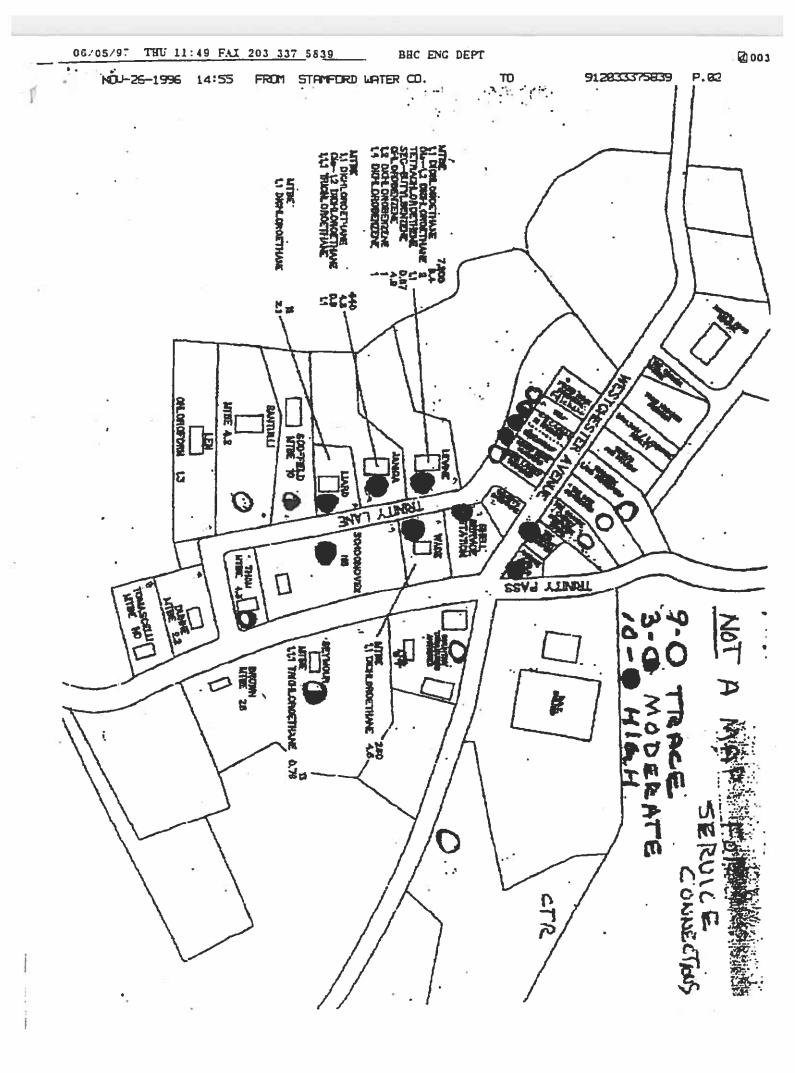
DESCRIPTION	LENGTH	ESTIMATE
Install 3 New Wells		\$15,000
Test, Permit and Install Pumps in Above Wells		\$60,000
Well & Treatment Structures, sitework, hydro., etc.		\$200,000
Westchester Ave Well Site to Trinity Pass	3,100	\$320,000
Trinity Pass - South of Westchester Ave.	1,200	\$90,000
Trinity Lane	950	\$67,000
Service Connections (13)	10.15	\$50,000
	TOTAL	\$802,000

Scenario 4: INTERCONNECTION (Moderate/High MTBE Homes)

DESCRIPTION	LENGTH ESTIMATE
Laurel Rd. to Ponus St. to Trinity Pass to Westch. Ave.	11,100 \$1,050,000
Trinity Lane	950 \$67,000
Westchester Ave West of Trinity Pass to Fire House	720 \$85,000
Services (13)	\$50,000
	TOTAL: \$1,252,000

SUMMARY	
DESCRIPTION	Estimate
Scanario 1: GROUNDWATER SUPPLY (Detected MTBE Homes)	\$1,132,000
Scenario 2: INTERCONNECTION (Detected MTBE Homes)	\$1,522,000
Scenario 3: GROUNDWATER SUPPLY (Moderate/High MTBE Homes)	\$802,000
Scenario 4: INTERCONNECTION (Moderate/High MTBE Homes)	\$1,252,000

See cost notes from 5/12/97 letter to Shell Oil



1998 Dec. 9, Agreement between BHC and Shell to prepare bid ready documents and an estimate for water main to Scotts Corners. The permitting process has not been addressed. It also contains a list of properties to be served.

- Burgard

EDWARD F. BEANE DAVID GLASSER RONALD A. LONGO RICHARD L. O'ROURKE LAWRENCE PRAGA JOEL H. SACHS' STEVEN A. SCHURKMAN' JUDSON K. SIEBERT

"ALSO ADMITTED IN FL "ALSO ADMITTED IN NJ "ALSO ADMITTED IN MA GALSO ADMITTED IN CT TALSO ADMITTED IN DC 4 CA {9|4}946-4777 TELEFAX (9|4}946-6868

WHITE PLAINS, NEW YORK 10601

KEANE & BEANE, P.C.

December 9, 1998

THOMAS F. KEANE, JR. (1921-991)

STEPHANIE L. BURNS" JOSEPH A. DETRAGLIA⁰ FREDERIC B. EISMAN^G DONNA E. FROSCO"⁺T LANCE H. KLEIN" PATRICK J. O'SULLIVAN FRANCES M. PANTALEO NICHOLAS M. WARD-WILLIS"^{-G}

OF COUNSEL PETER A. BORROK" JOHN F. BURKHARDT ERIC F. JENSEN^D

VIA UPS OVERNIGHT

Mark Weyman, Esq. Anderson Kill & Olick, P.C. 1251 Avenue of the Americas New York, New York 10020-1182

Re: BHC Company, Town of Pound Ridge Water Supply

Dear Mark:

I am pleased to enclose an executed copy of the Agreement between BHC and Shell Oil to prepare bid-ready documents and a cost estimate to install a water main to serve the Scots Corners section of Pound Ridge. I also enclose a copy of the transmittal letter from Peter B. Galant, P.E., BHC Company Director of Engineering.

Thank you for your consideration.

Very truly yours,

ROR/mq

Enclosure (s)

cc: Peter B. Galant, P.E.
BHC Company;
Hon. Joy Simpkins (w/o encl.);
Caesar Manfredi, P.E. (w/o encl.);
Timothy Eidle, Esq. (w/o encl.);
James J. Sullivan, Esq.
Town Attorney, Town of Pound Ridge

KEANE & BEANE, P. C. Mark Weyman, Esq. December 9, 1998 Page 2

Addresses for cc's:

Peter Galant, P.E. Director of Engineering BHC Company 600 Lindley Street Bridgeport, Connecticut 06610-5243

Hon. Joy Simpkins Supervisor Town of Pound Ridge Town House 179 Westchester Avenue Pound Ridge, New York 10576-1743

Ceasar Manfredi, Esq. Water Quality Unit NYSDEC, Region 3 200 White Plains Road Tarrytown, New York 10591-5805

Timothy Eidle, Esq. NYSDEC New York State Department of Environmental Conservation Division of Legal Affairs 50 Wolf Road, Room 638 Albany, New York 12233-1500

James J. Sullivan, Esq. Town Attorney Town of Pound Ridge Town House Westchester Avenue Pound Ridge, New York 10579

BHC Company 600 Lindley Street Bridgeport, CT 06606-5044



Mr. Richard L. O'Rourke, Esq. Keane & Beane, P.C. One North Broadway, Suite 700 White Plains, NY 10601

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December 2, 1998

Re: Scott's Corners Water Supply

Dear Rick:

Enclosed are two executed copies of the agreement between BHC and Shell Oil to prepare bid ready documents and a cost estimate to install water mains to serve the Scott's Corners section of Pound Ridge.

It is my understanding that the bid documents and cost estimate that we are to prepare will be utilized by the parties involved to make a go/no go decision regarding the provision of public water supply to this area. As discussed at our meetings, there is a considerable amount of work to be done after that decision is made before construction of the system can begin, including:

- Creation of a New York subsidiary to Aquarion Company to own and operate the system
- Satisfactory agreement between the New York State Department of Environmental Conservation (DEC) and the Aquarion subsidiary to proceed with construction
- Obtaining all necessary approvals in New York including; DEC, Westchester County Health Dept, NY State Dept. of Health and the New York State Public Services Commission
- Obtaining all necessary approvals in Connecticut including; Department of Public Utility Control, Department of Public Health design approval and sale of excess water approval, potential Department of Environmental Protection diversion and stream crossing permits.

BHC will work to identify these permit requirements during the project design, but will not apply for any permits until the project is authorized. The time period to receive these permits is generally out of BHC's control and is difficult to estimate at this time. However, everyone should realize that the permit timeframe may be significant (greater than 1 year). While there is much to be accomplished, I don't foresee any insurmountable obstacles at this time. Page 2 Mr. Richard L. O'Rourke, Esq. November 16, 1998

Please forward the enclosed contracts to Shell Oil, as necessary. I am looking forward to successful completion of this challenging project.

Sincerely:

Peter B. Galant, P.E. Director of Engineering

Enclosures

cc:

Hon. Joy Simkins – Town of Pound Ridge (w/o enclosure)
 Cesare Manfredi, PE – NYDEC (w/o enclosure)
 Timothy Eidle, Esq. – NYDEC (w/o enclosure)
 Mark Weyman, Esq. – Anderson Kill & Olick (w/o enclosure)
 James Sullivan, Esq. – Town of Pound Ridge (w/o enclosure)

AGREEMENT

This Agreement is made this $\underline{33}$ day of November, 1998 between Shell Oil Company ("Shell") and BHC Company ("BHC").

WHEREAS, the New York State Department of Environmental Conservation ("DEC") is considering the installation of a water system to serve certain properties located in the Town of Pound Ridge, and

WHEREAS, it is desirable to prepare "bid ready documents" in the event that the DEC determines to proceed with the installation of such a water system, and

WHEREAS, the parties hereto are willing to assist the DEC to expedite its considerations by this Agreement to prepare such bid ready documents,

NOW, THEREFORE, it is hereby agreed as follows:

 BHC shall perform all services necessary to prepare bid ready documents for a construction of the water system including:

- a. 1 inch equals 40 foot survey of the pipe line route with 2 foot contours;
- drilling test holes at approximately 100 foot intervals to determine the depth to bedrock;
- c. inspect service locations to determine size, routing and location of meters;
- d. determine expected pressures at each service connection;
- coordinate with local and state authorities regarding permits and pavement requirements for construction;
- f. prepare plans, specifications and bid documents necessary to obtain bids to furnish all necessary labor, material and equipment

31

to construct the proposed water system; and

- g. provide two sets of final documents for review and comment.
- 2. The water system for which the bid ready documents

will be prepared will include:

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- a. approximately 11,100 feet of pipe line along Laurel Road to Ponus Street to Trinity Pass to Westchester Avenue;
- b. approximately 950 feet of pipe line in Trinity Lane;
- c. approximately 1,500 feet of pipe line in Trinity Pass, north of Westchester Avenue;
- approximately 500 feet of pipe line in Westchester Avenue east of Trinity Pass;
- e. approximately 720 feet of pipe line in Westchester Avenue west of Trinity Pass;
- f. a meter pit at the New Canaan/Pound Ridge line to meter water sales from BHC Company to the new water company to be formed in Pound Ridge;
- g. installation of the remote reading water meters; and
- h. installation of service line from main to curb valve located at property line.
- 3. The water system is to be connected to certain

service connections. The DEC is considering whether connections should be made for the properties lised on Exhibit "A" hereto. It is specifically understood and agreed that the list annexed as Exhibit "A" is non-final and non-binding. The inclusion of any property on Exhibit "A" should not be viewed as an indication that such property will be connected to the water system.

4. The bid ready documents shall be delivered for review and comment as follows:

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For New York State Department of Environmental Conservation Timothy Eidle, Esq. New York State Department of Environmental Conservation 50 Wolf Road Albany, New York 12233

For Shell Oil Company Mark L. Weyman, Esq. Anderson Kill & Olick, P.C. 1251 Avenue of the Americas New York, New York 10020

5. BHC shall arrange for a professional cost estimator to provide a conceptual/preliminary estimate for the cost to construct the water system which is the subject of the bid ready documents.

6. Shell agrees to pay BHC for the work required by

this Agreement as follows:

- a. BHC shall be paid at its usual and customary rates for its services and expenses, up to a maximum of \$32,000.
- b. In addition, BHC shall be reimbursed for the costs incurred for a professional cost estimator to provide a cost estimate for the proposed water system, up to a maximum of \$5,000.
- c. BHC shall furnish to Shell and the DEC reasonable documentation for its services, expenses and costs.
- d. BHC shall be paid for its services, expenses and costs within thirty (30) days of the date proper documentation therefor has been delivered, but in no event sooner than thirty (30) days of BHC's delivery of bid ready documents to Shell and the DEC.

7. BHC shall complete the work required by this Agreement (including providing a cost estimate) within 120 days of the execution of this Agreement by Shell which 120 days shall not include the period of review and comment set forth in

-3-

NY2-154490.3

paragraph 4 of this Agreement. The cost estimate shall be provided as soon as it is available but in no event later than 120 days from the execution of this Agreement.

BHC Company

By: Firletle Name : Charles V. Title: Senial Vice Plevident & COO

Shell Oil Company By: Stephen R. Shaw

General Manager - Retail NY/NJ Region

NY2-154490.3

POUND RIDGE SITES UNDER CONSIDERATION FOR CONNECTION TO PUBLIC WATER

Westchester Avenue

54 Westchester Avenue 56 Westchester Avenue 57 Westchester Avenue 60 Westchester Avenue 65 Westchester Avenue 66 Westchester Avenue 67 Westchester Avenue 68 Westchester Avenue 69 Westchester Avenue 70 Westchester Avenue 71 Westchester Avenue 72 Westchester Avenue 73 Westchester Avenue 74 Westchester Avenue 76 Westchester Avenue 77 Westchester Avenue 78 Westchester Avenue 79 Westchester Avenue 80 Westchester Avenue 81 Westchester Avenue 83 Westchester Avenue 85 Westchester Avenue 87 Westchester Avenue 89 Westchester Avenue

Trinity Lane 8 Trinity Lane 10 Trinity Lane 14 Trinity Lane 18 Trinity Lane 22 Trinity Lane 25 Trinity Lane 26 Trinity Lane

- Lower Trinity Pass 6 Lower Trinity Pass 8 Lower Trinity Pass 10 Lower Trinity Pass 15 Lower Trinity Pass 17 Lower Trinity Pass 23 Lower Trinity Pass 24 Lower Trinity Pass 26 Lower Trinity Pass 27 Lower Trinity Pass 35 Lower Trinity Pass 37 Lower Trinity Pass
 - 7 Trinity Pass 10-12 Trinity Pass 15 Trinity Pass 16 Trinity Pass 17 Trinity Pass 21 Trinity Pass 23 Trinity Pass 25 Trinity Pass 26 Trinity Pass 27 Trinity Pass 29 Trinity Pass 30 Trinity Pass

Trinity Pass

4 Trinity Pass

31 Trinity Pass

NY1-245844

1999 Nov. 23: Letter indicating the New Canaan would oppose any street openings that are required for the BHC Pound Ridge Water Supply Project.

June 22 1999: Letter from BHC to Keane and Beane regarding an estimate for the work for a pipeline at the cost of \$2.1 million and a ground water source for Pound Ridge from wells at \$500,000. The proposed well location is in a wetland. Permissions and permitting not addressed.

June 14, 1999 Letter from New Canaan selectman to Joy Simpkins, vague denial (of project?). May 4, 1999 Meeting Notes from New Canaan and Pound Ridge representatives resulting in denial of project based upon New Canaan not issuing permits. A water line might cause cause "downzoning" of that area of New Canaan. This superseded discussions about repaving costs which were also discussed. Sept. 8, 1999 Letter from BHC to Joy Simpkins regarding rights of BHC to put pipelines in New Canaan roads and a comment on water rates. V. 29. 1999 S: DIFT REFAIL & DENNER F.C. PLC LEVAL ATTAINS

BHC Company 600 Undley Street Bridgeport, CT 06606-5044 Telephone 203.367 8621



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RECEIVEN

NOV 3 0 1999

November 23, 1999

Richard L. O'Rourke, Bsq. Keane & Beane, P.C. One North Broadway White Plains, NY 10601

Re: BHC Company - Pound Ridge Water Supply

Dear Rick:

This letter is written in response to Shell Oil's request to proceed with bidding the proposed pipeline project in order to select a contractor and apply for the street opening permit required from the Town of New Canaan. The Town of New Canaan has made clear not only its opposition to the proposed pipeline in Ponus Ridge Road but its intent to deny any permit application for the pipeline construction. BFIC does not, therefore, see any reason to spend the time, money and effort necessary to proceed with the proposed project.

Please call me if you have any additional questions regarding this matter.

Sincerely,

Poter B. Galant, P.E. Director of Engineering

cc: R. Bond - Town of New Canaan C. Firlotte - BHC



600 Lindley Street Bridgeport, CT 06606-5044 lelephone 203.367 6621



Mr. Richard L. O'Rourke Keane & Beane, P.C. One North Broadway White Plains, NY 10601

June 22, 1999

Re: BHC Company - Pound Ridge Water Supply

Dear Rick:

The following information is provided in response to questions asked at the last Pound Ridge Water Supply meeting and Caesar Manfredi's email:

- The current project cost estimate can be reduced by approximately \$160,000 if the pavement overlay is removed from the New York portion of the project. BHC would still provide 3 " trench paving that could remain as a base course for the final overlay.
- The current project cost estimate can be reduced by approximately \$115,000 if Upper Trinity Pass is deleted from the project and the NY pavement overlay remains in the project.
- Based on the above estimates, the current project cost estimate can be reduced by approximately \$240,000 if Upper Trinity Pass and the NY overlay are removed from the project.
- Bill Brennan, of J.J. Brennan Construction has indicated that his firm would construct this pipeline under BHC's annual bid contract for a not-to-exceed price of \$1,970,000 (total project approximately \$2.1 million including BHC costs). This approach assumes that only trench repair will be required in New York and that JJB will take the risk of finding the remaining savings elsewhere on the project. While I understand that NYDEC policy may not allow this type of contracting, I think the offer provides a better sense of what the ultimate project cost may be.
- The attached memorandum from Leggette, Brashears and Graham (LBG) updates the cost estimate for developing a ground water supply source in Pound Ridge to approximately \$100,000. Note that this approach does not guarantee a quantity, or quality, of water. In addition to the well costs, a treatment building/pump station would be required at the well site. Based upon recent experience, a conceptual cost estimate for this type of facility (w/o MTBE treatment) is approximately \$400,000. Utilizing J.J. Brennan's pipeline estimate of approximately \$138/ft including/paving, and approximately 8,000 ft. of piping, the piping

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Mr. Richard L. O'Rourke June 21, 1999 Page 2

> construction cost estimate is \$1.1 million plus approximately \$85,000 for taps and services to the property line. The total project cost estimate, including BHC costs, is therefore approximately \$1.8 million. Please keep in mind that these costs, particularly for the treatment building, are conceptual only and may vary significantly based upon preliminary and final design. In particular, the site identified by LBG for locating the wellfield is shown as wetlands on the USGS map. The feasibility and cost implications of constructing in a wetland have not been investigated, nor have the resulting permitting requirements.

I hope this answers the cost questions raised at and after our last meeting. After your review, please distribute this information prior to our June 22 meeting.

Sincerely,

Peter B. Galant, P.E. Director of Engineering

cc: B. Brennan – JJB R. Furano - BHC

TOWN OF NEW CANAAN TOWN HALL, 77 MAIN STREET NEW CANAAN, CT 06840

RICHARD P. BOND FIRST SELECTMAN TEL: (203) 972-2311 FAX: (203) 966-0309

June 14, 1999

Ms. Joy G. Simpkins, Supervisor Town of Pound Ridge Town House 179 Westchester Avenue Pound Ridge, New York 10576-1743 SUPERVISOR'S OFFICE POUND RIDGE, MY 99 JUN 16 AH11: 58

Dear Ms. Simpkins:

I apologize for responding so late to your note of May 14, 1999. I'm sorry that we could not be of any help. We are continually faced with this type of problem. This coming week, we have Level 3 Communications coming to visit us about running a fiber optic cable from the Stamford end of town to the opposite side of town. Again, they want to tear up the roads to do this.

It would be interesting to know the results of your "contaminate water group's" meeting.

Sincerely,

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Richard P. Bond First Selectman

RPB:dh

TO:	JOY		<u> </u>
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MEETING NOTES

May 4,, 1999 New Canaan Town Hall

3."

Re: Bridgeport Hydraulics Contract Documents For The Pound Ridge Interconnection

Present: Richard Bond, First Selectman; Frank DeNicola, Head of Public Works Department; Hiram Peck, Head, Department of Zoning and Planning;

> Mort Miller, Superintendent of Highways, Town of Pound Ridge; Joy Simpkins, Supervisor, Town of Pound Ridge.

Joy Simpkins explained the reasons she had requested this meeting. They are:

1. To review the paving requirements of the Town of New Canaan and to request relief from the need to repave an entire roadway if a method can be used to repave the waterline trench effectively to achieve satisfactory results.

2. To review the requirement for police traffic control at the scene of construction when "flagmen" can do the job of protecting workmen and assisting drivers around the construction.

A two-hour discussion ensued, including the dismay of all present that the price for paving the 8-9 thousand feet of trench in Connecticut was so high, approximatily \$811,800. I believe the **2,5** million estimated for pipe and paving is for the entire length of the trench, both in Connecticut and New York and the figure of \$990,000., or 55% of that total is for paving the entire roadway, 11,000 feet.

Evidently New Canaan has been "burned" by partial paving projects before and they all held fast to their requirement that if "you put a hole in our road, you have to replace the road."

On the subject of Police Protection at a construction site, New Canaan's contract with their Police Department requires them to hold fast to this agreement. They did say that very often, in fact, more often than not, police officers are not available for this type of duty and in that case, flagmen are allowed to hold the jobs.

However, all of the above was beside the point which is, New Canaan will not issue permits to Bridgeport Hydraulics to install a waterline through their property under any circumstances! They have zoning concerns which appear to override all other considerations. They believe the possibility of their residents being able to connect to a water line will put enormous pressure on their zoning controls in that area and force

1

unwelcome downzoning on the Town. (This fear should be familiar to Pound Ridge residents.)

This news was devistating to me and to Mort. Surely there must be something missing here. Is there a requirement somewhere which gives public utilities leeway to operate when public health is involved? Is there a way to prevent access by New Canaan residents to the piped water except in case of emergency? Surely BHC must have some card to play here. Why would they go to all the trouble and expense of designing the system and engineering the specs if they did not know they had certain rights to proceed.

These are answers we need to determine at the May 19 meeting if not before.

I will call Marc Moran and Tim Eidle as promised!

Joy Simpkins

TOWN OF NEW CANAAN

RICHARD P. BOND FIRST SELECTMAN

Town Hall 77 Main Street New Canaan, CT 06840

Tel: 972-2311 Fax: 966-0309

600 Lindley Street Bridgeport, CT 06606-5044 203.367 6621



September 8, 1999

Ms. Joy G. Simpkins Supervisor Town of Pound Ridge 179 Westchester Ave. Pound Ridge, NY 10576-1743

Re: Water Supply to Scott's Corners

Dear Joy:

This letter is written in response to your August 24 letter regarding permit procedures for constructing a pipeline between BHC's Stamford System and Scott's Corners, NY, particularly the portion of the work to be constructed in Connecticut. Although I am not an attorney, and therefore can't give a legal opinion, the following is my understanding of the approval requirements for construction of a pipeline.

BHC is required to obtain a street opening permit from the Towns in which we operate for any excavation within a Town road. In the case of a State road, the permit would be obtained from the State Department of Transportation. As a public utility, BHC has the right to appeal the denial of a street opening permit to the State Department of Public Utility Control.

In response to your second question, water rates would be based upon the cost of purchasing water from BHC (rate chart enclosed) and any incremental costs for operating the Water District. A "typical" residential customer in BHC's Stamford System currently pays approximately \$60 per quarter. The Water District rates would be higher than this by the amount of its expenses for the operating costs and debt service of the New York entity. As a point of reference, a "typical" residential customer in BHC's Eastern Division currently pays approximately \$96 per quarter.

If you have any questions on either of these topics, please call me.

Sincerely,

Peter B. Galant, P.E. Director of Engineering

Enclosure

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Mr. Richard L. O'Rourke June 21, 1999 Page 2

> construction cost estimate is \$1.1 million plus approximately \$85,000 for taps and services to the property line. The total project cost estimate, including BHC costs, is therefore approximately \$1.8 million. Please keep in mind that these costs, particularly for the treatment building, are conceptual only and may vary significantly based upon preliminary and final design. In particular, the site identified by LBG for locating the wellfield is shown as wetlands on the USGS map. The feasibility and cost implications of constructing in a wetland have not been investigated, nor have the resulting permitting requirements.

I hope this answers the cost questions raised at and after our last meeting. After your review, please distribute this information prior to our June 22 meeting.

Sincerely,

Peter B. Galant, P.E. Director of Engineering

cc: B. Brennan – JJB R. Furano - BHC See Wastewater Appendix for these reports.

2000 September, Malcom Pirnie study proposal discusses regulatory issues with potable water wells and the possibility of combining them into a Community Water Supply, but wants to start over with a wastewater study.

2002 April, Folchetti study also discusses potable water solutions, water from Stamford, drilling wells and getting water from the golf course.

Pound Ridge Waste Water Task Force

Appendix C: Westchester County Health Department

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Separate Sewerage System Exprivate Water Supply

Pound ridge Municipality

CERTIFICATE OF CONSTRUCTION COMPLIANC	<u> </u>	WCDH File N	10. PR 91-07
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Waived

Erosion Control Completed

Other Requirements

I certify that the system(s) as listed serving the above premises were constructed essentially as shown on the plans of the completed work (copies of which are attached), and in accordance with the standards, rules and regulations, plans, filed, and the permit issued by the Westchester County Department of Health.

2/9/95 Certified by Jours & Sull Date

Any person occupying premises served by the above system(s) shall promptly take such action as may be necessary to secure the correction of any unsanitary conditions resulting from such usage. Approval of the separate sewerage system shall become null and void as soon as a public sanitary sewer becomes available and the approval of the private water supply shall become null and void when a public water supply becomes available. Such approvals are subject to modification or change when, in the judgement of the Commissioner of Health, such revocation, modification or change is necessary, said modification or change shall be done under the supervision of a licensed Professional Engineer or Registered Architect.

With proper maintenance these systems can be expected to function satisfactorily and are not likely to create an unsanitary condition.

Mark S. Rapoport, M.D., M.P.H., in 5. G Commissioner, By Date 2 Westchester County Department of Health

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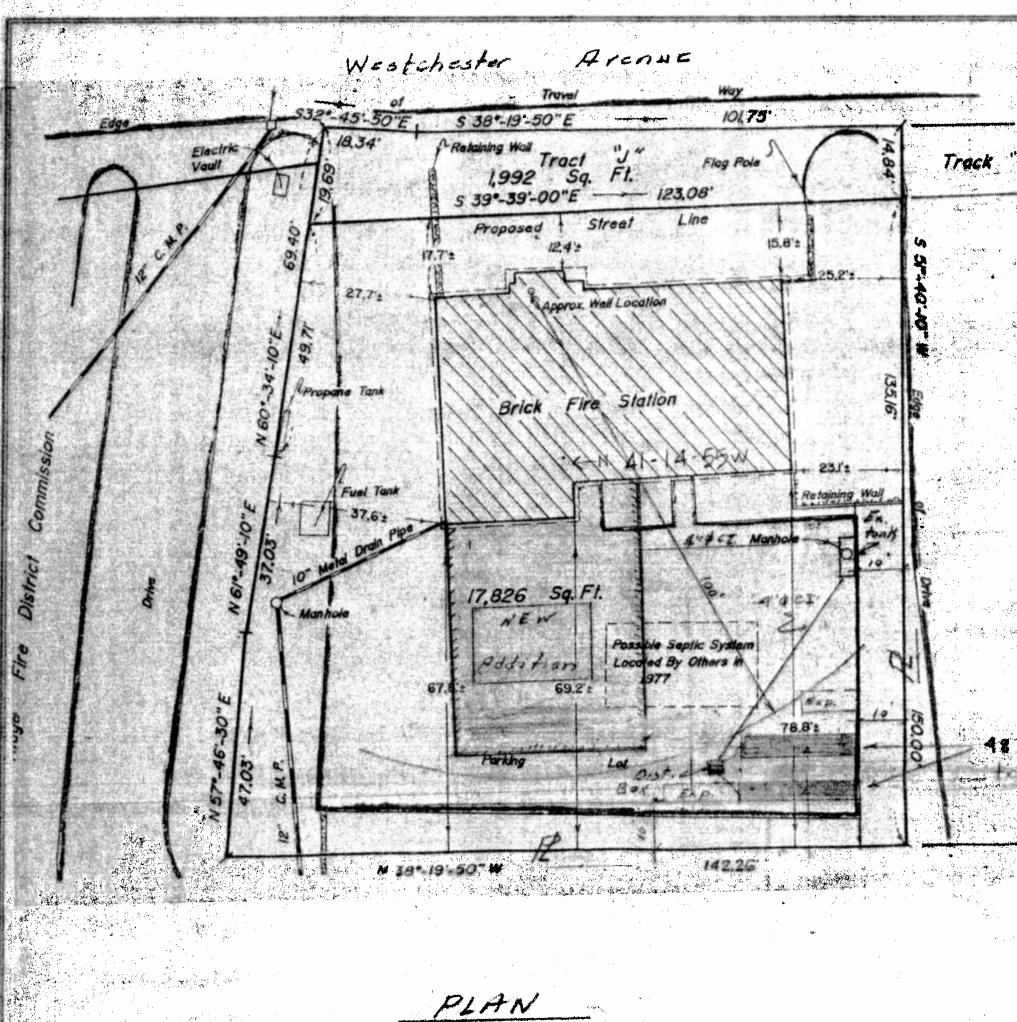
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9320-59 78 WESTCHESTER AVE

WESTCHESTER COUNTY DEPARTMENT OF HEALTH William A. Holla, M. D., Commissioner

FON OF SANITATION McLaughlin, P. E., Director Application Rec'd..... H. M. Gray, P.E., A. R. Secor Permit Issued..... R. H. Cummings, P. E., R. W. Germeroth Final Approval..... Sahitary Engineers APPLICATION FOR RESIDENTIAL SEWAGE DISPOSAL PERMIT (Please type or print) (See Rules & Reg.Form S.D.22) Will destruction of Flating & Sarah To the Commissioner of Health: Application is hereby made for a permit to construct a sewage disposal (Number; type; and use of building to be served.) 1. Owner John FRANK DI TORE Mail Address B. F.P. 1- NEW CANARN - CON, Note: (Owner must receive permit and approval. Check here for extra Property at WESTCHESTER AVE POUND RIDGE (SCOTTS CORNER) 2. Tax Map Location: Section.....Block.....Lot.....Subdivision..... 3. Construction: New, Replacement; Proposed Future Building. NEW (Expansion attic, etc.) wall uffing 40 5. Lot size 60 + 15.0... No. of rooms..... Bedrooms..... Bathroom. Source of water supply. WELL 6. 8. Settling treatment: Septic tank; liquid capacity below flow line. Material Manuel, inside dimensions: Length. G. .. width. 4... effective depth. 4. Minimum líquid capcity - 500 gallons; 200 gallons per bedroom. (MUST BE MADE BY APPLICANT AT SITE) Absorption area. 10. gals.waste(No.7) Absorption rate from table bottom area Absorption treatment: Trenches. 2.4. inches wide. 7.1. linear feet. 11. Gravel. K., cu.yds., to depth of ... inches below bottom of pipe. Leaching pits: numberoutside dimensions depth below flow line; wall area below flow line material built-up, rock-filled. Signature, Frank Di Tore Title. UWNER (By owner or person presenting owner's written authorization) Mail permit to RFD. 1. NEW CANASN, CONN. SKETCH REQUIRED showing all features of property, wells, streams and sewage disposal system. Failure to secure permit before construction is a viclation of the County Sanitary Code and is a misdemeanor, INSPECTION OF COMPLETED SYSTEM BEFORE BACKFILLING IS REQUIRED. S. D. 7 - 9/50

AL-880 Town-of Pound Ridge John & Frank Di Tore, Westchester Avenue C soleosab (list .5 brol 4/19/51 - 500 gal. 75' x 24" 1638 . 5 Bast - 68 15 13 -× 41 × 41=660 N -3-Wille Defining (? 29 20 ້ງ 30-85 רר 660 gol masning ST. 77 LF × 24" stot. 5-28-51 J. P.S.D. Town of Pound Ridge Date: Permit 4/19/51 Approval 5/29/51 Location: Westchester Avenue Section_ _____Block:_____Iot: Owner: John & Frank Di Tore, R.F.D. #1, New Canaan, Conr Builder: John Di Tore, (same) House: 1 building for dairy & garage. Soil test made: 4 min. per inch Rate: Tank capacity: 660 gal. Material: Masonry Absorption: 77 linear feet of 24 inches wide absorption

WESTCHESTER COUNTY DEPARTMENT OF HEALTH

William A. Holla, M. D., Commissioner White Plains, N. Y.

Issued April 19, 1951

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PERMIT FOR SEWERAGE SYSTEM

APPROVAL is hereby given pursuant to Article VII of the Sanitary Code of the Westchester County Health District to Frank Di Tore, R. F. D. #1, New Canaan, Connecticut.

to construct or provide a sewage disposal

system consisting of a 500 gallon mesonry septic tank and 75 linear foot of 24 inches wide absorption trench

to serve one building for dairy & garage, owned by John & Frank Di Tore, Westchester Avenue, Town of Found Ridge, New York.

for an occupancy of persons, provided that

V1

- I. No portion of the system shall be backfilled or covered until inspected. Inspections are made during regular working hours only. Twenty-four hours' notice is required.
- II. The system shall not be used until it has been constructed in an approved manner, inspected and backfilled, and the written final approval thereof shall have been obtained from the Department of Health. (See Item VIII).
- III. Additional or more adequate facilities shall be provided whenever it is determined by the Commissioner of Health that such facilities are necessary, for which an additional permit shall be obtained.
- IV. This system shall be maintained and operated in complete conformity with rules and regulations for the protection of public water supplies, all applicable laws, local ordinances, and the provisions of the Sanitary Code, existing or hereafter enacted.
- V. When sludge and scum shall so accumulate in any tank as to occupy a depth at any point of more than one quarter of the liquid depth of the tank, they shall be removed and disposed of in accordance with the requirements of the Sanitary Code, and so as to create no nuisance.
- VI. A connection to a public sanitary sewer shall be made whenever such sewer shall become available.
- VII. This permit remains the property of the Department of Health and is revocable at any time or subject to modification or change whenever the Commissioner of Health shall deem necessary.
- VIII. It shall be the responsibility of the person obtaining this permit to deliver a true copy thereof together with a copy of the final approval to the owner of the premises served by this system before this system is placed in use.

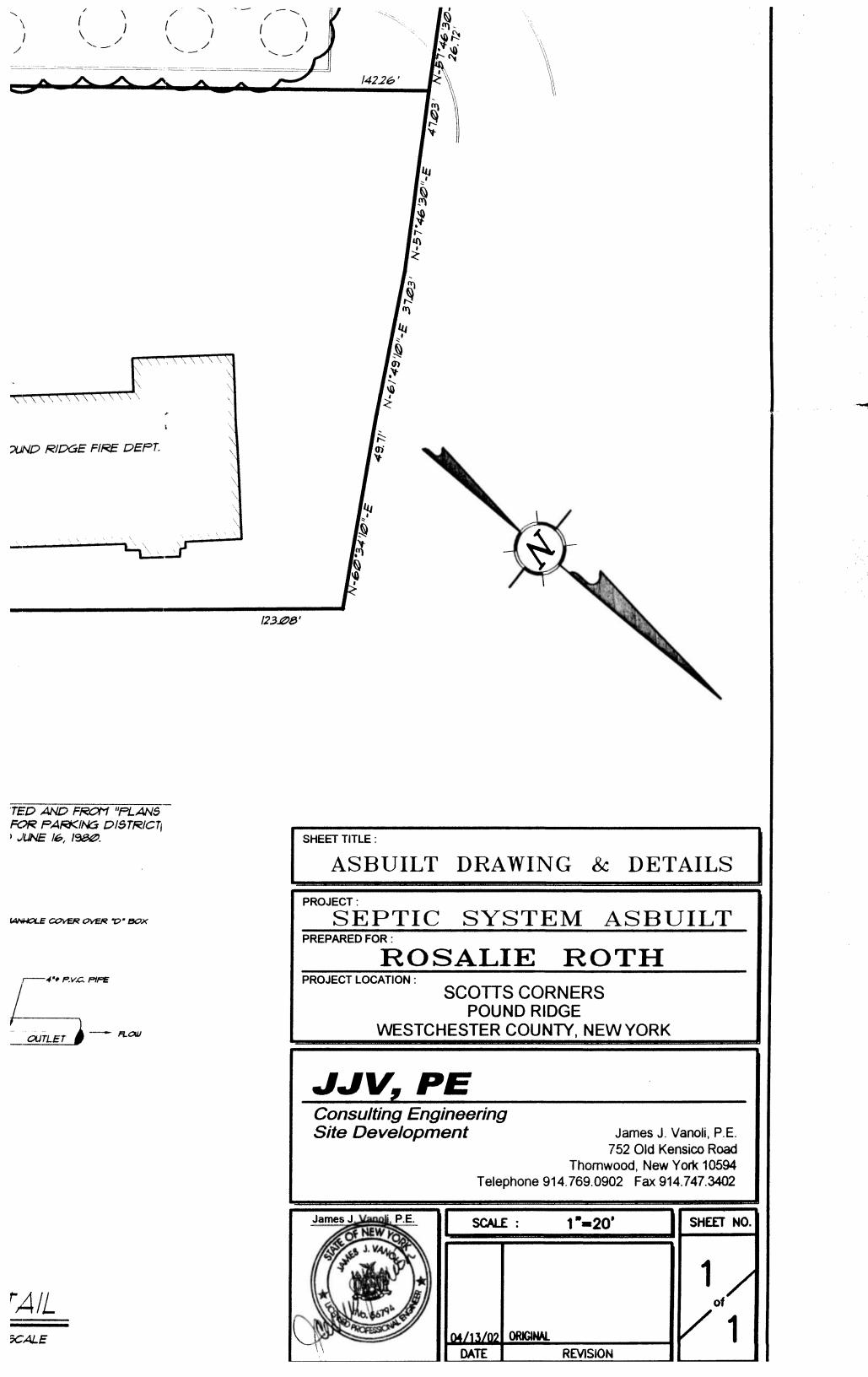
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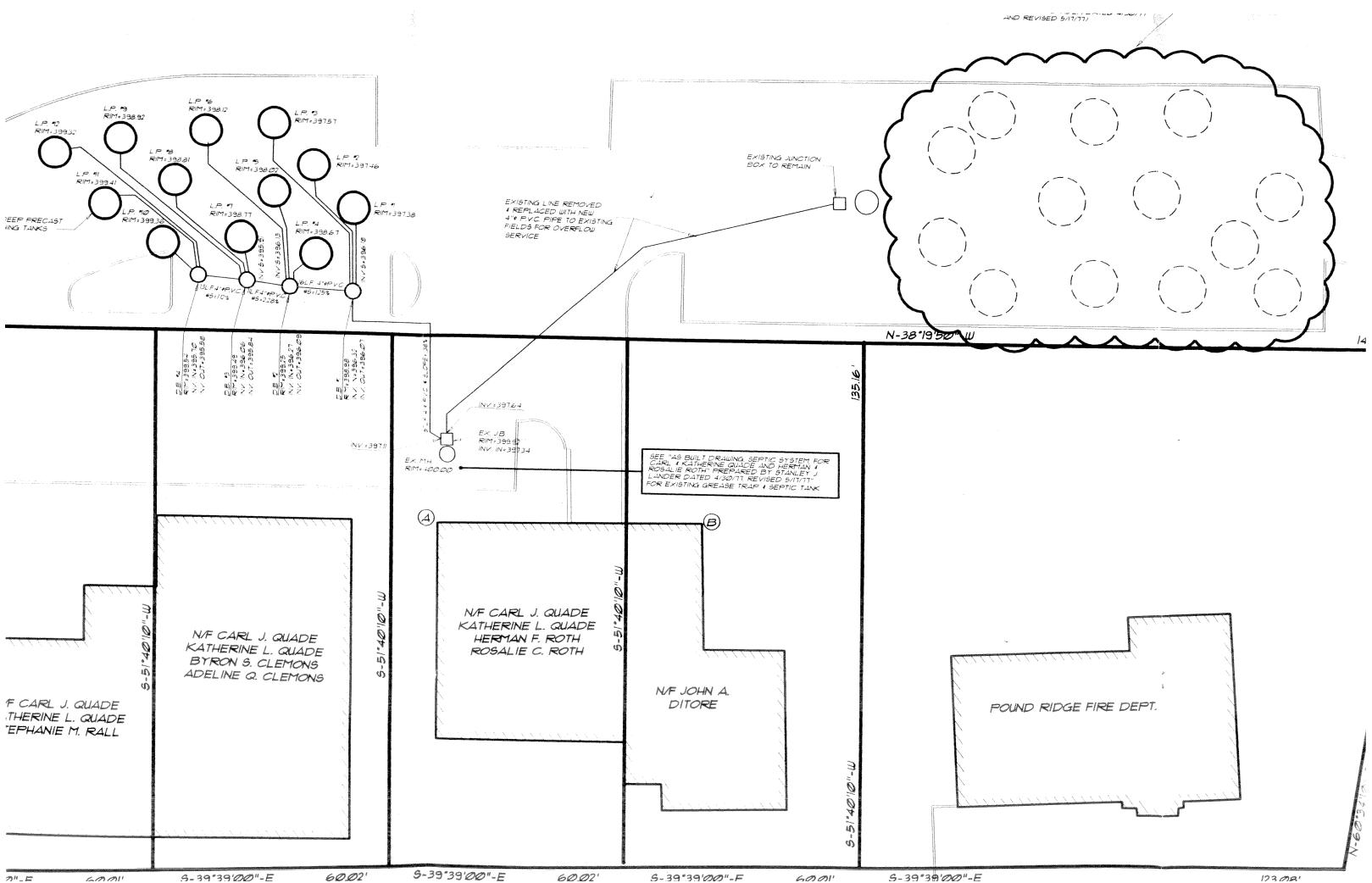
Commissioner of Health

S. D. 5 9-50-21767

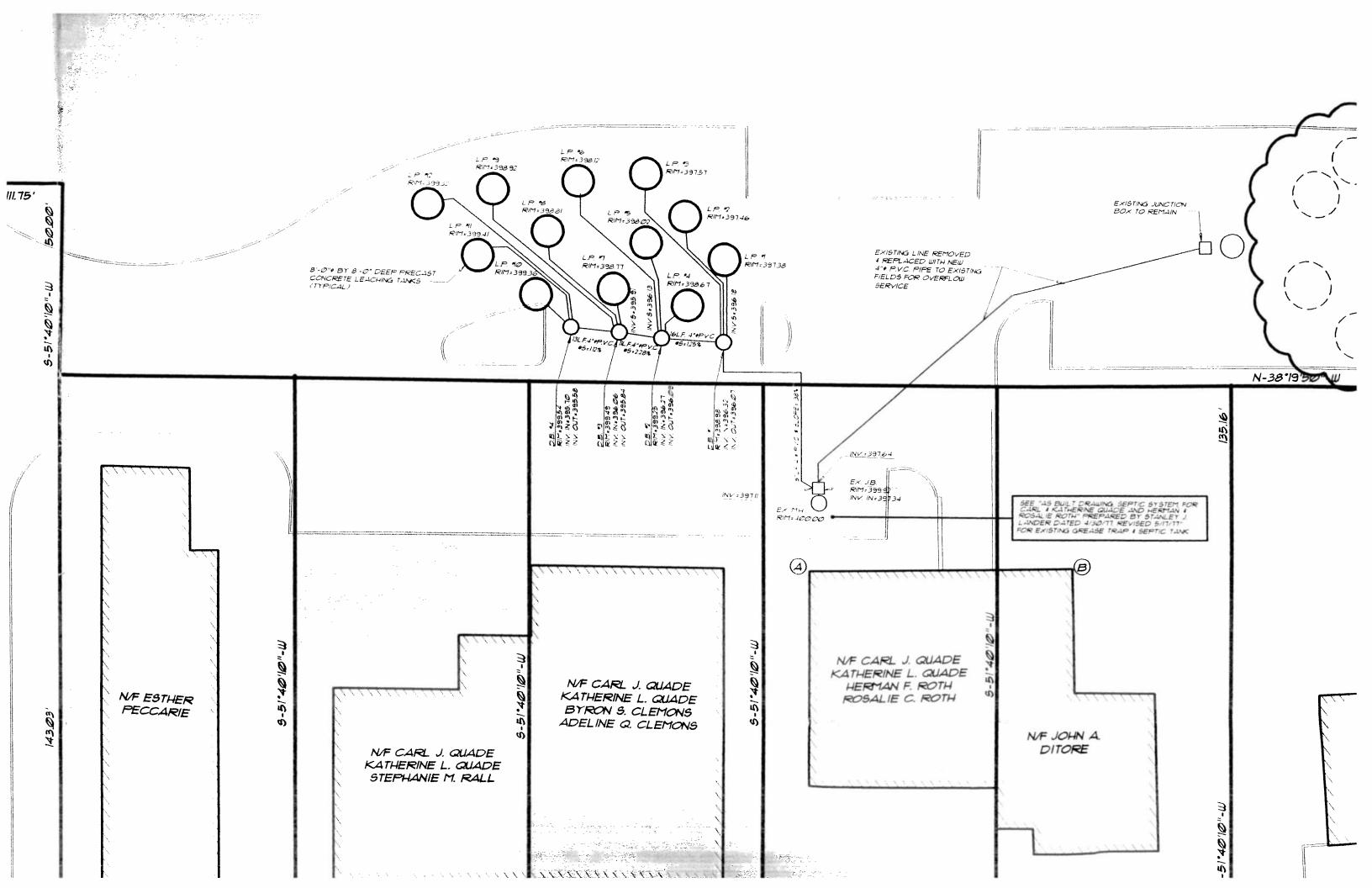
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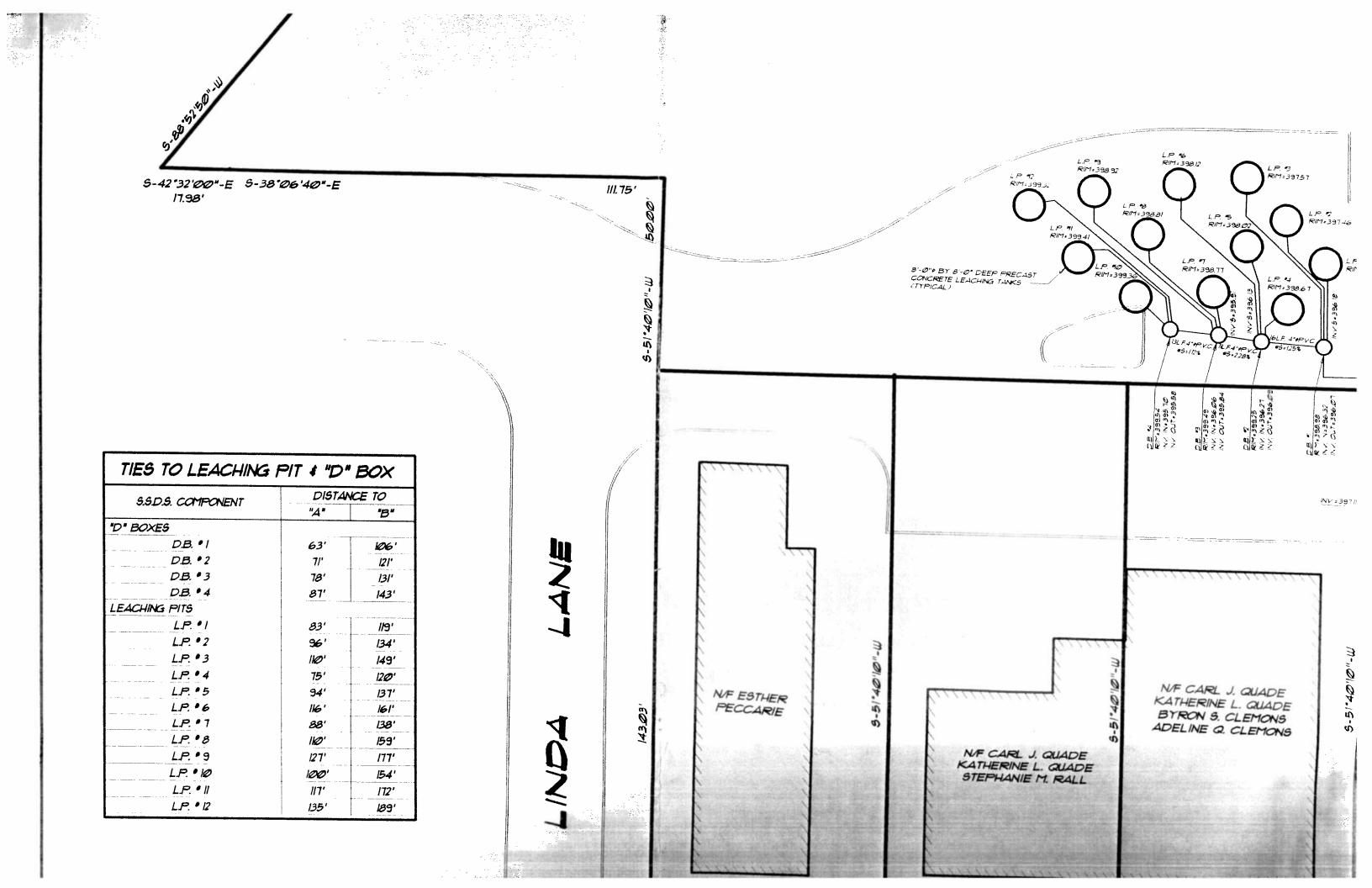
9320-60 76 WESTCHESTER AVE

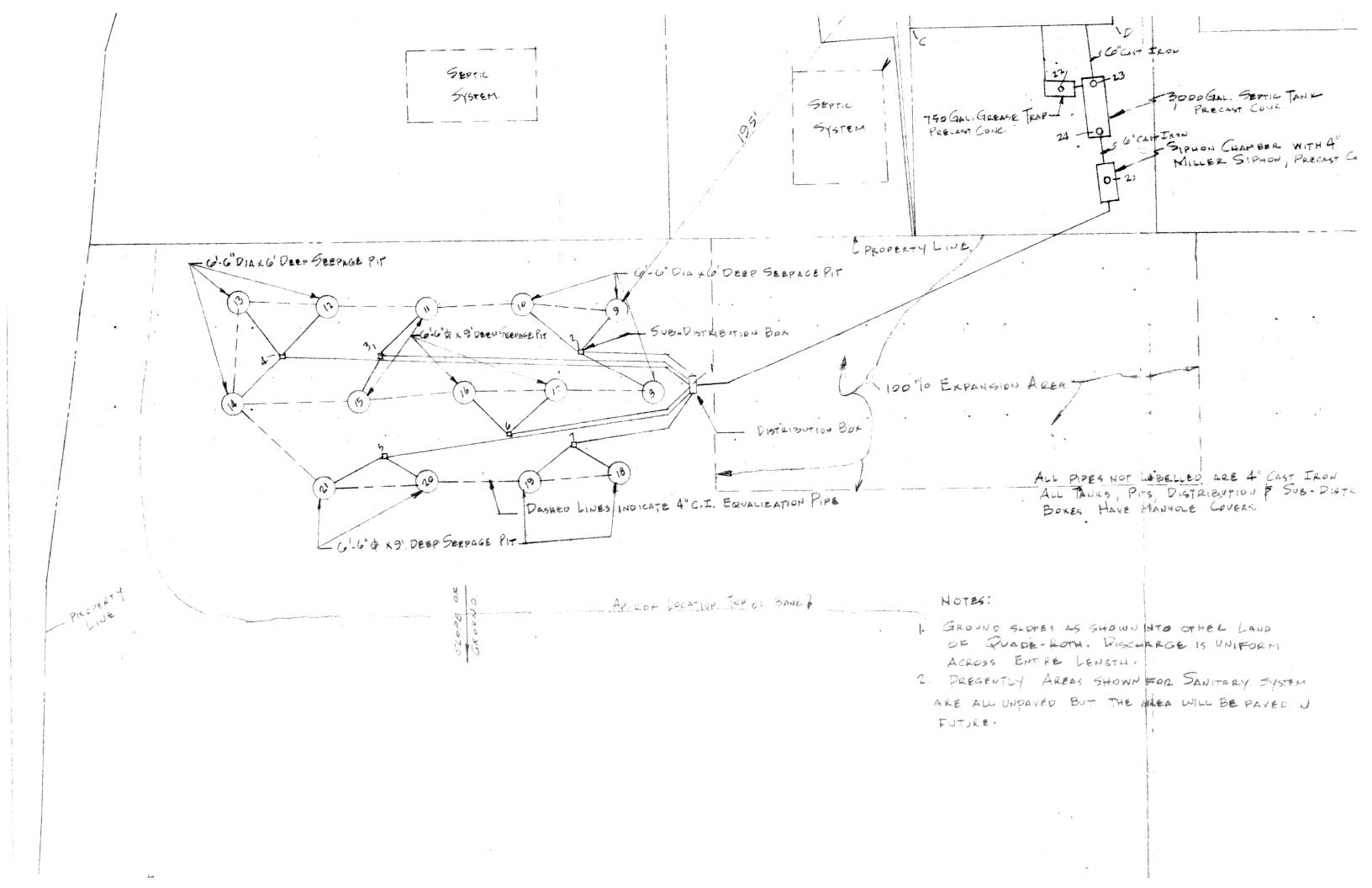


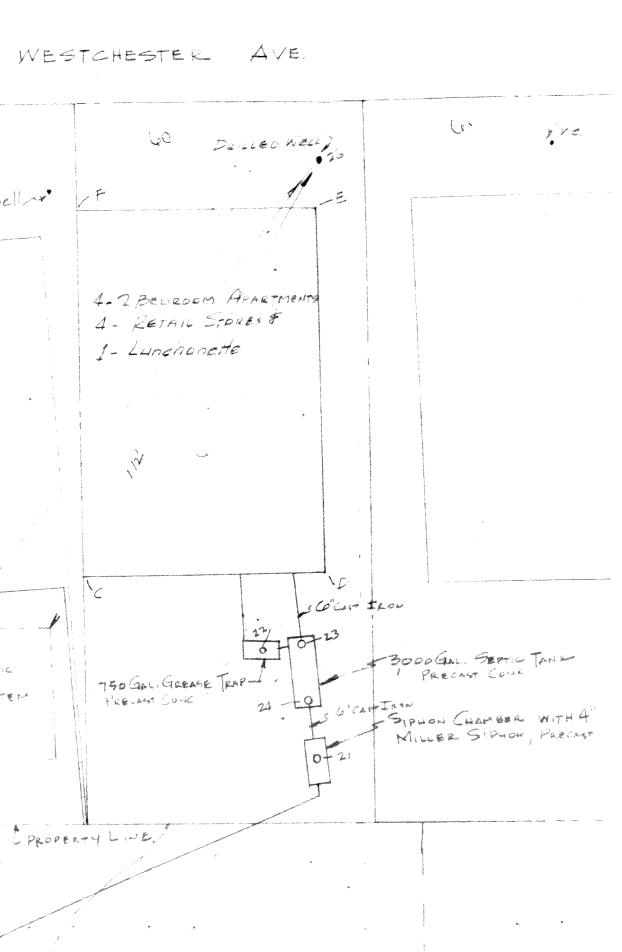


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7.	130'	140						
8	119'	142'						
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ACCEPTED AS FINAL PLANS DATE <u>5/23/77</u> WEST. CO. DEPT. OF HEALTH BY CELLUL

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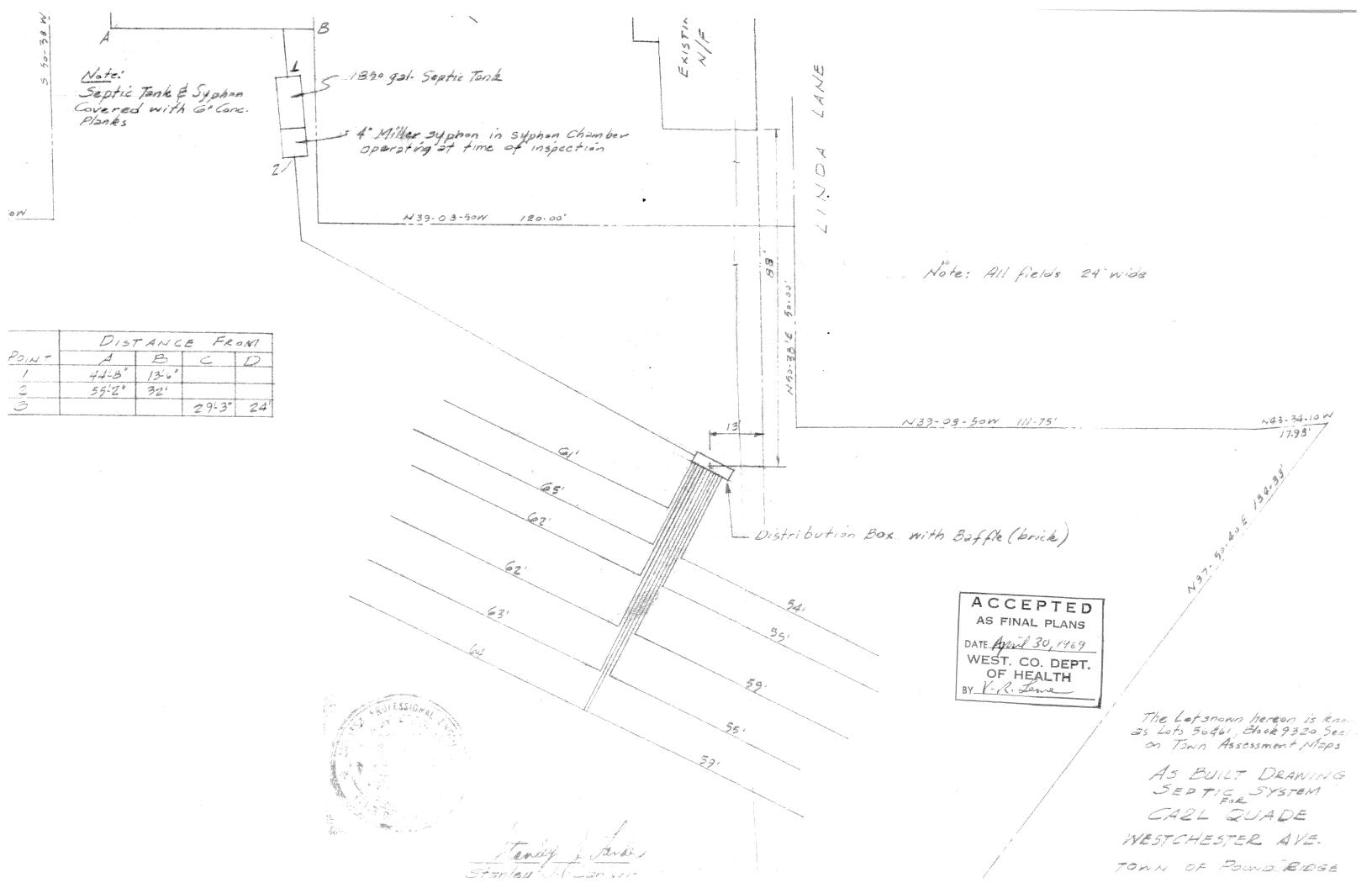
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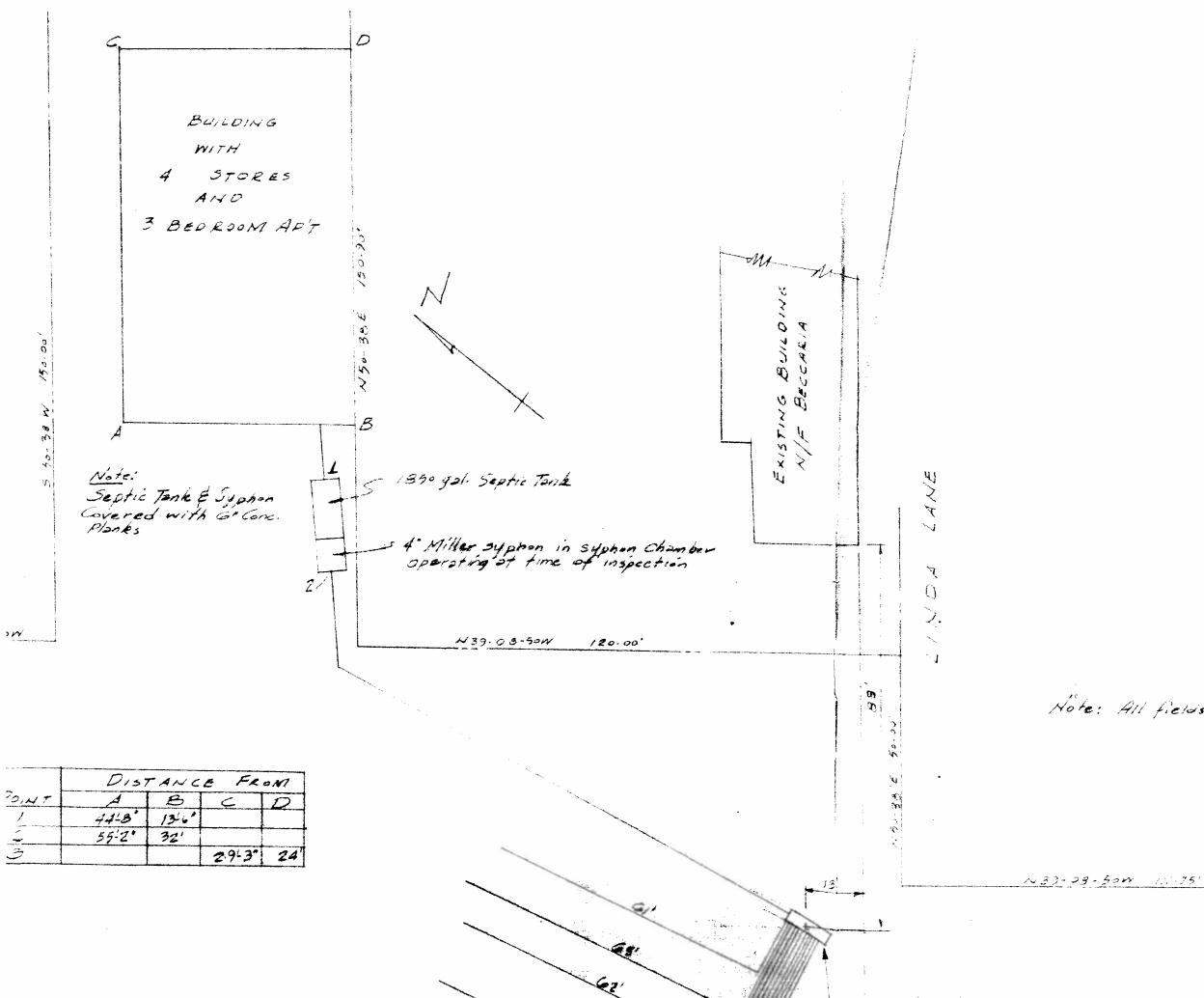
THE LOTS SHOWN HEREON ARE KNOWN AS LOTS 56460 BLOCK 9320 ON TOWN ASSESSMENT MAPS-

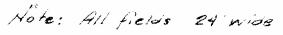


AS BUILT DRAWING SEPTIME SYSTEM FOR CARL & KATHERINE QUADE HERMAN AND ROSALLE ROTH WESTCHESTER AVE TOWN OF POUND RIDGE WESTCHESTER COUNTY, N.Y. AREIL 30 1977 REV. 5-17-77.

9320-61 74 WESTCHESTER AVE





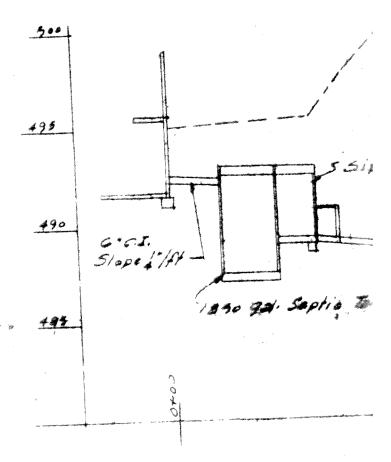




ENGINEER'S REPORT

- 1. Oasign Flow: 4 stores @ 430 galiday + 1800 galfolay + three ardroom
- sparting of a 200 gal per day & 2700 gal / day. 2. Size of septic tank from figure 1, Bulletin 1, Fant IT, Intermethote Maste
- Treatment Wards of N.Y.S. Health Dopt. a 1850 gals. 3. Tank size to serisfy requirements: 11:0"Long & 446" wide x 5:0" deeps 247.3 CU. FR. S. 247 1 145= 1830 gals. Prayide two compartments. For 7-9" Long inter comportment. Noold represent 7.15. 70.2% of tatal capacity
- 4. Langth of fields required: 2703 660 Lin. At of 24" tranch.
- 5. Size of Siphon Chamber Geo (0.5) = 44.1 East. Using 4' Siphon, Drawing depth: 1-5" width of Chembers 4-6" " Lengthe titles)" 6.9' soy a 644"

WESTCHESTER AVE. N 39 12 W 13:32' N 39 32 W 46-68' L Prop. Drilled Well 100 Min to existing septic systems. PROP. BUILDING NITH STORES s IND 3 BOBOOM HET



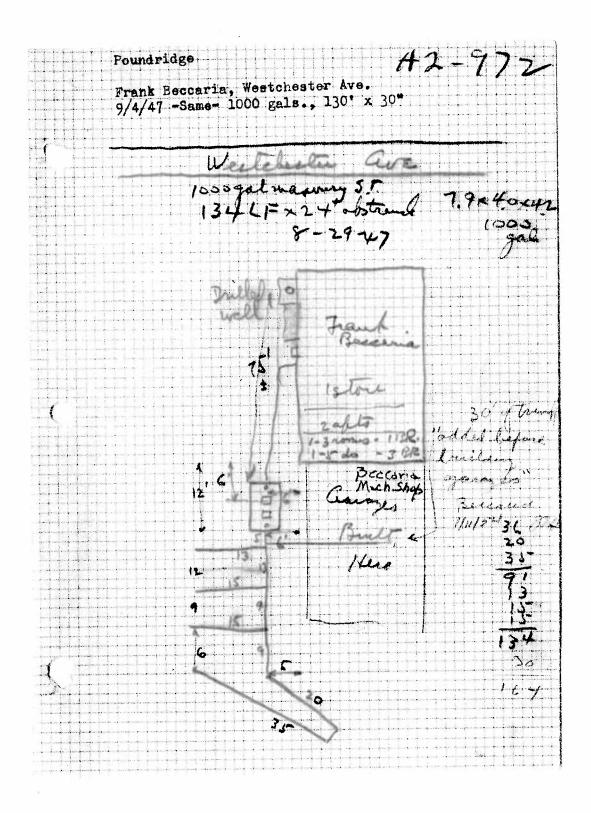
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9320-63 70 WESTCHESTER AVE

9320-63 70 WESTCHESTER AVE

Locatio	n: Westches	iter Ave.	Date: 9/3/4	
Sec	tiont	Block:	Lot:	
Owner:	Frank Beccs	ria, R.F.D.	5, Ridgefield	, Conn.
Builder	-Same-			
	4 bedrooms	and 2 bathro	oms (Max.Occ. Rate:	7 persons
Tank co	pacity: 10	000 gallons	Material: 24 inches wide	masonry e skaxax



S. D. 5

WESTCHESTER COUNTY DEPARTMENT OF HEALTH

William A. Holla, M. D., Commissioner

Servers Poundidge

White Plains, N. Y.

PERMIT TO PROVIDE A SEWAGE DISPOSAL SYSTEM

Application having been duly made to the County Commissioner of Health as required by Article II of the Sanitary Code of the Westchester County Health District permission is hereby given to Frank Beccaria, R.F.D. 5, Ridgerield, District, permission is hereby given to Connecticut, for the constru

Gennecticut, for the construction or provision of a sewage disposal system consisting of a 1000 gallon masonry septic tank and 130 linear feet of

30 inches wide absorption trench.

to serve a house owned by Frank Bescuria, Westchester Avenue, Town of Foundridge, (Maximum Occupancy - 7 persons) New York

subject to the following conditions:

I. That this department shall receive due notification and be afforded an opportunity to inspect the system before any portion is backfilled or covered.

II. That this system shall not be used until the written final approval thereof shall have been obtained from the Department of Health.

III. That such sewage disposal system on 1 be constructed in complete con-formity with the application data and plans to approved or with approved amend-ments thereto. Any changes in this system much be approved.

IV. That such system shall receive only the sewage or wastes from the structures or premises covered by the spermit.

V. That such system shall be so maintained and operated as not to expose sewage or sludge, or create a condition of nuisance.

VI. That this permit and not be construed to invalidate any rule or regu-lation enforceable by any local authority having jurisdiction.

VII. That all duly on ctobules and regulations for the protection of water supplies shall be completed with.

VIII. That a connection to the public sewer shall be made as soon as such is available.

IX. That whenever it is determined by the Commissioner of Health that additional or more adequate sewage disposal facilities are necessary, such facilities shall be provided, plans for which shall first be submitted to and receive the approval of the Department of Health.

That whenever the sludge and scum shall so accumulate in any settling х. tank as to occupy together at any point more than one-fourth of the distance between the bottom and the flow line, they shall be removed.

That whenever sludge and scum is removed from any settling tank or any part of the system, it shall be done in such a manner as to cause no nuisance and the material disposed of by burial in some remote place at least 250 feet from any house, road, well, spring, stream or other body of water, and covered with not lea than 6 inches of earth in such a manner that it will not flow or be washed by rain or melted snow or other means over the surface of the ground or into any well, stream, spring or other body of water.

XII. That this permit shall be revocable at any time or subject to modification or change when in the judgment of the Commissioner of Health such revocatio. modification or change shall become necessary.

COMMISSIONER

September 3, 1947. Date: HUG: NE

THE OWNER OR HIS AGENT MUST RECEIVE THIS PERMIT OR A COPY THEREOF

ec: Mr. Everett B. Knapp, Town Clerk.

IION COUNTY OF WESTCHESTER DEPARTIENT OF HEALTH Permit Director William A. Holla, M.D., Commissioner Inspected by Final Approval..... County Office Building rett Sketch File White Plains, New York ray 1 Secor ry Engineers APPLICATION FOR SEWAGE DISPOSAL PERMIT ۲ (See Rules and Regulations - Form S.D. 22.) Date To the Commissioner of Health: Application is hereby made for a permit to construct a sewage system to serve A house (Number, type and use of building to be served.) concerning which the following information is submitted: Owner Frank Beccaria Mail Address K. 5 Kidger 60mm ' Check here for extra copy Note: Owner must receive permit and approval. Property location. Wert chester arel, Place Prind vdg 2. Town, City) Tax Map Location: Section..... Block. 7.3.4 Lot..... Subdivision..... 3. +46 Construction: New, Replacement, Proposed Future Building. 4. Lot area 40 X 15.0. No. of rooms. 8. Bedrooms. 4. Bathrooms. 2. Extra Lavatories..... Special Fixtures..... Maximum Future Occupancy..... Source of water supply Arlesian Well 6. Watershed on which system is located. Distance to nearest watercourse. Sae / Owner's wells J.C. Adjacent wells J.C. f. 8. Settling treatment, Septic tank: liquid capacity below flow line. 100.0.2. diam......... Note: Liquid capacity of tank shall be not less than volume of waste per day with a minimum of 500 gals. 9. Type of soil: clay, loam, sand, boulders, rock; surface: flat, sloping, steep; drainage: good, fair, poor. (from table) Note: Except in clay soil, a rate of 1 gal. per sq. ft. of bottom area per day shall be used unless a higher rate is established by soil test. 10. Absorption area: 32.5.7. --- 2.6 bottom area. gals. waste (No. 7) Absorption rate from table 11. Absorption treatment, Trenches: . 3.0. inches wide; 4.3.6. linear feet of distributing tile; Gravel.1.Q... Cu. yards, to depth of inches below bottom of pipe. Leaching pits: number..... outside dimensions..... depth below flow line. wall area below flow line..... naterial.....built-up, rock-filled. Signature: Trank Deccaria Title: Contractor (By owner, builder, or officer of sewage disposal firm, or contractor Mail Address: R. F. D. 5 Ridgefield Comm. Sketch required on reverse side or on attached sheet showing plan with general relation of dwelling and property boundaries, wells and streams to system and arrangement of absorption facilities, together with all other pertinent data, including details of grease trap, manholes, diversion gates, siphon, curtain drains, special structures and unusual features. Failure to secure permit before construction or final written approval of the system before using is a violation of the County Sanitary Code and is a mislemeanor.

S.D. 7 2/47

9454-10 73 WESTCHESTER AVE

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ERTIFICATE OF COMPLETION	
rated at N/S Walte Gave Scalts Consection Ward	
mar Mendanger Developement Co Block	
rstem built by the Lace Co	
ilding type Bank Stars Permit issued 27 Jan 51 W.C.D.H. File PR3-1	
estem consists of 2700 Gal. masonry, metal septie tank 512 Lineal feet X 2 Width trench	
the second s	
The separate sewage system serving the above premises was constructed essentially in accordance with plans filed with this Department and the terms of a Permit issued on the above date and otherwise as shown on plans of the completed work, copy of which is attached. Any person occupying the premises served by this system shall promptly take such action as may be necessary to secure the correction of any unsanitary condition resulting from such usage. This approval is revocable as soon as a public sanitary sever shall become available and is subject to modification or change when in the judgement of the Commissioner of Health such revocation, modification or change shall be necessary. TRUE COPIES OF THE PERMIT, PLAN OF THE SYSTEM AND OF THIS CERTIFICATION, AND ANY CHANGES THEREOF SHALL BE MAINTAINED ON THE PREMISES AT ALL TIMES AND SHALL BE SHOWN TO ANY REPRESENTATIVE OF THE COMMISSIONER OF HEALTH UPON DEMAND. With proper maintenance this system can be expected to function satisfactorily and is not likely to create an unsanitary condition.	and the second
ate 1-17-59 William A. Brumfield Jr., M. D., Commissioner By aR. Sorn Westchester County Department of Health San Eng.	
parate Sewage System · D. D.	
PPLICATION & CONSTRUCTION PERMIT	
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Building TypeLot Area	
STEM CONSISTING OF	
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UARANTY: I represent that I am wholly and completely responsible for the location, material, construction and drainage of the proposed system and hereby guaranty to the owner, his successors, heirs, or signs, that the system above described will be constructed as shown on the approved pien, or approved amendment thereto, and in accordance with the standards, rules and regulations of the Westchester and Department of Health, and that an completion thereof I will furnish a written guaranty to the owner, his successors, heirs, or assigns, satisfactory to the Commissioner of Health to place in good operating addition any part of said system constructed by me during the period of two years immediately following the date of construction of the angual system or any repairs thereto.	
PROVED FOR CONSTRUCTION: This approval expires one year from the date issued unless construction of building or sewer system shall have been undertaken, and is revokable for cause or may be amended or addited when considered necessary by the Commissioner of Health. Any change or alteration of construction requires a new permit. Approved for disposal of domestic sanitary sewage only.	
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	County of Westchester DEPARTMENT OF
2	DESIGN DATA SHEET - SEPARATE SEWERA
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	Present Mail Address WICTOR. CHI
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	Watershed Damfand
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	Soil Rate Used
	Soil Rate Approved
	Absorption Area Required (Table Item 1
	Absorption Provided By 16.Lines of. TRIPLICATE PLANS AND PROFILES OF SEWER THAN 1" TO 20' HORIZONTAL AND 1" TO 10
	<pre>PLOT PLAN (Check Items) </pre>
	OWNER (); BUILDER (IF CORPORATION, GIVE NAME AND TITLE 77 MAIL ADDRESS 488 GAENBROCK .R

POF HEALTH Division of Environmental Sanitation
WERAGE SYSTEM FILE NO. P.R. 3-1.
STER AUE POUND RIDGE Job #.
ELOPMENT CO. Sec. Blk Lot.
CHRIST-JANER, ARCHT- NEW CONDAN CONN
Lot Area. 34 AC + .S. D. Usable Area 15,00050FT+
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• • • • • Future: Yes. • . No. • . Other • • • •
Item 5.1). 2-700 .Gals, Masonry. Metal.
in/1" Drop: Soil perc. test data;test pit data
A.ft/gal. Checked by Date
tem 10.5)
of. 32-ft. x 24" trench; other SEWERAGE SYSTEM REQUIRED DRAWN TO SCALE OF NOT MORE TO 10' VERTICAL FLOOR PLAN OF BUILDING (REQUIRED)
SEPARATE SEWERAGE DISPOSAL SYSTEM PROFILE 1. Identification 2. Scales, date 3. Section - main system 4. Pipe Invert Elevations 5
en grown Reviewed by <u>Area</u> . Date: <i>Larea</i> . Date: <i>L</i>

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		Westchester Coun
-	-	TEST PIT DATA REQUIRED T
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	12"	SANDYLOOM
	18,"	SANDY LOAM
	24"	SAND COAM GRAVEL
	30"	SAND GROVEL
	36"	
	42"	
	48 ⁿ	
	54"	
	6 0 "	
	6 6 "	
	72"	
	7 8 "	
	84 ^H	
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	Tests ma	de by . I.H.E. P.F. D.S.C.

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S.D.27.6 3/18/57

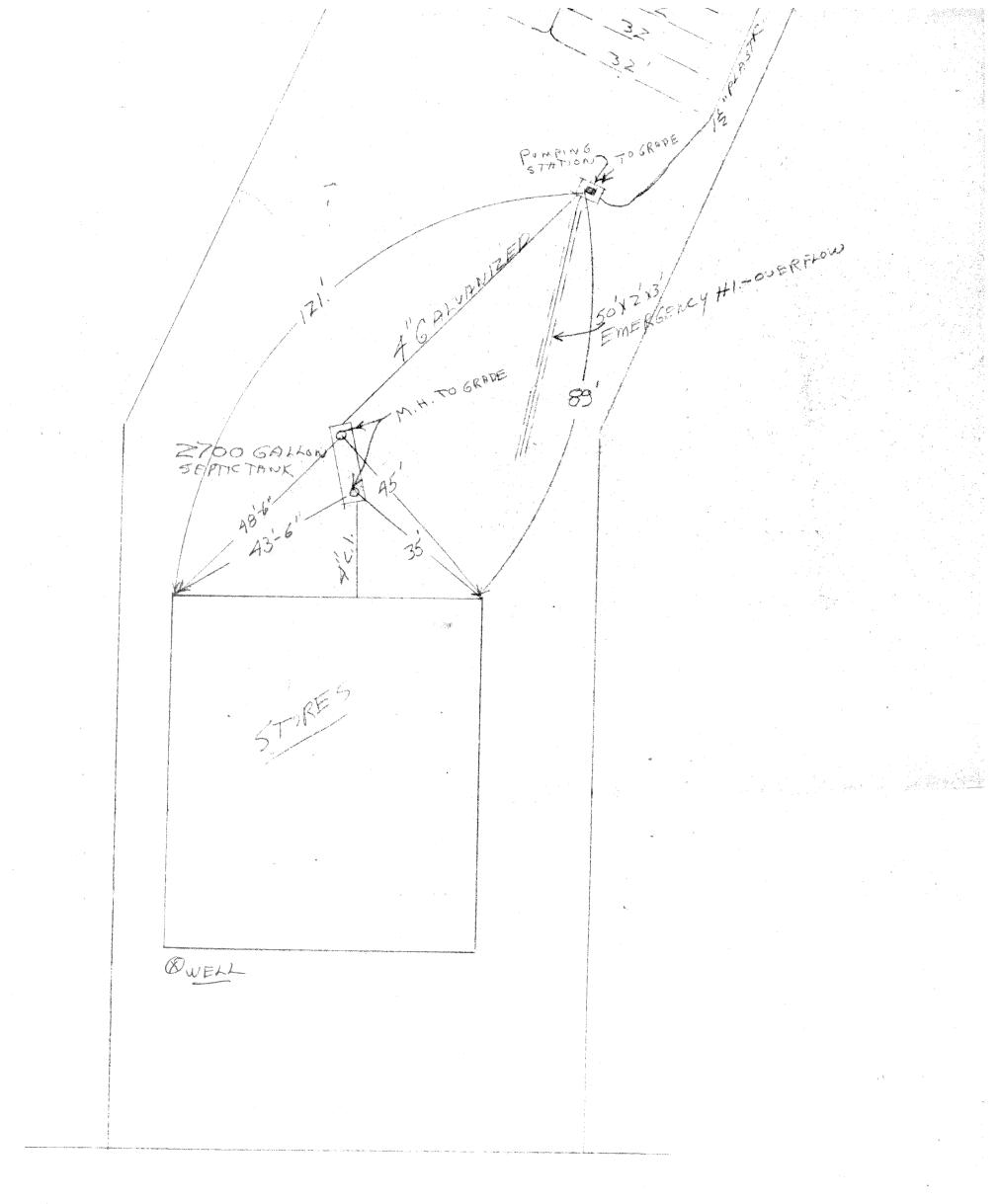
nty Department of Health $PR3-1$
TO BE SUBMITTED WITH APPLICATION
LS ENCOUNTERED IN TEST HOLES
HOLE NO. HOLE NO.
TOP SOIL
TOPSOIL
SANDY KOOM
SANDY LOAM
SANDY-LOAM GRAVEL
SAND & GRAVEL
IS ENCOUNTERED RISES AFTER BEING ENCOUNTERED
E CO Date DEC. 3.9,1958

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WESTCHESTER

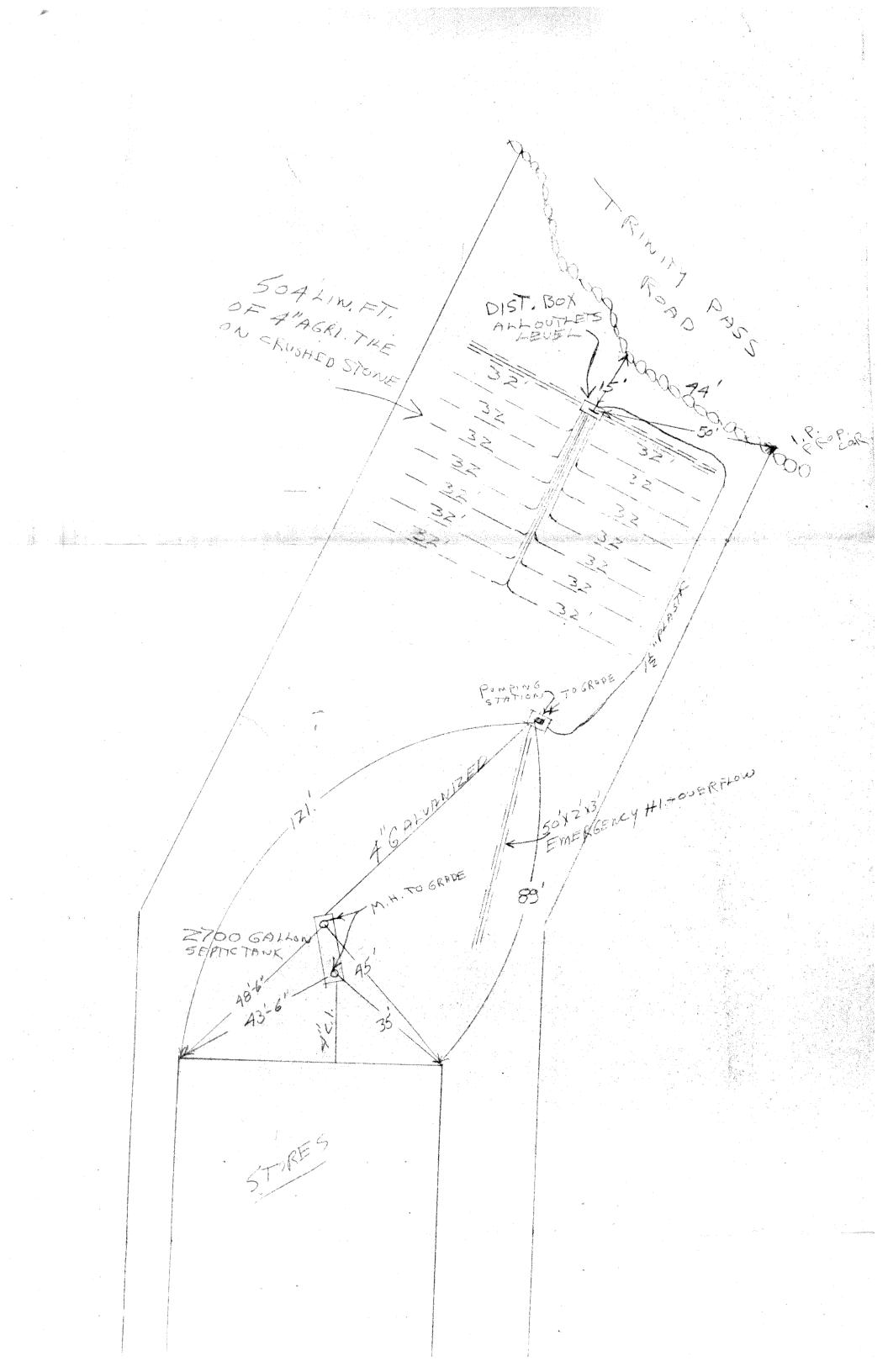
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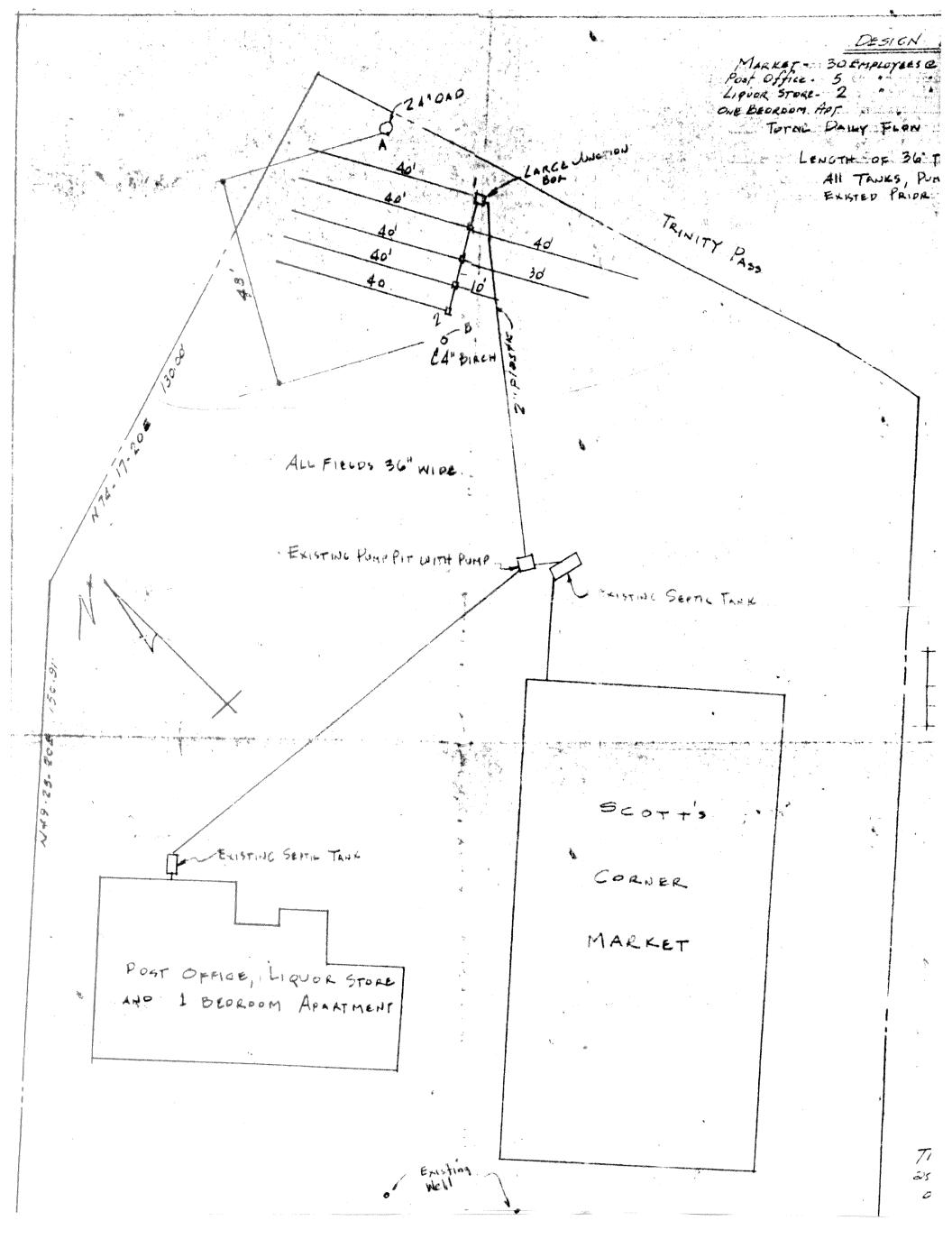
SEWAGE DISPOSAL SYSTEM FOR NEW CANAAN DEVELOPMENT SO, APPROVED SCOTTS CORNERS - POUND RIDGE THE PEASE SEP 1 71959 West, Co. Dept. STAMFORD By an deco SCALE 11-20'

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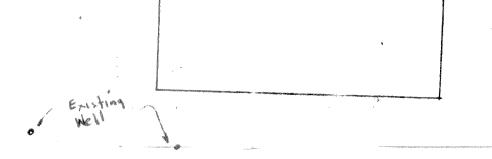
COMPANY CONN. AUG. 1959



9454-12 and 9454-11 69-71 WESTCHESTER AVE



DESIGN FLOW ٩., MARKET - 30 Employees @ 156PD+ 450 Post Office - 5 Liquor Store - 2 450 24'OND ONE BEDROOM APT 300 0. 8 56 Toroc Dawy FLAN LENGTH OF 36 TRENSH BASED ON 20 MIN SOIL All TANKS, PUNP PIT, PUMP & AGARN SYSTEM EXISTED PRIDE TO THIS WORK BOT TRINITY DASS 40 40 40' 30 40 CA" BIRCH N . ALL FIELDS 36" WIDE . · EXISTING PUMPPIT WITH PUMP KISTING SEPTIC TANK FROM DISTANCE B A POINT -32:10" 26-1" 1 6-1" 42-3" 2 SCOTTS è. ING SEPTIE TANK CORNER MARKET QUOR STORE DOM APAATMENT



The lot shown hereon is known as Lots 11 F12 Block 9454, MADIS on Town Assessment Mars.



Westchester County Department of Health Bureau of Environmental Quality

CERTIFICATE OF CONSTRUCTION COMPLIANCE APPLICATION
WCDH File #: PR2012-12 Municipality: Pound Ridge
Residential Commercial Watershed Basin Name: <u>Mignus River Higugrig</u>
NYCDEP Watershed: Y D N D Joint Review D NYCDEP Log # Delegated Review D
Property Information:
Property Name Pound Ridge Playa. LLC.
Property Address 69 Weitcheiter Ave Paula Rider All - and And
TMD: Section Block 9454 Lot R.S. Lot _/2_ Lot Area Acres
Really Subdivision: 1-+ / 1+ (ng (amage and))
St. #: 117 St. Address: Glendale Rd Scarsdele State: My Zip Code: 10583
Owner Phone #: 917-447.9830
Building Type: Commercial # of Bedrooms: Date Construction Approval Issued 2000
On-site Wastewater Treatment System (OWTS) Information:
Design Flow: <u>450</u> gpd Soil Percolation Rate: min./in
Slope of OWTS Area:% Septic Tank Size: Gallons (Gal.)
Absorption Trench(es): Length: Lin. Ft Trench Width: Et Area:
Absorption Pit(s): # Pits Diameter: Ft. Depth: Ft. Area:Sq. Ft.
Other (circle or specify): Tri-Galleys 4X4 Galleys Flow Diffusers Name:
Lin.Ft. Trench Width: Ft. Sidewall Area:
Pump System: Pump Chamber: Size: Gal. Gal. Overflow Tank: Size: Gal. Gal. Curtain Drain: Depth: Ft. Width: Ft. R.O.B. Sand and Gravel Fill Section: Depth: Ft. Erosion Control (EC) Completed Ft. R.O.B. Sand and Gravel Fill Section: Depth: Ft.
Erosion Control (EC) Completed
Separate Sewage Contractor (SSC): Name: <u>RRIbeiro (United)</u> WCDH SSC License # <u>00109</u> .
Water Supply System Information:
Private Water Supply Public Water Supply Name:
Well Driller Name:
Well Driller Name:
Address: Phone: ()
Other Requirements/Conditions: 1500 G41 Grease Trap
of which are attached), in accordance with the standards, use an reconstructed as nown on the plans of the completed work (copies
County Department of Health. Date: 1//// Signed: PE/R:A Seal 40230
Any person occupying premises served by the above system(s) shall promptly take such action as may be necessary to secure the correction of any unsanitary conditions resulting hom such usage. Approval of the on-site wastewater treatment system shall become null and void as soon as a public sanitary sever becomes available, and the approval of the private water supply shall become null and the Commissioner of Health, such revocation, modification or change is necessary, said modification or change shall be done under the supervision of a licensed Professional Engineer or Registered Architect. With proper maintenance the systems can be expected to function satisfactorily and are not likely to create an unsanitary condition.
() and the interview of the oreare an unsanitary condition.

Date: 12/3/12	Approved By:	
Land A.		

Joshua Lipsman, M.D., J.D., M.P.H., Commissioner, Westchester County Department of Health

Rev. 5/13/09

1) Flow Date - Pound Ridge Plana - 69 Wist-chester Ave - 450 gpdinch. Pizza Rest. 105 gpd - Existing Velocity Disupation Box Liquor Store - Tillestchester Ave - 405 gpd. Total Flow to Infilter tous 8559 2/ New Grease Trop 1500 gal - 25' From L'Force Plain New 1500 Gal. Grease Trop. (3) SITE MAP Existing Catch Basin - Grease Trap shall Be GT 5×10-15 (Precast) APPROXIMATE LOCATION OF J DISTING ABSORPTION TRENCHES 280 LF OF 36" WIDE TRENCH puc line to New Grease Trop 1500 Gallons . Heavy Duty Grease Trap is 25' From Existing the Basin by Rotondo & Sons Inc. 1 - Existing Septic Tank 2- 11 Pump Pit Storm Drain. 31 Water Shed - Mignus River 01 TO THE West Am Acquarian Water Ca. Ex. 4" Sewer Line 41 Owner - Pound Ridge Playa LLC 114 Cleadule Rd. 2 6 Scarsdule MM 10582 5/ Grease Trap in Pizzeria To Be Cleaned @ least once per 3 days As Built - 11/17/12. W.C.H.D. Comments 11/14/12; 11/15/12 Pound Ridge Plaga 69 Westchriter Ave. ACCESS DRIVE Pound Ridge, MM. 10576 "As Built" 1500 gal Greate Trop - PR.2012-12 Nov. 12, 2012. 15-9452-12 Existing Well Scale 1"=30" EExisting Well John P. Annicelli, P.E. WESTCHESTER AVENUE Troy La. Bedford, N.Y. 10506 Map Shown Based on a Mepby Keane, Coppelman Gregory Deted 5116/11 914-273-3682

. 1

NOTE HILL All Water Usuaje Meter Readings in Both 69 9 71 Westchister Ave Buildings To Be Submitted to the Writehester County Health Dyst Be Submitted to the Writehester County Health Dyst Burges of Envir. Quality Burges of Envir. Quality SCOTTS CORNERS 25 Moore Ave. 1st Fl. Mt Kisco N.Y. 0.... SARLES CORNERS Atta. F. Beck Jr. NE W Pound Ridge Country Club Å Item HIGH RÍDGE 1500 Gal Precast 23 67 Grease Trop Laurel Reservoir Vicinity Map N.TS 11 Flow Date - Pound Ridge Plana - 69 West-Chester Ave - 450 gpdincl. Pizza Acst. 105 gpd . 1 - Existing Velocity Disupation Box Liquor Store - Tillestchester Ave - 405 gpd. 0 *Вох* Total Flow to Infiltrators 855000 2/ New Grease Trop 1500 gal - 25' From Force Main New 1500 Gal. Grease Trop. (3) Existing Catch Basin - Grease Trap SITE MAP shall Be GT 5×10-15 (Precost) APPROXIMATE LOCATION OF J EXISTING ABSORPTION TRENCHES 280 LF OF 36" WIDE TRENCH yit puc line to New Grease Trop 1500 Gallons - Heavy Duty by Rotondo & Sons Inc. Grease Trap is 25' From Existing Catch Basin 1 - Existing Septic Tank 10 102 Storm Drain. 31 Water Shed - Mignus River Pump Pit 01 2- 11 10 71 West Ar Ex. 4" Sewer Line

The OWTS has been constructed in accordance with the Rules and Regulations for the Design and Construction of Residential Subsurface Sewage Treatment Systems and Drilled wells in Westchester County, NY.

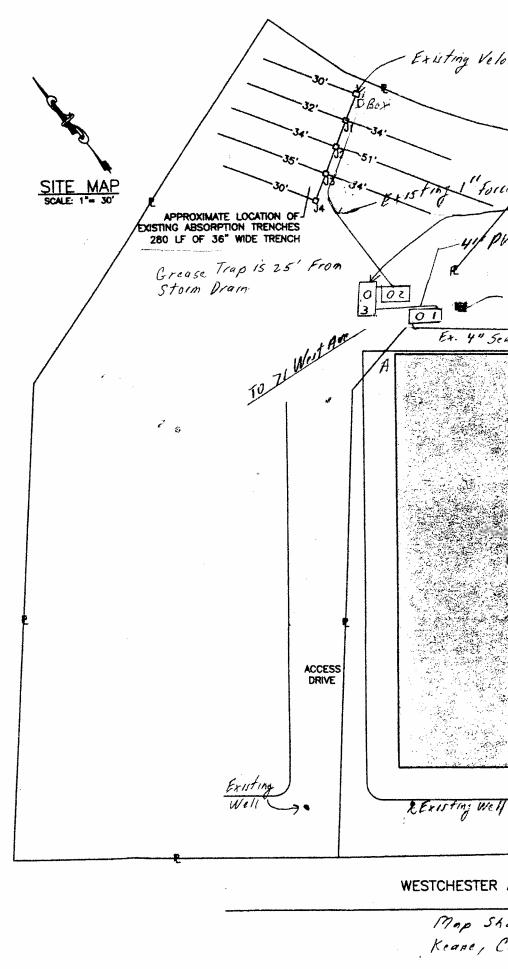
7 . 1

The design professional has supervised the construction of the OWTS and certifies to its installation is in accordance with the approved plans.

ALTERATION OF THIS DRAWING except by a licensed P.E. or Architect or licensed Land Surveyor is illegal. Any alteration by a P.E., Architect or Surveyor must be indicated and bear his seal, signature and date of alteration.

Wash	1 File # PR2012-12
A	CCEPTED
	AS FINAL PLANS
	DEC 03 ZOIZ
MES	
	T. CO. DEPT. OF HEALTH
BY_	

& GRAGE TRAD DNUJ &



- Existing Velocity Disupation Box

New 1500 Cal. Grease Trop. (3) Force Main puc line to New Grease Trop Existing Tatch Basin 1 - Existing Sepi 2 - 11 Para PARKING Ex. 4" Sewer Line EXISTING OF STORY BUILDIN mann TE OF NEW WESTCHESTER AVENUE Map Shown Based on a Mapby Keane, Coppelman Gregory Doted 5/16/11

9454-6 85 WESTCHESTER AVE

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Westchester County Department of Health Bureau of Environmental Quality

WCDH File #: <u>PR 2009</u> -067	R Mun	icipality:	Pound K	2100	
Residential Commercial	Wate	ershed Basin N	Name: Mina	a Russi	A quarrier Water
NYCDEP Watershed: Y I N I 、	Joint Review		00 #		11 goorien Water
Property Information:			-og #		Delegated Review
Proporty Name /// / /	1 . 1	1			
Property Name <u>Westchester</u> Property Address <u>85 West</u> TMD: Section <u>15</u> Block <u>947</u> Realty Subdivision: <u>Existing</u>	HUEL	<u> 11 - M</u>	orth Stor	Reston	ront
TMD: Section / Block 944	Merter 1	HVe Yo	und Ridge,	N.H.	Zip Code 10576
Realty Subdivision: <u>Fristing</u>	I LOT 6	$_$ R.S. Lot $_$	Lot	Area <u>1/2</u>	Acres
Owner Last Name: Weitcher te	- 1.co	or pull	1105		
Owner Last Name: <u>W1174Art</u> re St. #: <u>100</u> St. Address: <u>50.</u> Owner Phone #: (914) 760	Bedful	DI Own	er First Name:	L.P.	
Owner Phone #: (914) 760	-5818	<u>NU 111</u>	TRUCO S	tate: M	Zip Code: 105 %
Building Type: <u>Aes Faurant</u>	# of Be	droome			
On-site Wastewater Treatmost Suc		urooms	Date Constr	uction Approval	Issued
second of the annelit oys		ntormation [.]			
Design Soil Percolation Rate: 10	m	iin./in. Slope c	of OWTS Area:	12. % D	esign Flow: 695 apd
Components:	Existing	New			
Septic Tank:	1200		Gal.		
Pump Chamber: Dose:	550				
Overflow Tank:	1250		Gal.		
Absorption Trench(es):			Gal		
Gravelless Trench(es); Infiltrus	tio.	224	LF		Ft. Width
Absorption Pit(s): # of pits	th Copecity	1	LF		
Galleys			Ft Di a .		Sq. Ft.
Flow Diffusers:			LF		Sq. Ft.
75A Alternative:			LF		Sq. Ft.
Junction/Distribution Box(es):		2		7 J Botes	
Curtain Drain:	·····		Number	1D Box	Size
ROB Sand/Gravel Fill:			Ft Depth		Ft. Width
Other:			Ft. Depth	Anna	Sq. Ft Area
on Control (EC) Completed					
	10010		a i luctan	1 Tax	
rate Sewage Contractor (SSC): Name: (R. MIBEL	2 United	eptic Wor	H SSC License	<i>± 109</i>
Neurienienis/Conditions 4 A A	16 6.11	Y	/ A .		
ty that the system(s) as listed serving the	St Ker Sugar	TELD Dame	S. S.		
ch are attached), in accordance with the tment of Health.			approval	issued by the we	schester County

Any person occupying premises served by the above system(st shall promptly take such action as may be necessary to secure the correction of any unsanitary conditions resulting form such usage. Approval of the on-site wastewater treatment system shall become null and void as soon as a public sanitary sewer becomes available and the approval of the private water supply shall become null and void when a public water supply becomes available. Such approvals are subject to modification or change when, in the judgment of the Commissioner of Health, such revocation, modification or change is necessary, said modification or change shall be done under the supervision of a licensed Professional Engineer or Registered Architect. With proper maintenance the systems can be expected to supervision of a licensed Professional Engineer or Registered Architect. With proper maintenance the systems can be expected to function satisfactorily and are not likely to create an unsanitary condition.

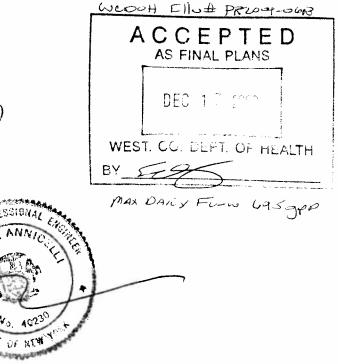
Date: 12/17/09 Approved By: .

Joshua Lipsman, M.D., J.D., M.P.H., Commissioner, Westchester County Department of Health

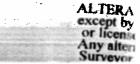
Dave ----- FHAMAA

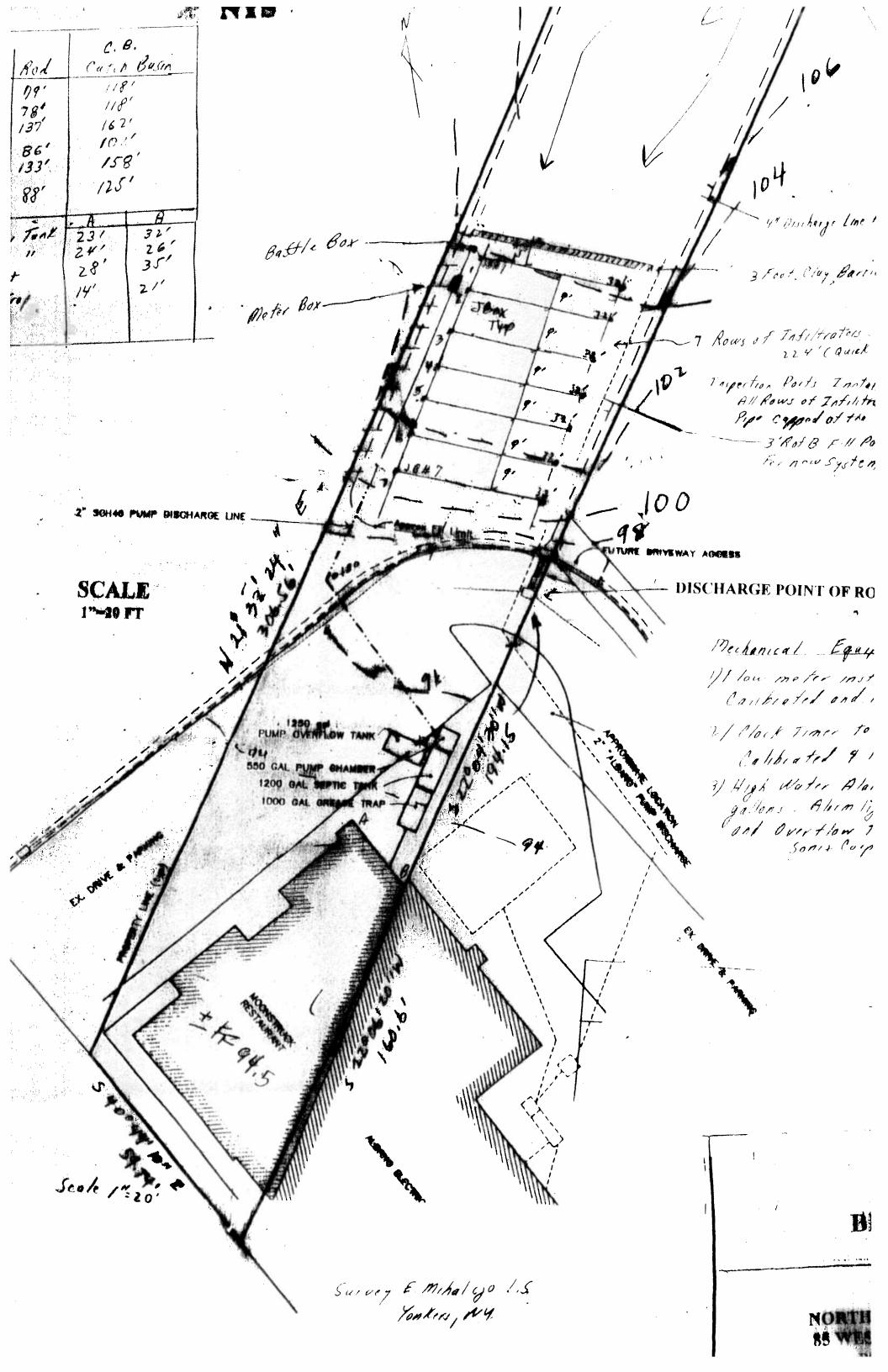
2/ Clock Timer to pump installed in No Star Electric Room. Calibrated & Pumps cach iz hours 310 gal / pumping Centipro Pump Control 3) High Water Alarm in overflow Jank set to 600 gallons. Alarm light & audible in Worth Stor Restourcet and Overflow Junk to be pumped when Alarm activetal Some Corp Level Sensor HL 1000 & Could High Water Alarm -94 Owner of Property F. Accoccella West hester Properties L.P North Star Restaurant 8 5 Westchester Ave Pound Ridge, N.Y. 10576 Manue River Drainge Basin (Acquario, Water Co) JOHN ANNICELL', P.E. TROY LANE 914-273-36 82 BEDFORD, NEW YORK, 10506 LATEST SCALE; 1.5 REVISION: NORTH STAR RESTAURANT 85 WESTCHESTER AVENUE SHOWN POUNDRIDGE (T) 10576 Sect. 15, BIK. 9454; Lat6 SHEET NO. DATED: "As Built" 11/21/09 REMEDIAL SSTS PLAN HECKED 1 OF | SSTS; OWTS-SEPARATE SEWAGE TREATMENT SYSTEM 12/10 W.C. HD Comments 12/02

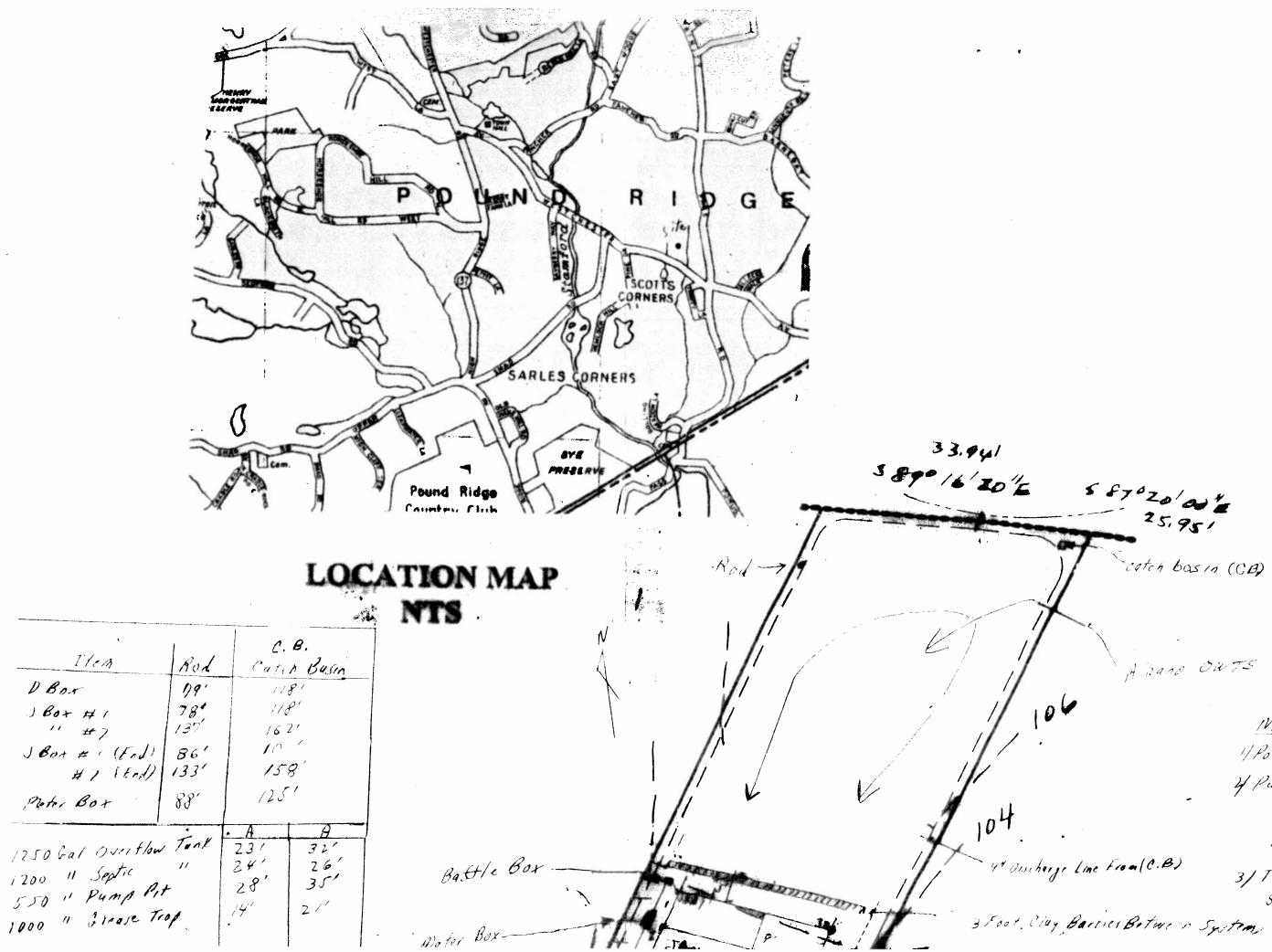
ALTERATION OF THIS DRAWING except by a licensed P.E. or Architect or licensed Land Surveyor is illegal. Any alteration by a P.E., Architect or Surveyor must be indicated and bear his seal, signature and date of alteration.



1) Pot Scrubbing Sink Conneted to Grove Tay 4 Pump Readings to be Fased to W.C. HD. Monthly 1 Water due - Meter in Bathroom - Doily Rodges If Flow To OWTS - Daily Readings 104 3) The Design Professional Engineer certifies the -supply line to North Stur Grill was insprited and 4ª Duscharge Line From (C.B.) not to have any other connections Bastle Box 3 Foot. Clay Barrier Between Systems Meter Box-JONY Tip 224 (Quick & High Capacity) Vol. 3472 gol Rows of Infiltrators Capacity 895 gal. Inspection Ports Installed @ the ends at All Rows of Infilitrators with a Vertical Pipe capped of the Ground Surface - 3'Rof B F. II Packaje Fernow System 00 HARGE LINE UTURE BRIVEWAY ADDERS DISCHARGE POINT OF ROOF LEADERS 1) I low meter installed Mc Crometer Ultra May UMOG (reads 100 gal) Mechanical Equipment Cambrated and read in office of North Star Transmitter EA401 (1A-12) 2/ Clock Timer to pump installed in No Star Electric Room. Calibrated & Pumps cach 12 hours 310 gal / pumping Cintipro Pump Control High Water Alarm in overflam Tank set to 600 PUMP OVERFLOW T 3) High Water Alarm in overflow Tonk set to 600 gallons. Alern light & audible in North Stor Restauroet 550 GAL PUMP MAN 1000 GAL GRIEAS TRAP and Overtion Tunk to be pumped when Alarm activated Sonit Corp Level Sensor UL 1000 & Could High Water Alarm 94



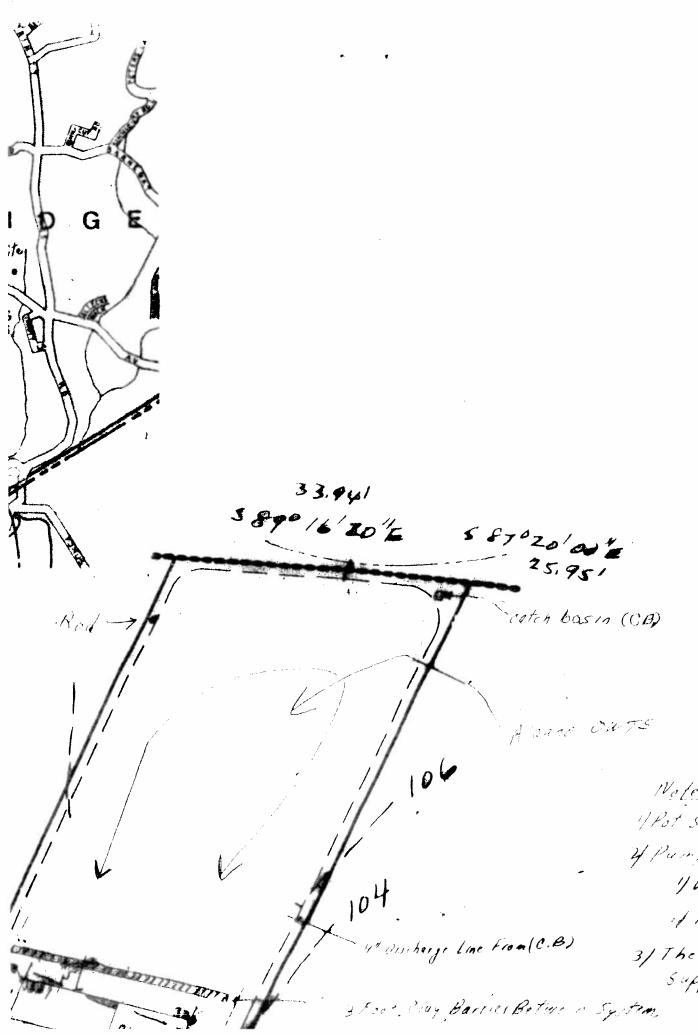




Note The Des the OW with t <u>Pesign</u> Restaura a) Cape B/Pun Pi.

c/Slo, Son Nen

Notes 1/Pot Scrubbing Sink Co 4 Pump Readings to be 1) Water die Me if Flow To OWTS 3/ The Design Prote. Supply line to No. not to hav



Note

The Design Protessional has supervised the construction of the OWTS und certifies to its installation and it is in accordance with the approved plans Restaurant capacity 46 scots @ 35 gol. / sout = 1610 gol/day 4) Capacity of intiltiators 695 gul /Day B/Pump Dose Verified in field. 221/2" Drow = 302 Gal/Dare Pump set to pump every 12 hrs therefore max. Dose to Fields 640 gpd. c/ Slope of OUTS - 1270

Soil Percolotion Rute Iomin/in. New Infiltrators 224 Volume 3472 Gol

Nofes Pot Scrubbing Sink Connected to Grease Trap 4 Pump Realings to be Fared to W.C. H.D. Monthly 1) Water Use - Meter in Bathroom - Doily Readings of Flow To OWTS - Daily Readings 3/ The Design Professional Engineer Celtifies the water Supply line to North Stur Grill was insprited and determined not to have any other connections



9454-7 83 WESTCHESTER AVE -ALBANOS

P.S.D. Town of Pound Ridge Date: Permit <u>8/1/51</u> Approval <u>4-25-52</u> Location: Westchester Avenue Section _____Block: _____Lot: _____ Owner: Alfred Albano, Hickory Lane, Bedford, ^New York Duilder: Herman Coutermash, R.F.D. #5, Ridgefield, Conn. House: three stores Soil test made: 3 minutes <u>Rate:</u> Tank capacity: 810 gallons <u>Material</u>: Masonry Absorption: 87 linear ft. of 24 in. absorp. trench Sketch-Bock: A5-422

No.A5-422 Town of Pound Ridge Alfred Albano, Westchester Avenue 8/1/51 - Herman Coutermash - 800 gal. 80' x 24" ave Westel 6.8 × 40 × 40 = 810 gal · miles well 100 17 Caro 36 10 70 28 SI 100 810 gal marony S.T. 81 LF × 24" abst 8-23-52

WESTCHESTER COUNTY DEPARTMENT OF HEALTH

William A. Holla, M. D., Commissioner White Plains, N. Y.

Issued

Servers Pomdudje

PERMIT FOR SEWERAGE SYSTEM

APPROVAL is hereby given pursuant to Article VII of the Sanitary Code of the Westchester County Herman Coutermanh, Ridgefield, Connecticut (R.F.D. #5) Health District to

to construct or provide a sewage disposal

an 300 gallon masonry septic tank and 30 linear feet of 24 inches system consisting of wide absorption trench

three commercial stores owned by Alfred Albano, Mestchester Avenue, to serve Found Hidge, New York. 4/25/52

for an occupancy of persons, provided that

- I. No portion of the system shall be backfilled or covered until inspected. Inspections are made during regular working hours only. Twenty-four hours' notice is required.
- II. The system shall not be used until it has been constructed in an approved manner, inspected and backfilled, and the written final approval thereof shall have been obtained from the Department of Health. (See Item VIII).
- III. Additional or more adequate facilities shall be provided whenever it is determined by the Commissioner of Health that such facilities are necessary, for which an additional permit shall be obtained.
- IV. This system shall be maintained and operated in complete conformity with rules and regulations for the protection of public water supplies, all applicable laws, local ordinances, and the provisions of the Sanitary Code, existing or hereafter enacted.
- V. When sludge and scum shall so accumulate in any tank as to occupy a depth at any point of more than one quarter of the liquid depth of the tank, they shall be removed and disposed of in accordance with the requirements of the Sanitary Code, and so as to create no nuisance.
- VI. A connection to a public sanitary sewer shall be made whenever such sewer shall become available.
- This permit remains the property of the Department of Health and is revocable at any time or subject VII. to modification or change whenever the Commissioner of Health shall deem necessary.
- VIII. It shall be the responsibility of the person obtaining this permit to deliver a true copy thereof together with a copy of the final approval to the owner of the premises served by this system before this system is placed in use.

HOKO: CAGE

<u>a parta popo</u>

Commissioner of Health

S. D. 5 9-50-21767 Stanford Mater Co. CCI verett B. Snapp. Town Clerk

WESTCHESTER COUNTY DEPARTMENT OF HEALTH William A. Holla, M. D., Commissioner AVISION OF SANITATION Application Rec d. R. M. McLaughlin, P. E., Director Permit Issued..... H. M. Gray, P.E., A. R. Secor Final Approval. R. H. Cummings, P. E., R. W. Germeroth Sahitary Engineers Mr. C. C. a the second APPLICATION FOR RESIDENTIAL SEVAGE DISPOSAL PERMIT (Please_type or print) (See Rules & Reg.Form S.D.22) To the Commissioner of Health: Application is hereby made for a permit to construct a sewage disposal system to serve. 3. COMMERCIAL STORES (Number, type, and use of building to be served.) 1. Owner ALFRED ALBANO Mail Address HICKORY CANE, BEDFORD, OF Y. Note: (Owner must receive permit and approval. Check here for extra Property at WEST CHESTER AVE, POUND RIDGE, NEW YORK 2. (Street) (Village, Town, City) 3. Tax Map Location: Section Block Lot Subdivision 4. Construction: New, Replacement; Proposed Future Building NEU (Expansion attic, etc.) 5. Lot size 100 × 150 No Extra lavatories.....Special Fixtures Maximum Future Occupancy Source of water supply. ARTESIAN WELL 6. Watershed on which system is located Distance to nearest watercourse.....Owner's wells.....Adjacent wells A.S.O. 8. effective depth. 4 ... Minimum liquid capcity - 500 gallons; 200 gallons per bedroom. (MUST BE MADE BY APPLICANT AT SITE) (from table) 10. Absorption area. 2.9.9. *********** SQ. It. gals.waste(No.7) Absorption rate Yrom table bottom area Absorption treatment: Trenches 20. inches wide linear feet. 11. Gravel. 10...cu.yds., to depth of inches below bottom of pipe. Leaching pits: number....outside dimensions....depth below flow line....; wall area below flow line....material.....built-up, rock-filled. Absorption area: trenches,leaching pits.....total......sq. ft. Signature Hermon Coulimnah Title Conota (By owner or person presenting owner's written authorization) Mail permit to Ridge fueld & onn R. F. D. J SKETCH REQUIRED showing all features of property, wells, streams and sewage disposal system. Failure to secure permit before construct tion of the County Sanitary Code and is a misdemeanor. INSPECTION OF COMPLETED SYSTEM BEFORE BACKFILLING IS REQU

S. D. 7 - 9/50

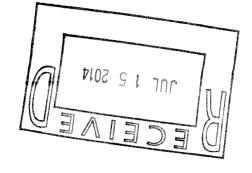


ONSITE WASTEWATER TREATMENT SYSTEM (OWTS) REPAIR AND REMEDIATION DATA FORM

Municipality:						
Property Maili	ng Address (N	lo. & Street): 😕	3 WESTO	-HESTE	E ALE	Zip: 10576
Town/ Village	: POUNI	D RIDGE		State:	NY	7: 1057/
Owner: <u>A</u>	LBANG	> PEALT	1			Lip:
Owner Mailing	g Address (No	. & Street) (if differe	nt):			
rown/ village	•			State:		Zip:
Property Use:	[] Single F	amily [] Multi-Fami	y [] Industrial R	ommercial		Z.ip.
		Describe:				
OWTS Rem	ediation [WCDH File	4.
wastes or offen	sive material	allation, replacement ing failure, resulting on to the surface of t e repairs, as defined	he ground, into a s above, to correct a	torm	vater treatment scharge of sewa	#: system components to correct age or domestic wastes or trade course or water body.
OWTS Dong		• / /• • •	OR			
Ow 15 Repa	ar 🗠 C	omplete the follow	ving information	n.		
treatment syste	ean the repair, m components	maintenance, and re	placement in kind	and in situ;	of broken, dama	aged, or worn onsite wastewater
Number of Bec	irooms	Number of Ba	athrooms:		Water Supply	Type: Public 🗖 Well 🗖
	Please	note below only o	components that	t have been	repaired or	renlaced
Repaired	Replaced				1	- prover
	See See See See See See See Ga Ga Ga Ga Of	puse Sewer or other : ptic Tank#1 Size(ga ptic Tank#2: Size (g nction/Distribution E wage Pump(s) or oth psorption Trench Ler epage Pit(s) alley(s) avelless Trench(es) -A Alternative Syste her Advanced Alterr her System Compon	allons): allons): Box(es) her Dosing Equipn ngth 320 ft. 1 m m bative System	nent X Trench W	OF WO OF TH	V BUILDING AND LOCATION ORK PERFORMED ON BACK IIS FORM
	Entire Sys	stem Replaced				
Contractor's Na	ame (print): U	NITED SEPTIC	EXCAVATION	U Date Ro	nair/Remediati	on Completed: 6 - 18 - 14
Contractor's Si	gnature:	Re	2	License	No.: 109	on Completed: $b = 18 - 14$
Upon completio	on please remi	t to:	an di kana jer		/	*
(1/12)		Westchest	er County Departm 25 Moore Ave., Mt. Kisco, NY ttn: Patricia Torne	1 st Floor 10549		File #: <u>REP</u>
						(WCDH Staff only)

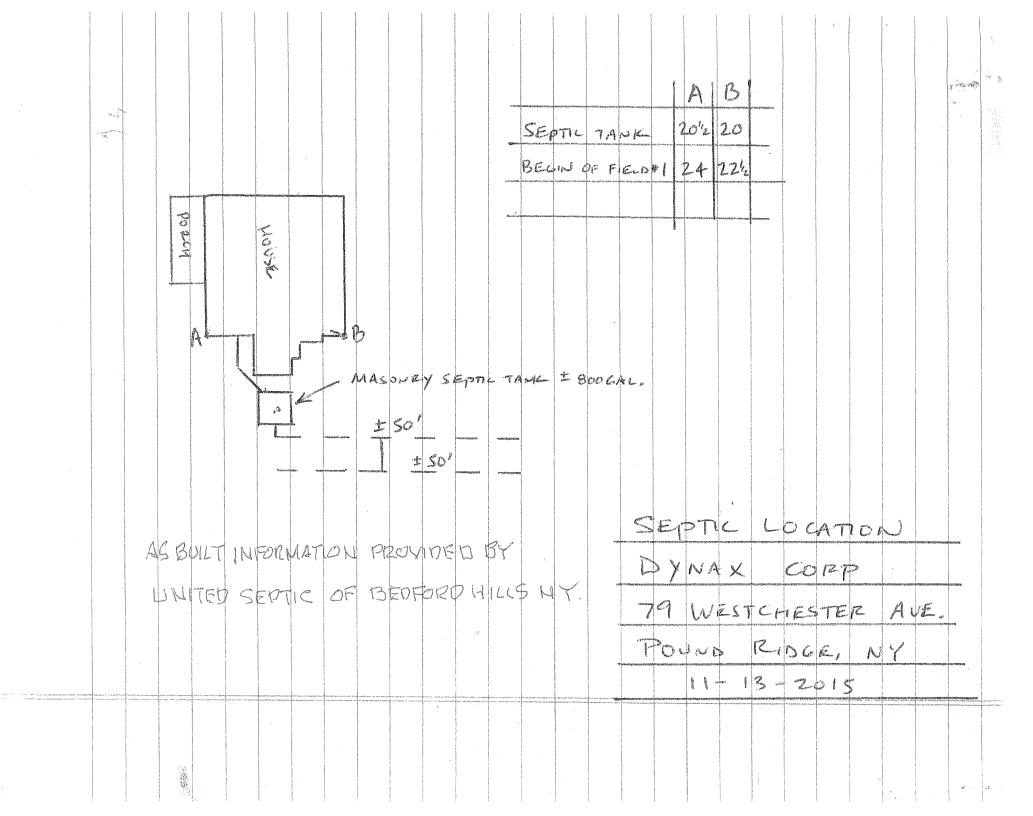
STONE WALL 2000000 Rod=2 Roo#1 2000000 2000)8^{*} ÆF 40 40 40 .Rock 24 tore have 16 40 10/20-36 44 Le to a 40 2 REPLACED EXISTING INFILTRATOR WITH 320 L.F. OF 2' WIDE INFILTRATORS

	Rod #	ROD #2
J-BOX #1	131/2	631/2
END #1	391/2	23
BEGIN #4	35	76
Eno # A	49	42
$J - Box \neq 2$	42'2	811/2
E~0 #5	481/2	49
BEGIN \$ 8	65	99
Ero #8	64	7)



SEPTIC REPAIRS ALBAND APPLIANCE 83 WESTCHESTER AVE. POUND ROGE NY 6-18-14

9454-8 79 WESTCHESTER AVE



9455-10 22 WESTCHESTER AVE

	99-34-28 5	ε.	-11/5 H1-3
eparate Sewage System $A \cup X$.	Send Miles	PR 6	5-5
ERTIFICATE OF COMPLETION	Municipality	W. E.D. H. File	e #
cated of Stiller to haster (4	ve E. Salla 6	Section-Ward	
mer Mildud B Kang	lman	Block	
stern built by farmy Kanen	7	_lot	ž
illding typePermit issued	13 Jan 65	Guarantee	
stem consists ofGalma	asonry, metal septic tank5	Lineal feet X3	Width trench.
ea drainage			and a second
nal grading & seeding: Completed	Waiver	Escrow	
nsanitary condition resulting from such usage. This approval is revocable as soon as a public oner of Health such revocation, modification or change shall be necessary. TRUE COPIES OF THE REMISES AT ALL TIMES AND SHALL BE SHOWN TO ANY REPRESENTATIVE OF THE COMMISSIONER OF 14th proper maintenance this system can be expected to function satisf	IE PERMIT, PLAN OF THE SYSTEM AND OF THIS CERTIFICA OF HEALTH UPON DEMAND. factorily and is not likely to create an u	ATION, AND ANY CHANGES THEREOF SHALL	dgement of the Commis- BE MAINTAINED ON THE
William A. Brumfield Jr., M.	M. D., Commissioner By er County Department of Health	<u>i dvi na</u>	R
X AUX X		221	
	ate Water Supply <u>10</u>	Municipality	/
CONSTRUCTION PERMIT	WCE	DH File No. PR 65-5	5
located at MESTCHESTER AVE - 14MIE.	Jection	2Block	0
Subdivision APUFMATAN BILDRED C.	Lot	lob	
Owner NAUFMANN, MILDRED B. Address	WESTCHESTER AV	E F. R. Lot Area_	6 ARRES
Building Type RAME DWELLING	No. of BedroomsTotal Habi	itable Space /985	Square Fee
Separate Sewerage System to consist of	Gal. Masonry, Metal Septic Tank	5lineal Teet X_3X/8	wighth trench
To be constructed by HARRY C. HAISER JE Jive. (ISTING DELLED NELPHONIC Supply from	Address 878 ALLEY Rol	AD NEW CAVAA	A Co-VA E
i done dappiy non		the second se	N.
Philipping Discounts of the second second	Address	i de la companya de la	
Private Supply to be drilled by			v)
Other Requirements			·

well described above will be located as shown on the approved plan and that said well will be installed in accordance with the standards, rules and regulations of the Westchester County Department of Health. 15 JAN.65 Date

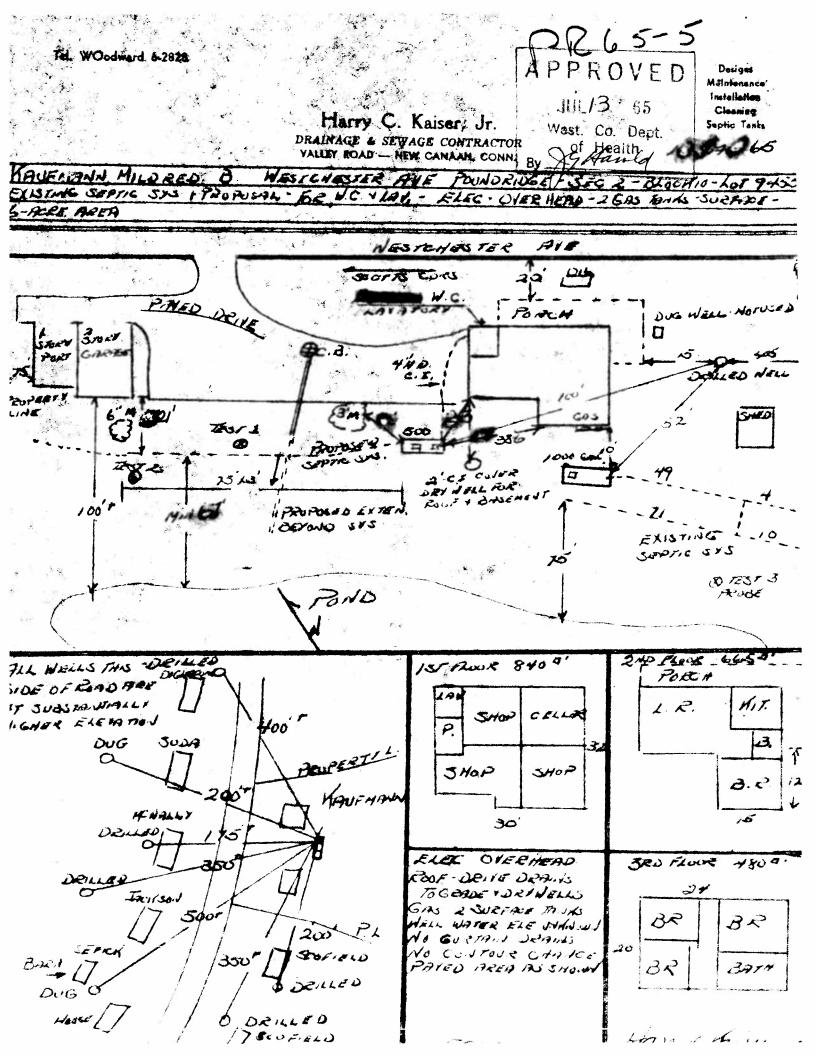
Kaufn Br mild Signed.

APPROVED FOR CONSTRUCTION: This approval expires one year from the date issued unless construction of the building has been undertaken and is revocable for cause or may be amended or modified when considered necessary by the Commissioner of Health. Any change or alteration of construction requires a new permit. Approved for disposal of domestic sanitary sewage, and/or private water supply only.

6 3 will be Date SD 46.64

William A. Brumfield, Jr., M. D., Commissioner Westchester County Department of Health

3 lawlel 8y



OUNTY OF WESTCHESTER DEPARTMENT OF HEALTH - Division of Environmental Sanitation
DESIGN DATA SHEET - SEPARATE SEWERAGE SYSTEM 14 MI ETAST SCOTTS CORS FILE NO. <u>PR6555</u>
Loost ad At (Street) NEST HESTER HVE MUNICIPALITY / UNID RIDGE /V/
Owner HAUFMANN, MILDRED B. Sec. 2 Block 10 Lot 9455
Present Mail Address WESTCHESTER AVE POUNDRIDGE N.Y.
Watershed STAMEDRD CONN. Lot Area 6 A S.D.Usable Area 5000 +
Water Supply: Drilled X Driven Dug Well : Depth ? Public
No. of Rooms 19 Bedrooms 4 Future: Yes No X Other
Septic Tank Capacity (From Table. Item 5.1) 500 Gals. Masonry X Metal
Soil Rate Used Min/l" Drop: Soil Perc. Test Data Test Pit Data
Soil Rate ApprovedSq.Ft./Gal. Checked ByDate
Absorption Area Provided ByL.F. x 24" 36" width trench
TRADE DIANG AND PROFILES OF SEWERAGE SYSTEM REQUIRED DRAWN TO SCALE OF NOT MORI
THAN 1" TO 20' HORIZONTAL AND 1" to 10' VERTICAL FLOOR PLAN OF BUILDING (REQUIRED) SEPARATE SEWERAGE DISPOSAL SYSTEM PROFILE
PLOT PLAN SEPARATE SEMARATE DEFORM ON PLANS Check off items required to be shown on plans Identification I. Identification (Name-Title)
OWNER
MAIL ADDRESS 878 Walley Lond TELEPHONE NUMBER 966 2828 S.D. 7.1 - 1962 No. J. Canoan, Com.

- Location M. B. HAVF MANN - WESTCHESTER AVE TOUND RIDGE

WESTCHESTER COUNTY DEPARTMENT OF HEALTH R65-5SOIL PERCOLATION TEST DATA REQUIRED TO BE SUBMITTED WITH APPLICATION

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	1 2	3:05	3:55	50	26	1/1	614	5	1 1	10
••••••••••••••••••••••••••••••••••••••	' <u>3</u>	4:05	4:25	20	26	<u>.</u>	514	3/4	t	6-
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2	<u>' 1 7</u>	ROBED	105	<i>4″-</i> (G ROU	ND	WA	TER I	<u>97 4</u>	49"
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	' 5	1		1	1	+		t	1	1

Notes:

- 1) Tests to be repeated at same depth until approximately equal soil rates are obtained at each percolation test hole. All data to be submitted for review.
- 2) Depth measurements to be made from top of hole.

pm65 _____Date__/4 Tests made by

Hole # / Saturated - Water absorbed 35 41N. S-46-A (9-18-62)

Job Location

PTH	HOLE NO.	HOLE NO. 2 PROBED TO 4 SOD	HOLE NO. <u>3</u> 26" PROBED TO	HOLE NO
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12"	<u>}</u>			
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24"	LOAM TO CLA.	YMAX		
30 "	alpanantalista dala biochi su parta su tagan da parta da		HARD BENED CLA	<u>y</u>
36"				
42"				
48"		GROUND WA.	TER	and a second and a s
54"	division that the stand many balance and some operation of the stand many some stand many some stand many some	GROUND WA. GROUND WAS	122	an a
60 n	10000000000000000000000000000000000000			anter anter and a start a start a start a start a start a start
66 "				
72"				
78"				
84 "				
	LEVEL AT WHICH	GROUND WATER IS EN	COUNTERED	

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S.D. 27.6 8.14.63

9455-21 34 WESTCHESTER AVE

Separate Sewerage SystemPrivate N	Water Supply	Pos	T. K	403 6.50
CERTIFICATE OF CONSTRUCTION COMPLIANCE	e . L	N N	n_{10} n_{3}	-30 HO)
A CALL A ST		WCDH File No.	PR 13	HIP
Swnerlalambi q & Mustrumuvou		Section 2 / 2	Block	<u> </u>
	1.		dollob	e 1 - 1 - G
eparate Sewerage System built by	Address	2 G	OK LEG	
Consisting of Gal. Mason	nry, Metal Septic Tank	lineal féet	X - ZHEVF	width trench
Other requirements	***************************************			
/ater Supply: Public Supply From		teres.		- <u>L, j.</u>
	Address		<u> </u>	
uild g Type Number of Bedrooms_		Date Permit Issued	1 - <u>A A 1-19</u>	
rosion Control Completed		Waived		84
ther Requirements				
ith the standards, rules and regulations, plans filed, and the permit issued by the Wes ateCertified By	1/10/11	en M.		
ecome_null and void when a public water supply becomes available. Such approvals evocation, modification or change is necessary.	soon as a public sanitary sen are subject to modification	wer becomes available and or change when, in the jud	the approval of the priva gment of the Commissi	the meters and the shaft
when usage. Approval of the separate sewerage system shall become null and void as s become₂null and void when a public water supply becomes available. Such approvals evocation, modification or change is necessary. With proper maintenance these systems can be expected to function satisfa hate May 1, 1175	soon as a public sanitary set are subject to modification actorily and are not like ., Commissioner By Port	wer becomes available and or change when, in the jud ely to create an unsanit	the approval of the priva gment of the Commissi ary condition.	ite water supply shall oner of Health, such
when usage. Approvan of the separate sewerage system shall become null and void as specome, null and void when a public water supply becomes available. Such approvals evocation, modification or change is necessary. With proper maintenance these systems can be expected to function satisfa	soon as a public sanitary set are subject to modification actorily and are not like ., Commissioner By Port	wer becomes available and or change when, in the jud ely to create an unsanit	the approval of the priva gment of the Commissi ary condition.	ite water supply shall oner of Health, such
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Separate Sewerage SystemPrivate N ONSTRUCTION PERMIT hocated at <u>Mesterage Ave</u> holdivision wher ilding Type parate Sewerage System to consist of parate Sewerage System to consist of Separate Sewerage System to consist of	soon as a public sanitary set are subject to modification actor ily and are not like Commissioner By rtment of Health Water Süppiy 	wer becomes available and or change when, in the jud ely to create an unsanit antil Teons Bection WCDH File No. Section 10 Lot 20 Lot L	the approval of the priva gment of the Commissi ary condition.	Arrow Square Feet
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Utility age: Approval of the separate sewerage system shall become null and void as secone null and void void as secone null and void as secone null and void as secone	soon as a public sanitary set are subject to modification actor ily and are not like Commissioner By rtment of Health Water Süppiy 	wer becomes available and or change when, in the jud ely to create an unsanit antil Teons Bection WCDH File No. Section 10 Lot 20 Lot L	the approval of the priva gment of the Commissi ary condition.	Arrow Square Feet

structed as shown on the approved plan or approved amendment thereto and in accordance with the standards, rules and regulations of the Westchester County Department of Health, and that on completion thereof a "Certificate of Construction Compliance" satisfactory to the Commissioner of Health will be submitted to the Department, and a written guarantee will be furnished the owner, his successors, heirs or assigns by the builder, that said builder will place in good operating condition any part of said sewage disposal system during the period of two (2) years immediately following the date of the issuance of the approval of the Certificate of Construction Compliance of the original system or any repairs thereto; 2) that the <u>drilled</u> well described above will be located as shown on the approved plan and that said well will be installed in accordance with the standards, rules and regulations of the Westchester County Department of Health.

Date 3/2-8/73

Jen Alexan

APPROVED FOR CONSTRUCTION: This approval expires one year from the date issued unless construction of the building has been undertaken and is revocable for cause or may be tary sewage, and/or private water supply only.
Date 1/1/973 Jack J. Goldman M. D. Commissioner

SD 47.66

Jack J. Goldman, M. D., Commissioner Westchester County Department of Health

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CCUNTY CF	WESTCHESTLR DEFARTMENT OF HEALTH - Division of Environmental Sanitation	n \
DESIG! DAT	TA SHILT - SEI ARATE SI II AGE SYSTEN FILE NO	
Owner J.	Columbo q Maitremaure Address Writchester Ave	
	(Street) Weiter Ave @ Sec @ Block 941 Flot 121	

(Indicate nearest cross strect)

Municipality Pound Kidy (T) Watershed_

SCIL PLECCLATION TEST DATA RECUI ED TO BE SUBLITTED WITH APPLICATION

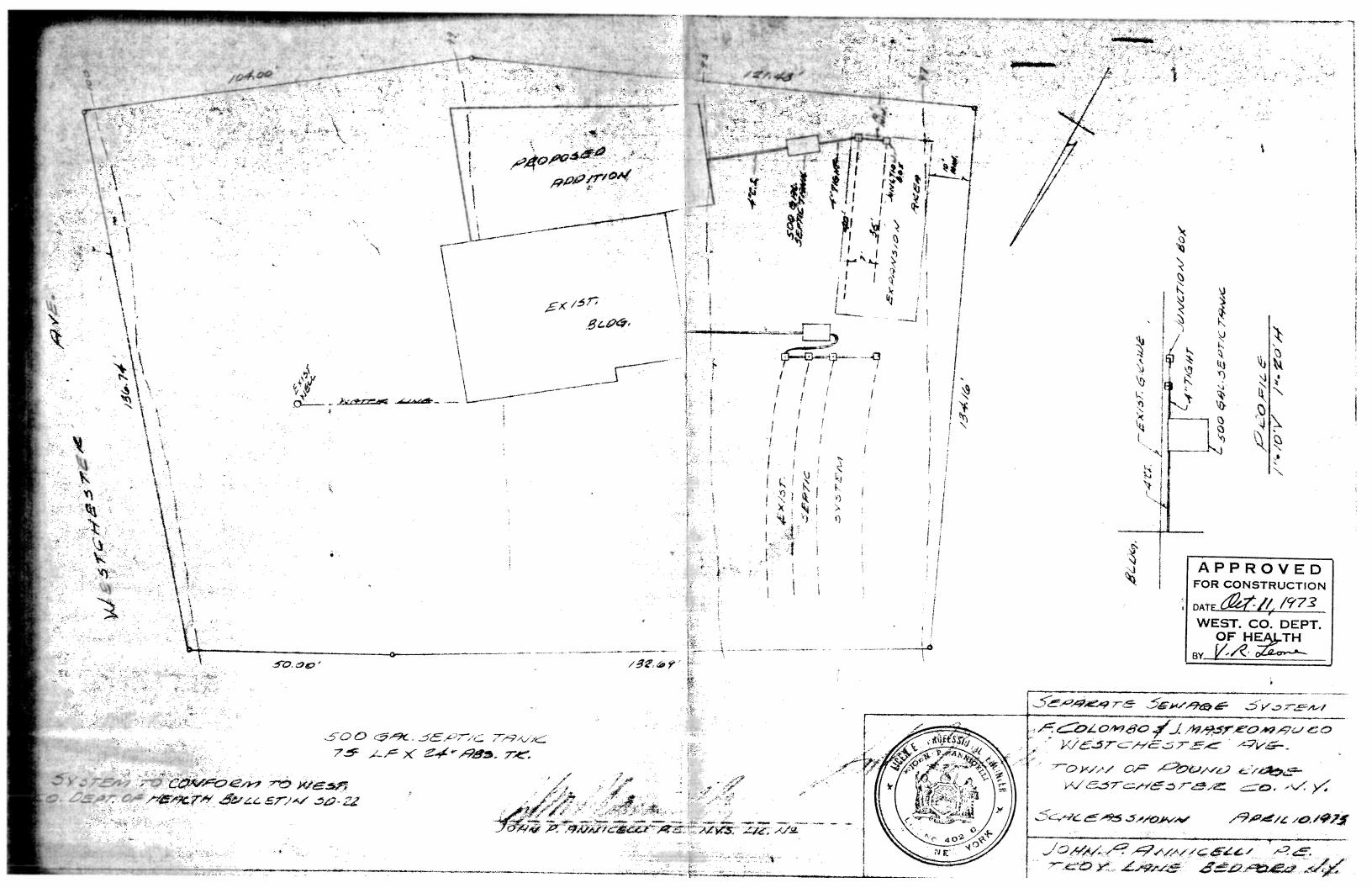
Hole Number	1	CLOCK 1	PT.F	*****	1 1 DEBU	OT ART ON	na de la construcción de la constru	
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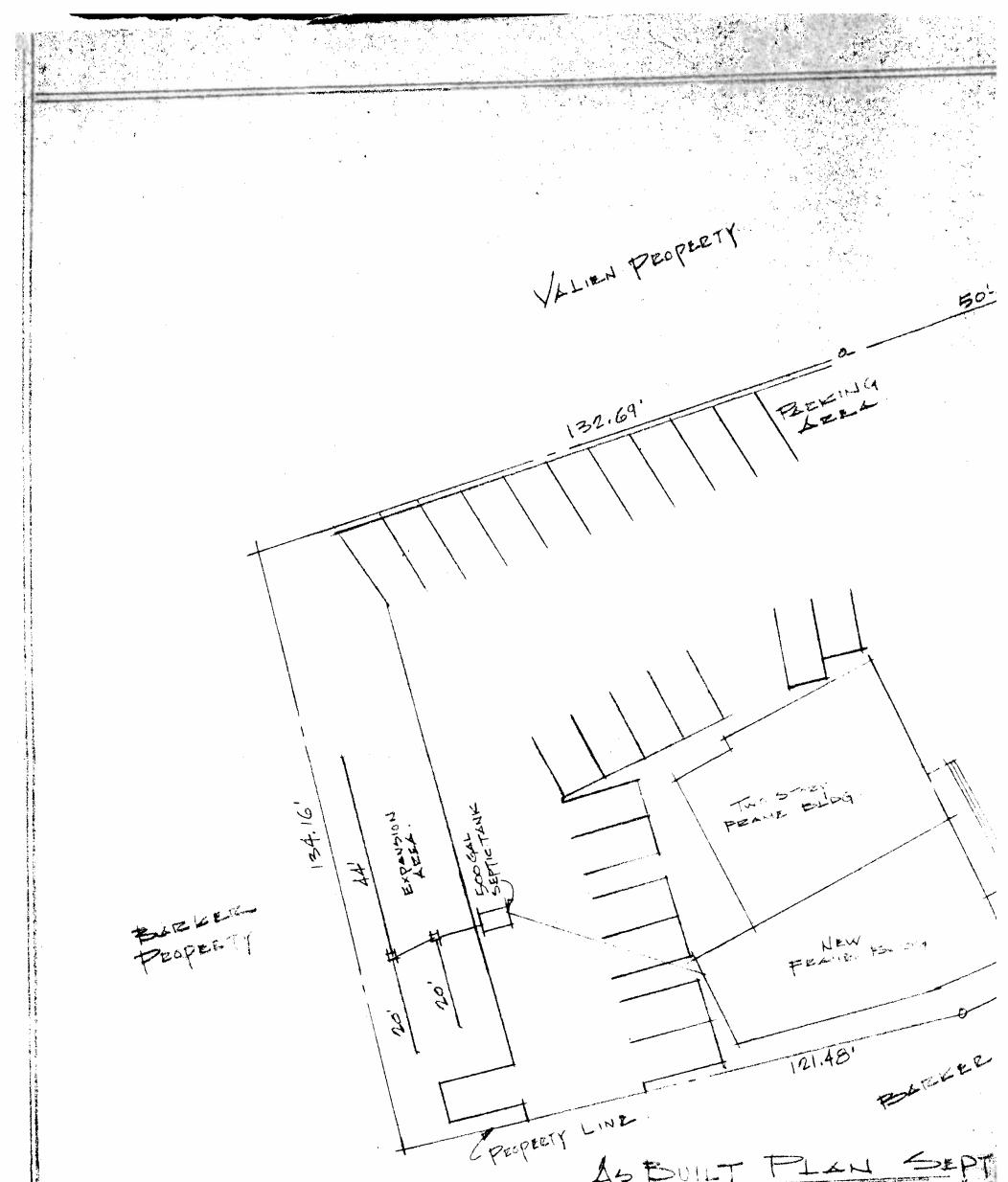
Notes:

1) Tests to be repeated at same depth until approximately equal soil rates are obtained at each percolation test hole. All data to be submitted for review.

2) Depth measurements to be made from top of hole.

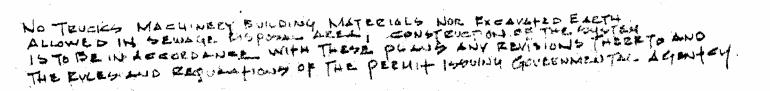
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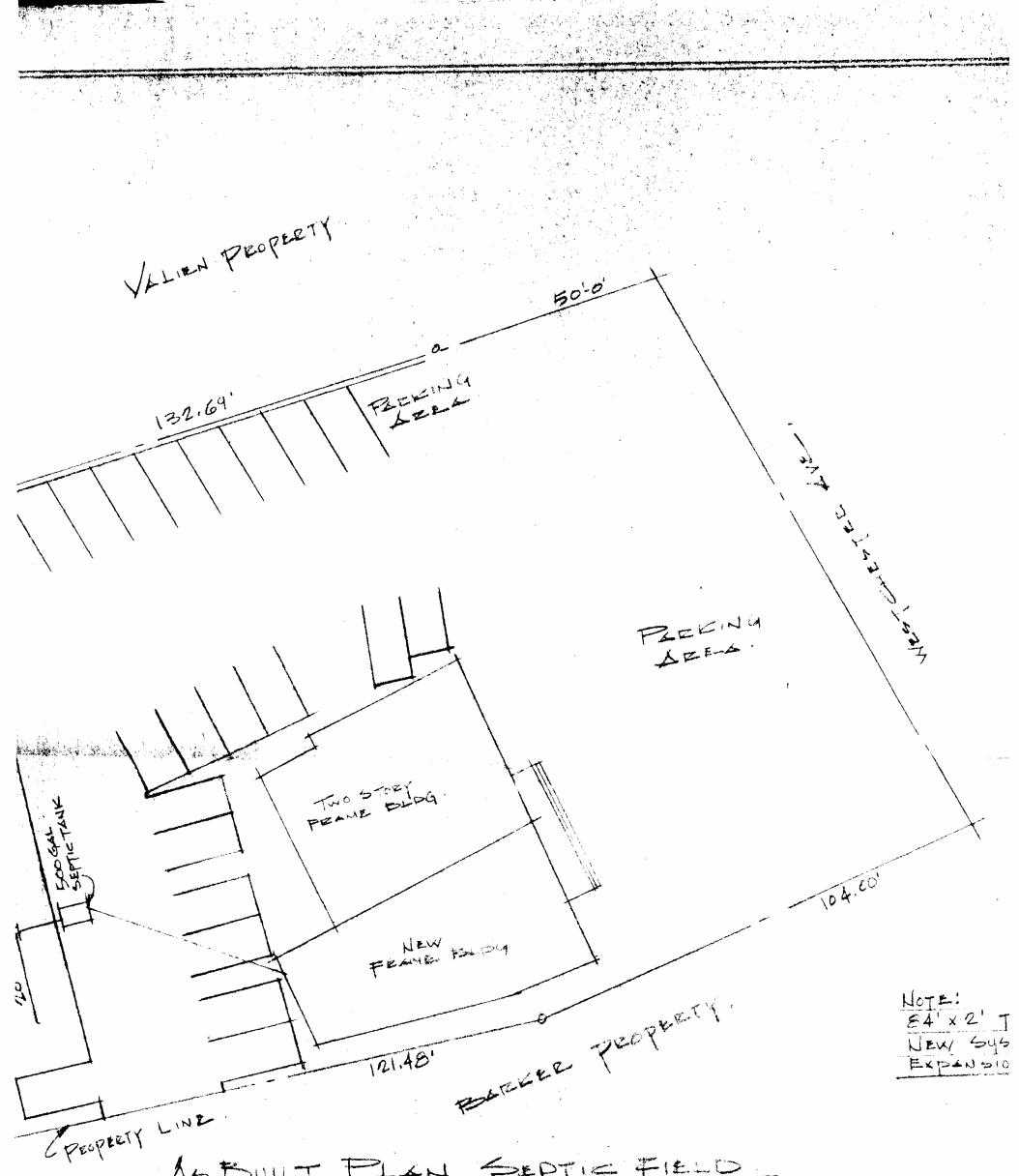




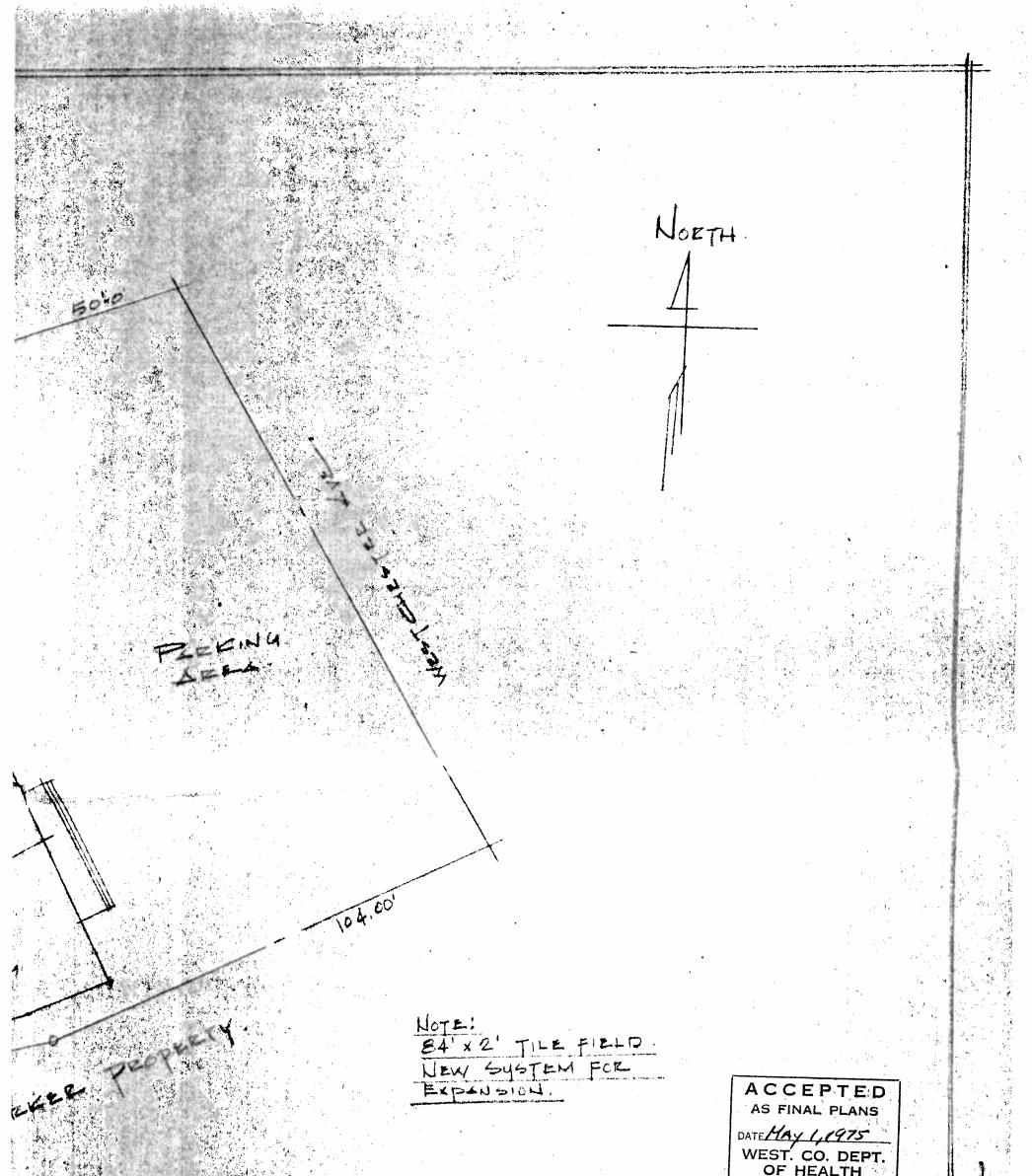
ASBUILT PLAN SEPT SEALE 1

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PTIE FIELD ASBUILT PLAN >EP LO NOR EXCAVATED EARTH CONSTRUCTION OF THE FUNTER - MISS ANY REVISIONS THERE TO AND MIT 1-DOUBLY GOVERNMENTAL ACTINGEY. **1** 11 1. 1. k #25 57 P



OF HEALTH BY V.R. Leone PTIE FIELD 1"=20-0" FEOPERTY OF FEDLUMBO \$ J.MASTROMAURO LOCATION : WESTCHESTER AVE POUND ELDGE NEW YORK PATE APRIS, 1975 SEALE 1: 20 JOHN P ANNICELLI PE. TROY LANE BEDFORD NY

9455-25 54 WESTCHESTER AVE

Separate Sewerage System_	Private Water Su	pply Pour	11 11 404
CERTIFICATE OF CONSTRUCTION	COMPLIANCE 94-3	Mu 5-18 WCDH File No.	PR 75-25-1
cocated at	ro 1 / 1 million	Section	Block 99
Owner Languell Allor	And have been a series	Lot2 /	Job
Separate Sewerage System built by	Partiers Address	Ala topl	a part
Consisting of	Gal. Masonry, Metal Septi	c Tank lineal feet X	3.6 width trench
Other requirements			
Water SupplyPublic Supply From	Fortas		· · · · · · · · · · · · · · · · · · ·
Private Supply Drilled By	1-4 II J	Address	
Building Type	Number of Bedrooms	Date Permit Issued	Qet. 8,1175
Erosion Control Completed		Walved aneranes	i. i
Other Requirements		Press Dage EDSIGNAL	Emple
I certify that the system(s) as listed serving the above pre with the standards. rules and regulations, plans filed, and			s of which are attached), and in accordance
Date	Certified By	and the	
Any person occupying premises served by the above syste such usage. Approval of the separate sewerage system st become null and void when a public water supply becom revocation, modification or change is necessary, said mo	all become null and void as soon as a publi es available. Such approvals are subject to	ic sanitary sewer becomes available and th modification or change when, in the judg	e approval of the private water supply shall ment of the Commissioner of Health, such
With proper maintenance these systems can be ex Date July 26,1476	pected to function satisfactorily and	21 to -	[2] S. SARRADANS, AND AND A DEPARTMENT OF VALUE, AND AND A DEPARTMENT OF DEPARTMENT.
	stchester County Department of H		
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TY OF	WESTCHESTER	DEPARTMENT	of	HEALTH -	Division	of	Environmental	Health	Services

DESIGN DATA SHEET - SEPARATE SEWERAGE		FILE NO	1.1.41.54		
Owner Barnwell Associates	Address	Martch	ty	Ang	
Located At (Street) Westchester	Der	Sec 8			24
(Indicate nearest of Municipality Round Ridge N	cross stre	et)	· · · ·		ž

SOIL PERCOLATION TEST DATA REQUIRED TO BE SUBMITTED WITH APPLICATION

National	CLOCK	TIME		PERCO	LATION	e en stationer en e	PERCOLATION
		Stop	Elapse Time Min.	Depth to Wa	ter Surface Stop	Drop in	'Soil Rate 'Min/in.drop
1	11:00	11:25	36	17	24	3	***** 12
• •		 * NARASSEL 	33	· 17 ·	24	2	· 17
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4 '				and the second sec			
51							
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Notes:

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1) Tests to be repeated at same depth until approximately equal soil rates are obtained at each percolation test hole. All data to be submitted for review.

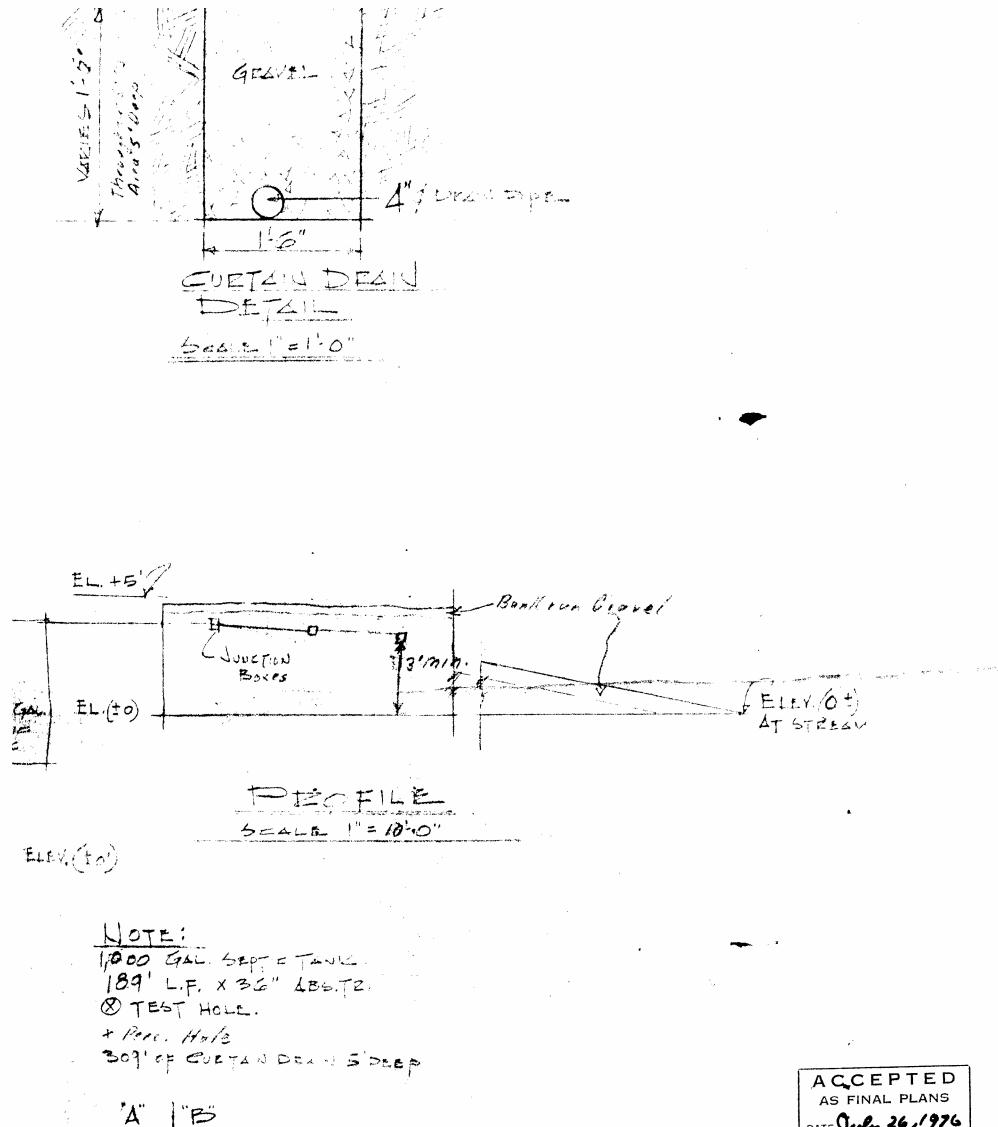
2) Depth measurements to be made from top of hole.

TEST	PIT	DATA	REQU	JIRED	10	BE	SUBMITTED	WITH	APPI	ICATION
	DESC	CRIPTI	ION C	DF SOI	<u>[LS</u>	ENC	COUNTERED	IN TE	ST EC	DLES

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DEPTH	HOLE NO. /	HOLE NO	HOLE NO	HOLE NO.
G.L.	Top Joil	-		
['] 6"	11			
, 12"	Sundy Lung	2	·	
18**	[1	• • • • • • • • • • • • • • • • • • •		
24**	11	-		
30"	1(
36"		·		
42"	11.			
48*	11			
54**		w. Clay		
60"		11		
66"		11		
72"		"		ч.
78"	11	11		
84"	4	11		
INDICATE	LEVEL AT WHICH GR LEVEL FOR WHICH W DE BY	ATER LEVEL, RISES	AFTER BEING ENCO	OUNTZRED 3 1
Soil Rat	e lised 11 - 15	DESIGN	I S.D. Hashla Ana	Double Provided Orra
No. of B	edroome 600 jalloni	- Idin I Drop: Idoj	S.D. USADIE Area	hason Metal
	on Area Provided B		1	
Name			Signature	the total m
Address	John P. Troy La. Bedlor	Annice 1 8296	SEAL X	No. 40230 VOIN HERE
Westches	ter County Health	Department	10	CB .
Soil Rate	Approved	Sq.Ft./Gal.	Checked by	Date

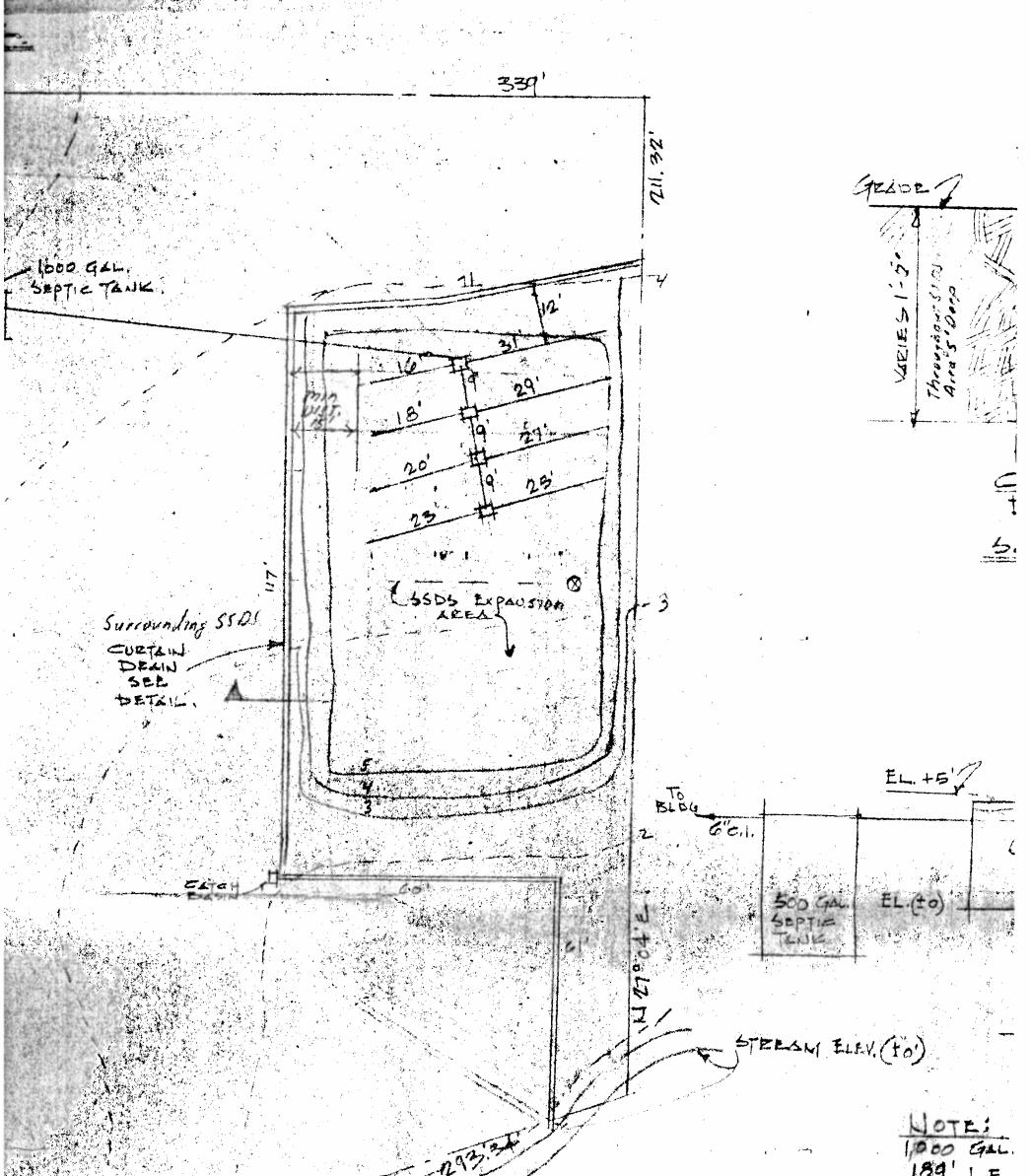
S.D.27.6 (Rev. 5-22-73)



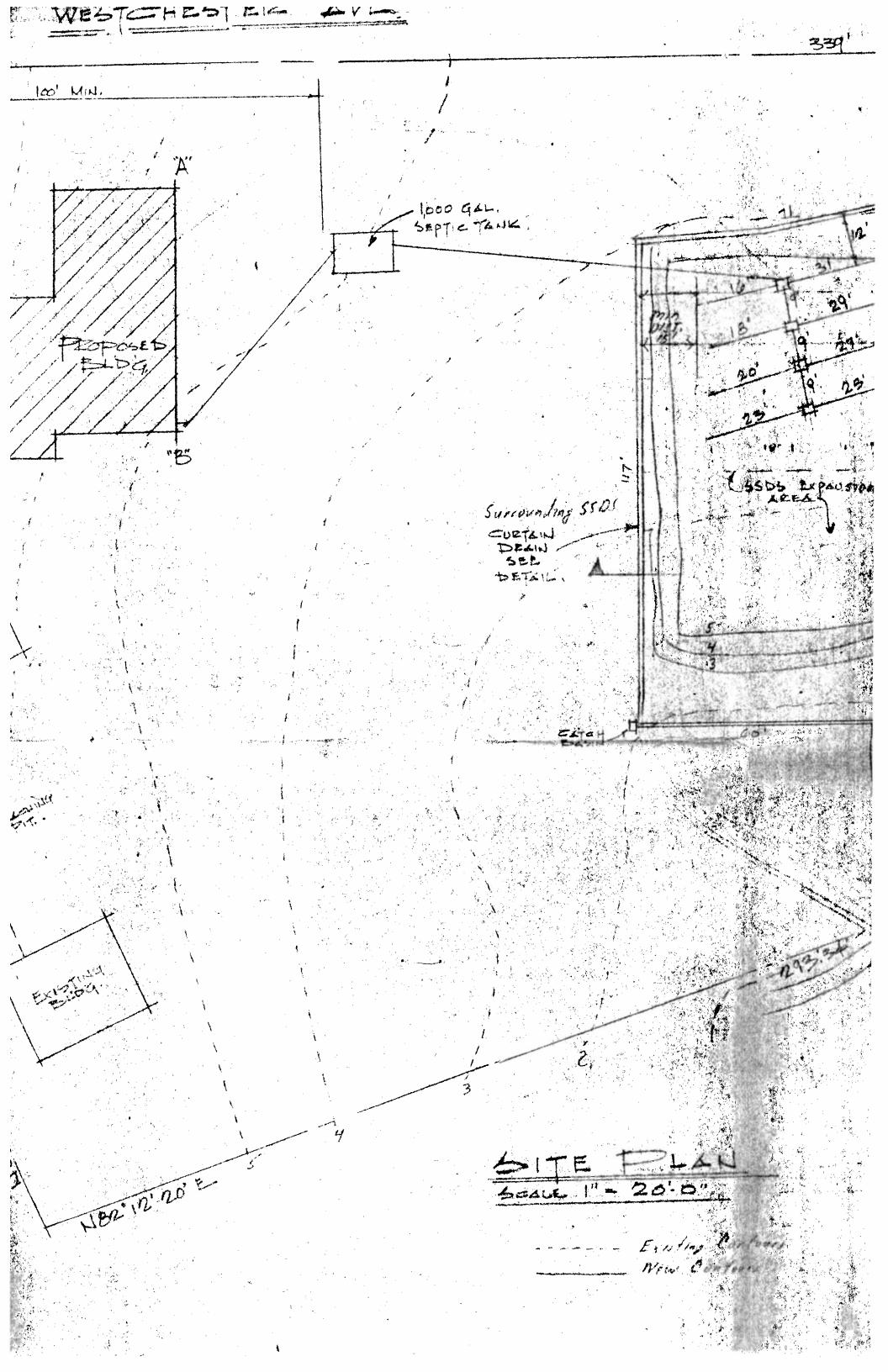
"TIC TENK 42' 54' "TICN BOX"1 152' 143: G" TION BOX "4 161' 154! 6" ACCEPTED AS FINAL PLANS DATE July 26/976 WEST. CO. DEPT. OF HEALTH BY

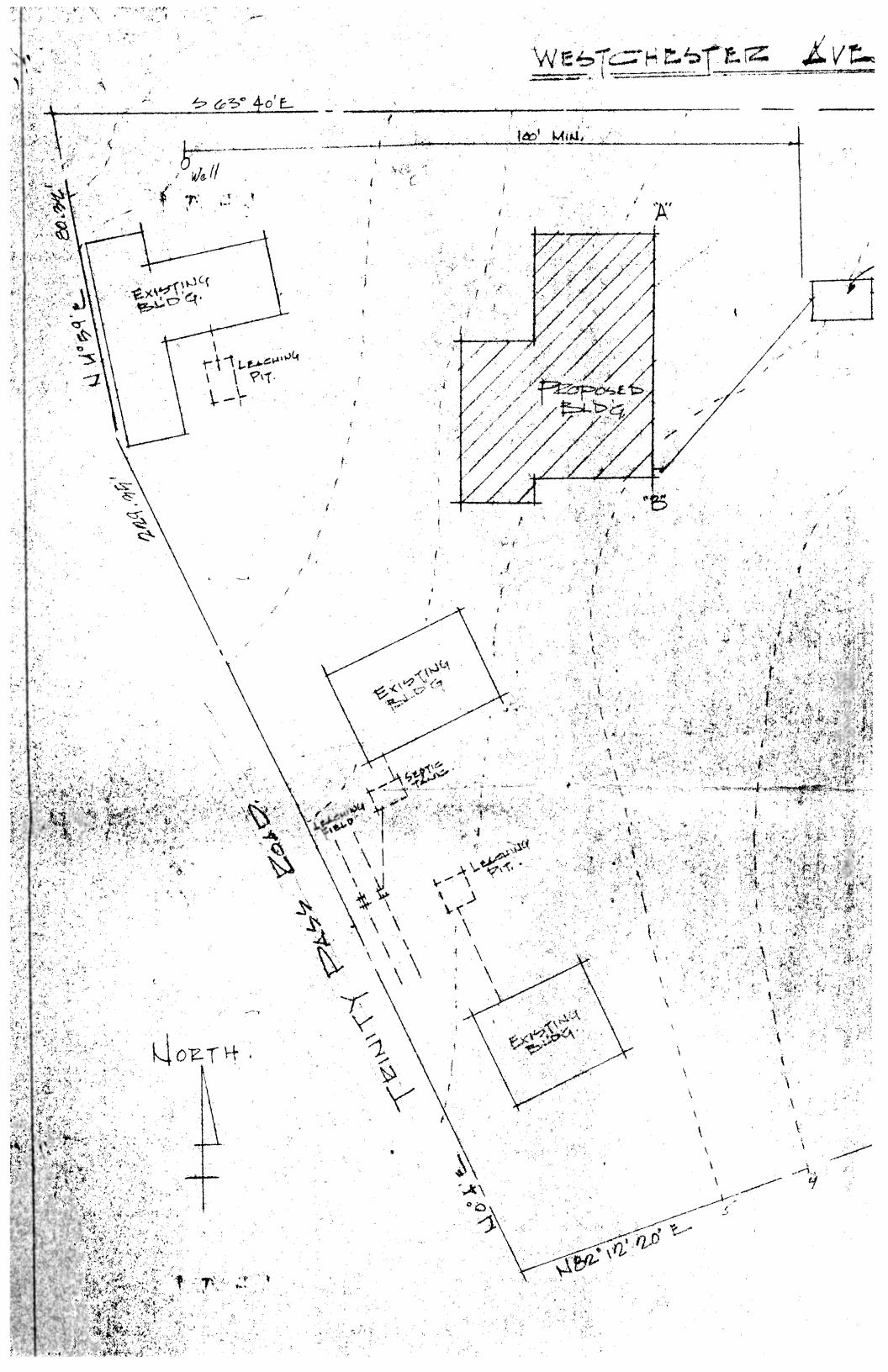
REVISION AS BUILT JULY 13, 1976. BARNWELL 1.44005. SEPERATE SEWAGE SYSTEM LOSATION: W'CHESTER ED & TRINITY PASSED FOUND RIDGE NEW YORK SECTION: 8 BLOCK: 9455 LOT: 24 DATE SEPTIO, 1975 SOULE AS NOTED John P. Annicelli, P.E. Trov La. Befford, H. T. 10506





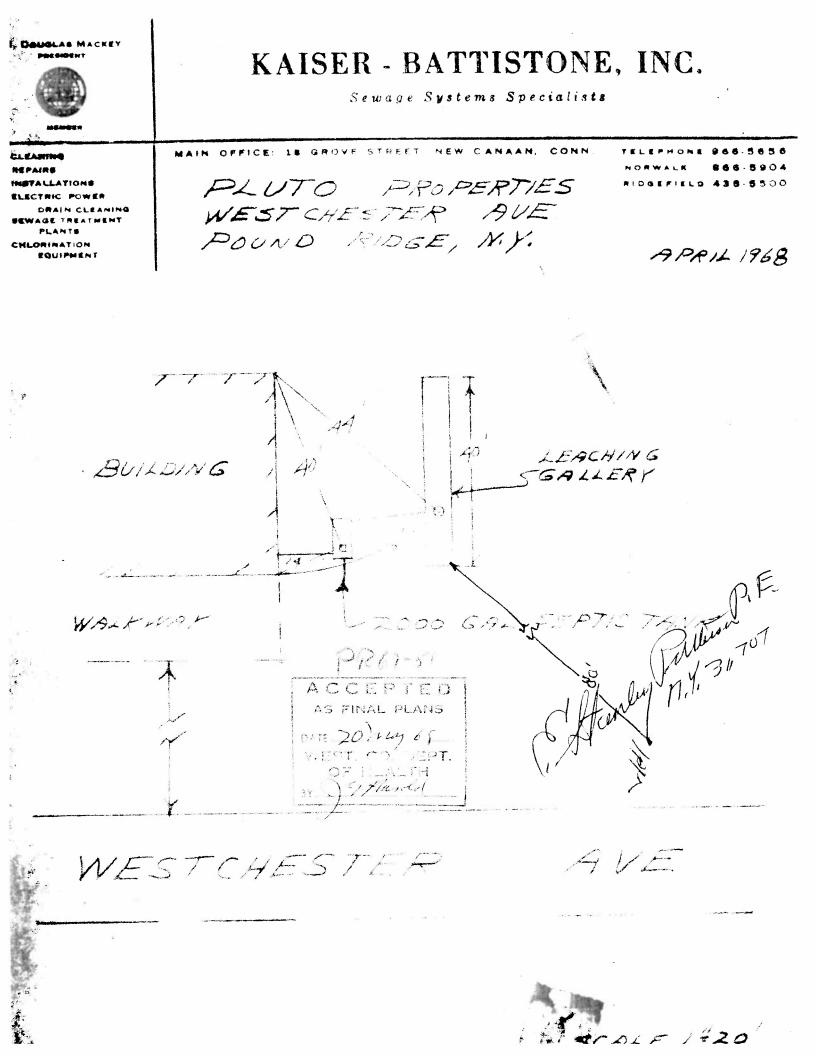
109 L.F. O TEST H + Perc. Ho Bog' of Cu Ż, Å P 5. SEPTIC TENE 42' 5. JUNCTION BOX"1 152' 14. JUNCTION BOX "4 167' 151 F LAN 1" - 20'-0" SEALE Existing Confurry New Contraction





9455-25 54 WESTCHESTER AVE

Existing
Separate Sewerage SystemPrivate Water Supply
CERTIFICATE OF CONSTRUCTION COMPLIANCE WCDH File No.
Mpschester AUC Section 2 Block 9455
Plute Provides 25
Separate Severage System built by 07110 V5 offer Fres Address 32 ME Dougal Dr, No. W. M. M. M.
Consisting of Gal. Masonry, Metal Septic Tank lineal feet X width trenth
hine - Acarding Callery 40x5x3
Other requirements
Water Supply:Public Supply From
Private Supply Drilled By Address
Staves lame and by E
Building Type Date Permit Issued
Erosion Control Completed Waived
Other Requirements Burness using min and water only
I certify that the system(s) as listed serving the above premises were constructed essentially, as shown on the plans of the completed work (copies of which are attached), and in accordance
with the standards, rules and regulations, plans filed, and the permit issued by the Westchester County Department of Health.
nate May 20 1968 cartilled By . Tanky matters in SETE
Any person occupying premises served by the above system(s) shall promptly take such action as may be necessary to secure the correcting of any ungapita o wonotions resulting from
such usage. Approval of the separate sewerage system shall become null and void as soon as a public sanitary sewer becomes available and the approval of the private water supply shall become null and void when a public water supply becomes available. Such approvals are subject to modification or change when, in the judgment of the commissioner of Health, such
revocation, modification or change is necessary.
With proper maintenance these systems can be expected to function satisfactorily and are not likely to create an unsanitary condition.
Date 20 may 6 5 William A. Brumfield, Jr., M. D., Commissioner By J. Hauld
SD 47.64 Westchester County Department of Health



COUNTY OF MESTCHESTER DEPARTMENT OF HEALTH - Division of Environmental Sanitation DESIGN DATA SHEET - SEPARATE SEVERACE SYSTEM FILE NO. PHG7-S 41.0 Owner / in porties fuc Address 32 9 C . $d_{\rm LC}$ 1/2 40 Located At (Street) e si Chester Sec. 2 Block 9953Lot 25 (Indicate nearest cross st t) 4. Municipality oung am Watershed 2011 SOIL PERCOLATION TEST DATA REQUIRED TO BE SUBLITTED WITH APPLICATION

Hole Number	r CLOCK TILE			PERCOL	ATION		PERCOLATION	
	Run No.	Start	Stop	Time	Depth to W	later	Water Leve in Inches Drop in Inches	Soil Rate Min/in.drop
	1	12/27	12/37	10	191/2"	201/4"	3/4*	13 Min
	2	12:37	12:47	10	201/4"	203/4	1/2 *	20 Min
	3	12447	12:57	10	203/4	211/4"	1/2"	20 Min
alland and a state of a	4	12:57	1:07	10	21/4	21314	Yz 4	20 Min
·	5							
	1					 		
	2						1994999°	
	3						oon, sido retare oo saa adaa dagaa dagaa ayaa ayaa ayaa ayaa	
	4				a an		*******	
	5		**************************************	and a second with the second with a second with a			600870070770770070000000000000000000000	
	. 1					an Malanda, andre vierten an enteren an enter	nye mastan data atalaka kana data atala data data data data data da	
	2	anne-Antonia aite Antonia ann an Anna ann an Anna Anna ann an Anna Anna Anna Anna Anna Anna Anna Anna Anna Ann Anna Anna			and a second an			2 James of a state of the st
	3							
	4						an -algebilder-skarsteller i ogeninden skaligielde ogen-seg	
umulium ortagodiktekkanaktapuna zao	5					1		
Nota				1	H	Leona		

Notes:

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1) Tests to be repeated at same cepth until approximately equal soil rates are obtained at each percolation test hole. All data to be submitted for review.

2) Depth measurements to be made from top of hole.

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	TEST PIT DESCH	DATA REQUIRED TO RIPTION OF SOILS	BE SUBMIT ENCOUNTER	ED IN TEST	HOLES	4 14	é > =	
DEDOGU		HOLE NO.	2		HOLE NO.	. 	•	
DEPTH	6 * Top						ŧ	
G. L.	<u> </u>	11		N	z k Z s			
6 n	Yollow Sab	s!/						
12"		11						ġ
18"		in a second s		an de antinense. En en en gene fregenski perskan fregenska fregenska fregenska fregenska fregenska fregenska fr			٩	
24"	·!						e	
30"	Comp	an a				an a		
36"	- pact	Arcs announserationspace estimation contraction and announcement	anna an ann an an Ann an A			un nach mättelinde standiskon nätte ind som hälfsonsättende		
42 [#]	- Jan	1	- 	n nin data karang manang karang ka		ar an		
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54"	¥.			alaata madaana ka		1999 - 1990 - 1990 - 1990 - 1999 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 - 1990 -		
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66"				an a	and the factor of the system is a set of the system of			
72"						an a decembra por la decembra de composition de la composition de la composition de la composition de la compos		
78"	(No Whate	er)	an a	an de la grand	and the second se	an ang ang ang ang ang ang ang ang ang a		
84"	Rock	di'		alainean ar an		Alfred and a state of the sta	500 (1997)	
INDIC	LATE LEVEL AT W	HICH GROUND WATE	R IS ENCO	UNTERED	COUNTERED			~
INDIC	CATE LEVEL TO W	HICH WATER LEVEL	s rioro ar	DATE		a giring aga an ta saga saga na an ang ang ang ang ang ang ang an	adoxNigheria	é
	and a second				ima Dratic	and EACC	al `	
Soil	Rate Used	20 Min/1"	Drop:	S.D. USADLO	Area riova	180 2000		
No.	of Bedrooms	Septic Tank Cape Leac ovided ByL.F	acity 2000 hing Ge	Gals. Maso	nryreta			
Abso	rption Area Pr	ovided ByL.F	• X24.*	36"width	trench. Ot	ber		
antenenenenenen Nier erstene	F. Stanle	Pattersm		Signature_	Kom	by Lat	tverm	P.E.
		co fieldform, O		SEAL	STATE OF MILE	FRI FOR		
Addr	vess <u>viv or</u>	amford C	1 11 21					
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We st	cestor county	Health Departme	nţ		PROFFECT	ONAL SALAS		
	-	redSc.Ft./Gal		Checked by		Datte		
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S.D	. 27.6 (Rev.	5-24-66)		suite	returned	al-eth	flime	T'

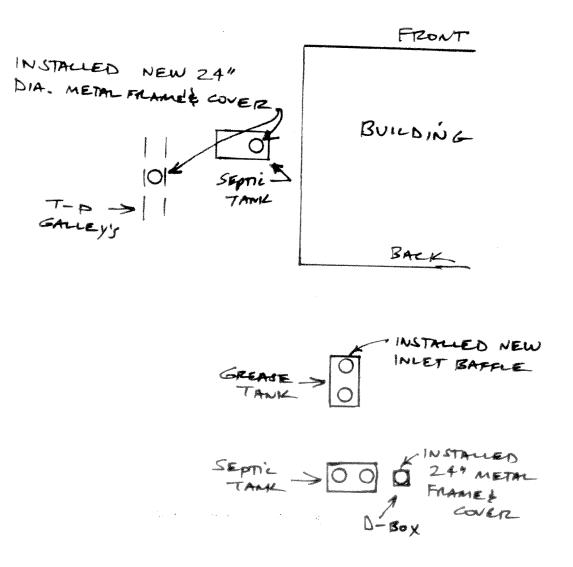
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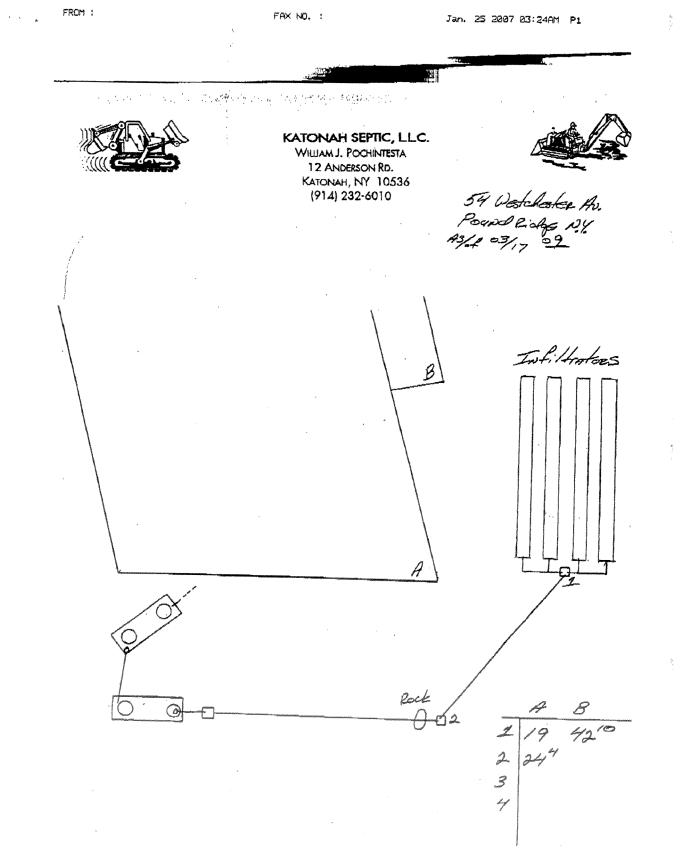
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		Westchest	er. com		
	C	NSITE WASTEWATER TREAT REPAIR AND REMEDIAT	MENT SYSTEM (NON DATA FORM	OWTS) I	
М	funicipality: low	D LIDGE			
P	roperty Mailing Address		toster Are		
-	Village: 10	ind high	State: N.Y.	Zip: 10576	
		MANAgement, LLC	••••••••••••••••••••••••••••••••••••••		
		No. & Street) (if different): 1.0.			
				Zip: 10576	
r		e Family [] Multi-Family [] Industrial			
	U Other	- Describe:	CAS	6 #	
C	WTS Remediation		WCDH Fi	10#: BEQ-2665-1	FMK-
W	astes or offensive mater	installation, replacement, or expansion of or ending failure, resulting in, or that may tem ial on to the surface of the ground, into a st fude repairs, as defined above, to correct ar	orm sewer or into a wat	water or domastic waster as the to	VAST-SS
	/	C OR	*		
C	WTS Repair 🖌	Complete the following information	· z · ·		
R tr	tepair shall mean the representation of the second	air, maintenance, and replacement in kind ents.	and in situ; of broken, da		r
N	umber of Bedrooms	Number of Bathrooms:	WaterSupp	ly.Type: Public 🗖 Well 🎽	
	Ple	ase note below only components that	have been repaired (r replaced.	
R	epaired Replaced	\$	•	<u>x</u>	
		House Sewer or other Solid Pipe(s)	DR	AW BUILDING AND LOCATION	
		Septic Tank#1 Size(gallons):		WORK PERFORMED ON BACK	
* * · ·		Septic Tank#2: Size (gallons): Junction/Distribution Box(es)	OF	THIS FORM	
5.800	D , , , D	Sewage Pump(s) or other Dosing Equipm			
		Absorption Trench Length ft. : Seepage Pit(s)	X Trench Width	_ft	
		Galley(s)			
		Gravelless Trench(es)			
		75-A Alternative System			2
		Other Advanced Alternative System Other,System Component(s) - Describe:	lowers of	loops by to land	tradior
第1 年 。 公	C Entire	System Replaced	installed a	with seak tight	lids
* 4.* P	Contractor's Name (print	VUNITER (E _ Mi Leur	service a service and a service and a service	WITE AT INAN	or way has
	Contractor's Signature:	22	License No.: /D	iation Completed: <u>//-/9-/4</u> 9	1.000.04
C	pon completion please	remit to:			
		Westchester County Departm 25 Moore Ave., Mt. Kisco, NY	1 [#] Floor 10549		
		Attn: Patricia Torn		nair Fils #-DCD	
()	1/12)		nc.	pair File #: <u>REP</u> (WCDH Staff only)	

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	Attention Vincent Silva	
	Westchester gov.com	
	ONSITE WASTEWATER TREATMENT SYSTEM (OWTS)	
Municipality	REPAIR AND REMEDIATION DATA FORM	
		4
		-Al-
	PMNG MANAGEMENT 66C	
	e: Bardonia State: N.Y. Zip: 19954	
Property Use:		
Property Use.	[] Other - Describe:	
OWTS Remed	wCDH File #:	
an OWTS failure wastes or offensi	shall mean installation, replacement, or expansion of onsite wastewater treatment system components to correct lure, or impending failure, resulting in, or that may result in, the discharge of sewage or domestic wastes or trad- ensive material on to the surface of the ground, into a storm sewer, or into a watercourse or water body. shall not include repairs, as defined above, to correct an OWTS failure.	
	OR	
OWTS Repair	air 🛛 Complete the following information.	
	n Tur	
Repair shall mea treatment system	mean the repair, maintenance, and replacement in kind and in situ; of broken, damaged, or worn onsite wastewaitten components.	ter
Repair shall mea treatment system Number of Bedro	mean the repair, maintenance, and replacement in kind and in situ; of broken, damaged, or worn onsite wastewat tern components.	ter
treatment system	mean the repair, maintenance, and replacement in kind and in situ; of broken, damaged, or worn onsite wastewat tem components.	ter
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reatment system Number of Bedro Repaired Re	mean the repair, maintenance, and replacement in kind and in situ; of broken, damaged, or worn onsite wastewat tem components. edroomsNumber of Bathrooms:Water Supply Type: Public [] Well [] Please note below only components that have been repaired or replaced. Replaced [] House Sewer or other Solid Pipe(s) [] Septic Tank#1 Size(gallons):	ter
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03-27-09;02:54PM;Brosnan

2/ 2

; (845) 624-9253

9455-27 38 WESTCHESTER AVE

Westchester	Westchester County Department of Health Bureau of Environmental Quality
WCDH File No. PR2007-18 Municipalit	X Private Water Supply
CERTIFICATE OF CONSTRUCTION COM	PLIANCE:
Located at: <u>38 Westchester Avenue</u> Owner Last Name: Ferrara Owner Firs Becker	Watershed Basin : L.I. Sound Section: 8 Block: 9455 t Name: Thomas Lot: 27 Doctor
	Sarah Eot. 27 R.S. Lot:
Separate Sewage System to Consist of:	
Septic Tank Size: 1,000 Gallons Trench I Other Requirements:	ength: 34 Lin.Ft. X Trench Width: 24 Inches
	Bedrooms 0 Date Permit Issued: 10/19/07
Erosion Control (EC) Completed Yes	EC Waived
Other Requirements:	
Separate Sewage Contractor (SSC): Fracher	IUX # 159
Water Supply:	
- I ublic water Supply Publi	c Water Source:
Well Driller (WD) Company Name: TORLISH	+ Sons
	WATER METER INSTALLED AS REDUCED
I certify that the system(s) as listed serving the above pro (copies of which are attached), and in accordance with the the Westchester County Department of Health.	emises were constructed as shown on the plans of the completed work ne standards, rules and regularions plans filed, and the permit issued by
Date: 5/9/08 Certified by The add	A A A A A A A A A A A A A A A A A A A
Certified by: Theod	
ecome null and void when a public sanitary sever hen, in the judgement of the Commissioner of Health, su change shall be done under the superior of Health, su	tem(s) shall promptivitate such action as that be necessary to secure such usage. Approval of the approval of the private water supply shall becomes available and the approval of the private water supply shall s available. Such approvals are subject to modification or change the revocation, modification or change is necessary, said modification and Professional Engineer or Registered Architect. With proper sfactorily and are not likely to create an unsanitary condition.
ate: Recommended By:	
ite: 3 2 of Approved By:	Lel B
shua Lipsman, M.D., M.P.H., Commissioner, Westo	hester County Department of Health

Westchester ov.com	Westchester County Department of Health Bureau of Environmental Quality
WELL COMPLETION REPORT:	WCDH File No. PR2007-18
This report is to be completed by well driller an water sample indicating water is of satisfactory	WCDH File No. PR2007-18 d submitted to Health Department, together with laboratory report of analysis of bacterial quality, before certificate of construction compliance is issued.
	action to be in accordance with Bulletin SD-62, ATIONS RELATING TO INDIVIDUAL WATER SUPPLIES"
Located at: <u>38 Westchester Av</u>	enue Section: 8 Block: 9455
	pund Ridge Lot: 27
Bockan	r First Name: Thomas
ot. #. 50 St. Name: West	Municipality: In Pound Ri State: NY Zip Code: 10576
Well Driller (WD) Company Name: 136	LISH + SONS
Well Pit and Pump Equipment Details: P	tless Adapter. J Other - Describe:
Pump Make: Grundfos Pump	Type: Cuburge FI D
Storage Tank Type: Welly Trol	Storage Tank Capacity: Wt /02
Well Details:	
Casing Length : 35 Ft.	Yield Test Type : Airo Measured from Land Surface:
	eld Test Duration : 6 Hrs. Water Level, Static : 7 Ft.
Casing Material: ST-ReL	Well Yield : 5 G.P.M. Water Level, Pumped : 400 Ft.
Screen Make : Scre	en Diameter : In.
Screen Length : Ft. Scre	en Slot Size : TOTAL WELL DEPTH 525 Ft
	WELL LOG :
Coarse), color of	of formation penetrated, such as: peat, silt, sand, gravel, clay, hardpan, e, granite, etc. Include size of gravel (diameter) and sand (fine, medium, material, structure (loose, packed, cemented, soft, hard). For example: packed, yellow sand; 27 ft. to 134 ft. gray granite.
6 Ft. to 2 Ft. Well Geology,	the contraction of the gray granite.
Z Ft. to 7 Ft. Well Geology, 2	
18 Ft. to 525 Ft. Well Geology, 3	ZITA ICUN STAINT
Ft. to Ft. Well Geology, 4	
Ft. to Ft. Well Geology, 5	h Strata :
I Certify that the individual water supply indicates SD.62 of the Westchester County Department	ed above was installed as per the rules and regulations of Bulletin of Health.
Data Malling and the state	
Sworn to before me this day	ate of Signature : 6/16/08 DEC # 10318
A Section and a section of the secti	Driller Signature : Cecane torhul

WESTCHESTER COUNTY DEPARTMENT OF HEALTH Bureau of Environmental Quality 118 North Bedford Road Mount Kisco, NY 10549

DESIGN DATA SHEET – SEPARATE SEWAGE SYSTEM FILE NO. Owner Thomas Ferrara/Sarah Becker Address 38 Westchester Avenue, Scotts Corners Located at (Street) Block 9455 Lot 8

(Indicate nearest cross street) Municipality_Town of Poundridge

Watershed

Sec.

SOIL PERCOLATION TEST DATA REQUIRED TO BE SUBMITTED WITH PPLICATION

Presoak Date: 3/27/07

Run Date: 3/28/07

27

Hole #		CLOC	K TIME		PERCOLATION			
Hole Number	Run No.	Start	Stop	Elapse Time Min.	Depth From Grou Start Inches	to Water und Surface Stop	Water Level Drop In	Soil Rate Min/in
1	1	11:59	12:16	17	20	Inches 23	Inches	Drop
	2	12:18	12:37	19	<u> </u>		3	17/3=5.6
	3	10:00	; ;		20	23	3	19/3=6.3
	4	12:39	1 :03	24	20	23	3	24/3=8.0
	1	1:05	1::30	25	20	23	3	1
	5	1:32	1::57	25	20	23	3	25/3=8.3 25/3=8.3
2	1	12:02	12:21	19	20	1		
	2	12:24	12:48	24	20	23	3	19/3=6.3
8 8 8	3	12:50	8	* * *		23	3	24/3=8.0
	4	1	1:16	26	20	23	3	26/3=8.6
9 	5	1:20	1:46	26	20	23	3	26/3=8.6
1 1 2 1		1 1 2	1	8 8 8	5 5 5 5	8 8 8 9		
3		12:04	12:25	21	20	23	~	
3 4 8	2	12:29	12:51	24	20		3	21/3=7.0
9 8 8 0	3	12:54				23	3	24/3=8.0
1 2 2 2	4	1:22	1:20	26	20	23	3	26/3=8.67
1 2	5	1 1 1	1:48	26	20	23	3	26/3=8.67
				। र र	4 8 8	ę ę		

Notes:

Perc test done by: Theodore L. Strauss

1. Tests to be repeated at same depth until approximately equal soil rates are obtained at each percolation test hole. All data to be submitted for review.

2. Depth measurements to be made from top of hole. DO NOT REPORT INCREMENTS OF LESS

DEPTH G.L.	HOLE NO. 1 Topsoil	HOLE NO. <u>2</u> Topsoil	HOLE NO. <u>3</u> Topsoil	HOLE NO	
6"	Topsoil b	Topsoil	Topsoil		
12"					
18"	<u>Sandy Loam</u>	<u>Sandy Loam</u>	<u>Sandy Loam</u>		
24"					
30"	- Eine exaded a				
36"	rine graded so	an <u>d with small to</u> "	medium stones		
42"		 I1		2 <u>1</u> 0F	c_{2}
48"	11	11	11		
54"	·				12
60"	11		II.		
66"	H	11 	11		
72"	Water	Water	Water	· •	
78"		water	water 		
84"					
INDICATI INDICATI	DUNDWATER ENCO E LEVEL AT WHIC ED LEVEL FOR WH T MADE BY 7.	H GROUND WATER IS IICH WATER LEVEL R	ENCOUNTERED 7 ISES AFTER BEING EN DATE OF DEEP TES		
		DESIGN			
Soil Rate U	Jsed <u>8-10</u> Min	/1" Drop: S.D.	Usable Area Provided	4,500 s.f.	
		ic Tank Capacity 1,00		Metal	
Absorption	Area Prov. by 150	L.F. x 24" width t	rench. Other	- TEREDARCA	
NameG	race Lynch		Signature	G GANGE LYNCH	
Address_6	3 Moore Avenue		Seal		<u> </u> *
M	t.' Kisco, NY,	10549	00	17 - C31180 - C	\$]]
Westchester	r County Health Dep	artment		OF NEW	1
Soil Rate A	pproved	Sq. Ft./Gal	Checked by	· · · · · · · · · · · · · · · · · · ·	

TEST PIT DATA REQUIRED TO BE SUBMITTED WITH APPLICATION DESCRIPTION OF SOILS ENCOUNTERED IN TEST HOLES

S.D. 27.6 4/98

THEODORE LAURENCE STRAUSS A S S 0 С I A Т Ε S architects planning consultants ٠ 63 maore avenue · mount kisco · new vork · 10549 · 914-241-3354

• •

27 August 2008

WESTCHESTER COUNTY DEPARTMENT OF HEALTH 118 North Bedford Road, Mt. Kisco, NY, 10549

RE: Permit No. PR 2007-18 - 38 Westchester Avenue, Poundridge, NY.

Dear Fred,

Pursuant to your request, and the condition of the above referenced permit for the installation of the well and septic system, specifically the installation of a water mandewater use meter, I have inspected the building and found same to be properly installed on the lower level.

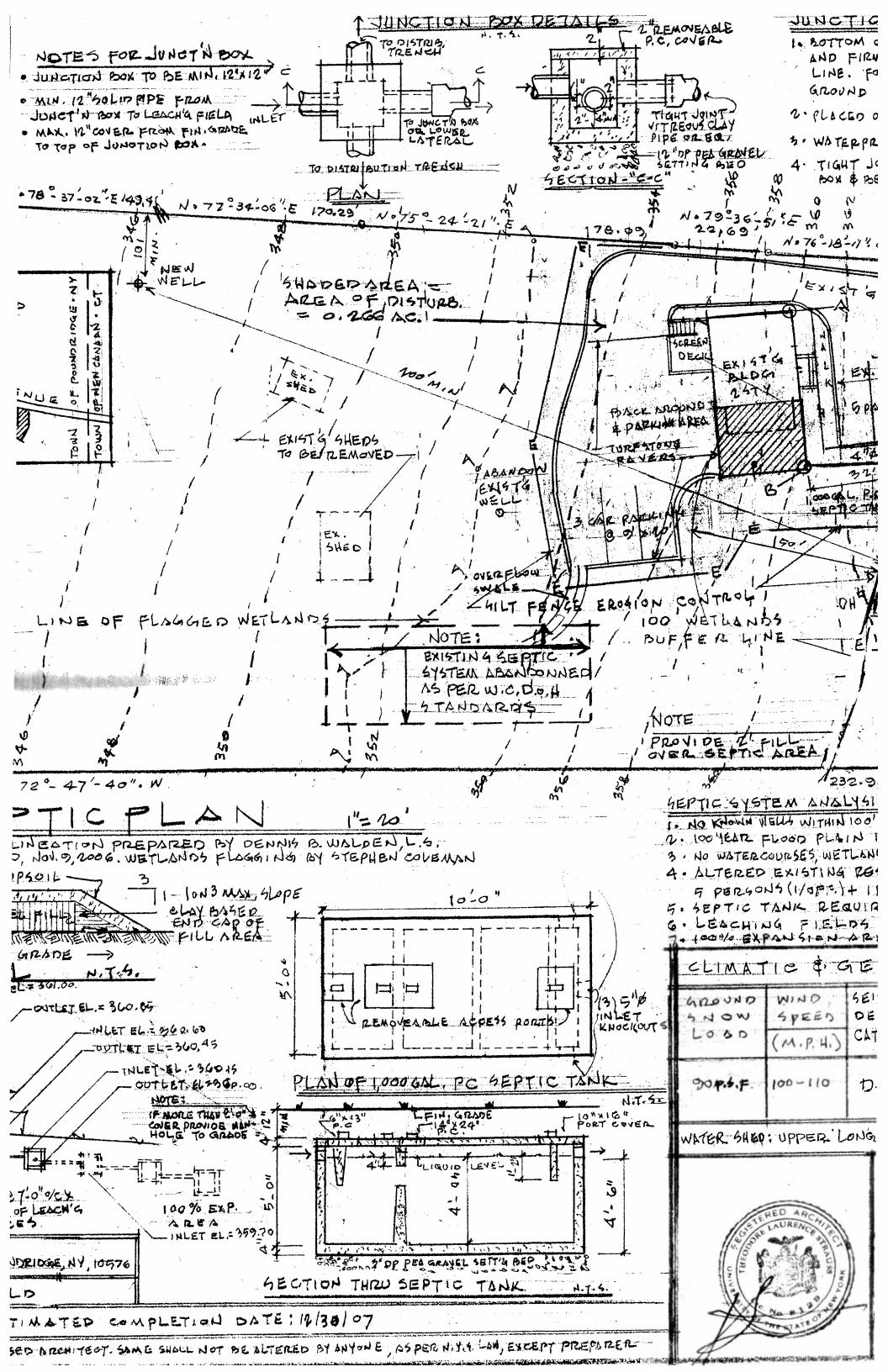
Trusting that the above provides the certification and verification of

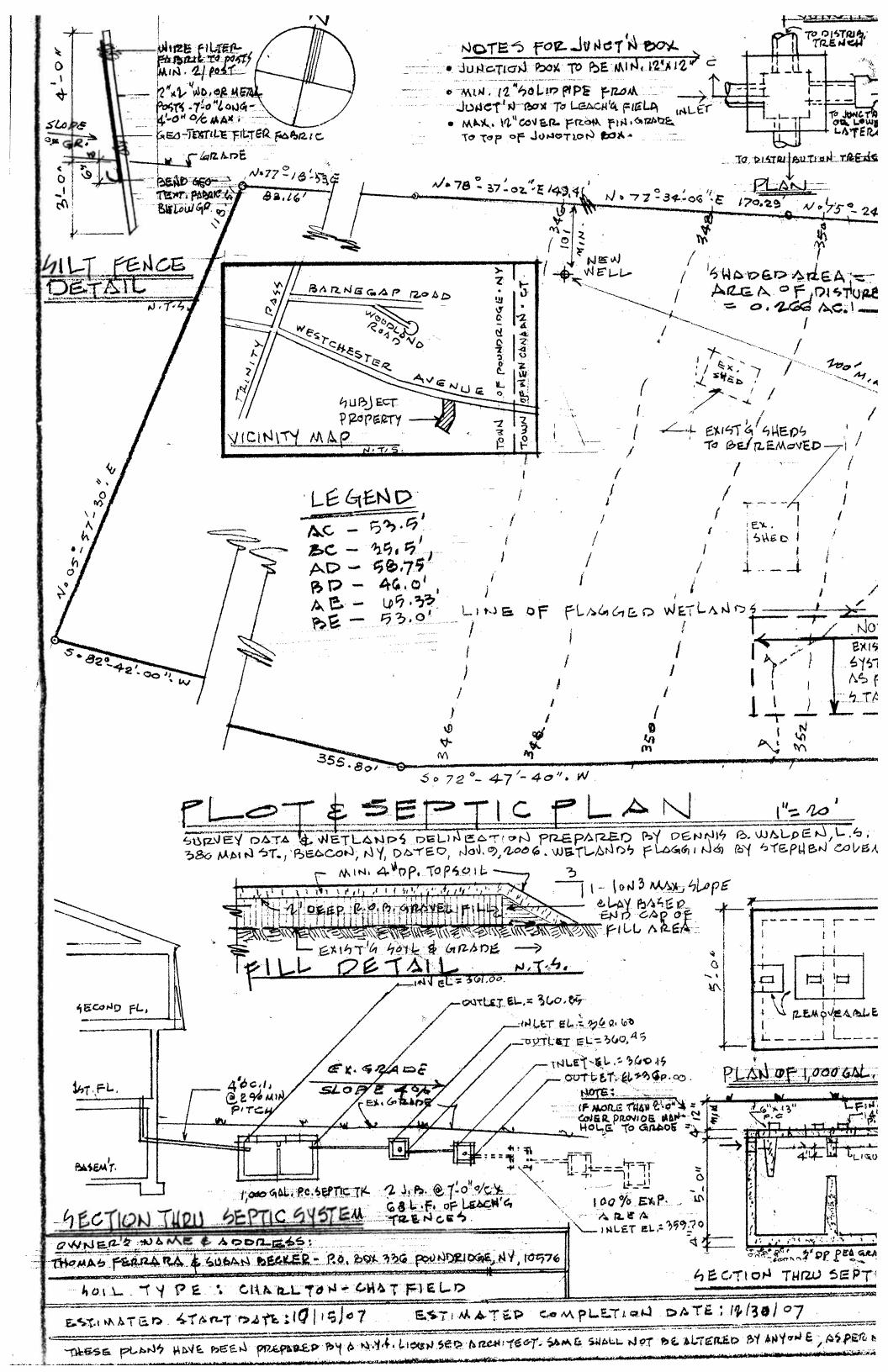
this item requested.

Consideration of the State Sta	Westchester gov.com WESTCHESTER COUNTY DEPARTMEN Bureau of Environmental Quali PERMIT NUMBE	ty
λ_{i} di $\partial_{i} \lambda_{i}^{i}$,	Name: Ferrent Bechet Municipali Description: 150 GPO Max - office Use of + WELL (W/ Meter) # of Sheets: ONC (1)	ty: Ponos Kidk July SSTS
ינינטאנינטאראראנאנטאראינאנטערא ראראנעראראראנעראנערארארארארארארארארארארא		
AND DESCRIPTION OF A DE	Reviewed by:	Date
n verse og ganget og sy statistisk og som en so	Recommended by	Date 5/23/33 Dec.

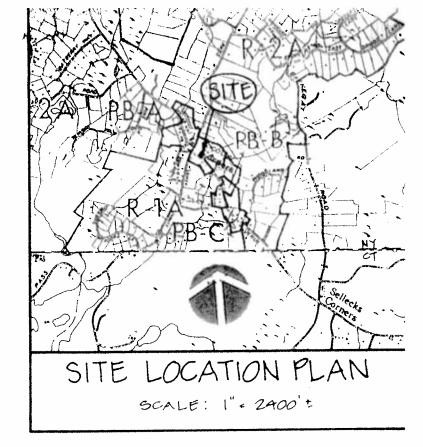
24 JUNCTION BOX NOTES DETALLA P.C. COVER FIN ; GR IN BOTTOM OF JUNCTION BOX MUST BE LEVEL TOPADILE OVERPILE AND FIRMLY SUPPORTED TO BELOW FROST LINE. FOOTING TO EXTEND TO 3'-GUBELOW GARTH BACKFILL | Z D 30 GROUND LEVEL. GEO- TEXT MATL TIGHT JOINT 2. PLACED ON SINGLE BRANCH DISTRIBUTORS, -1 14 mil DISTRIB Ð PIPE OR BOT 3. WATERPROOFED MASONRY CONSTRUCTION. NOCHEP SETTING BED CE ST OR JA-U DRAVEL 3/4-U IV74-NO FINES 4. TIGHT JOINT PIPE FROM SEPTIC TANK TO . SECTION -"C-C" BOX & BETWEEN ALL BOXES. N. 79-36-51 5 m C GROUND WS TER, 178.09 m ABSORPT'N TR=7LOW DE DROCK OR SOIL 4PACING DP N= 76°-18-11 " E \$3.63 m F 5 HORIZONTAL SECTION N.T.S. EXIST 5 DRIV TOP SOL OVERFILL TITLE SCREEN EARTH BACKFILL DECK EXIST G NEW EX. PARILG M BLOGI T GEO-TEXT. MATL PARK 6 1.574 P HPACIGS !! D PER NG PACE AROPHO SPARIX E 0151 + PARKING AREA r c o PH 23 4. T. PZ WOSHED CRIST. 40 GRAS Ù URF STONG 4 the e.l. IENU 50 B ODEGAL. P.C. - GROWNO WOTER, BEDRON G 3. CAR PARLICI 17J 0 7 44 LONGITUDINAL SECTION LEACHING TRENCH DETAILS AGE EROSION CONTROL 1-DO NOT INSTALL TRENCHES IN WET SOL 10H 2. PAKE SIDES & BOTTOM OF TRENCH BEFOR WETLANDS 100 DH #3 TOH#2 ş. PLOGING GRAVEL. BUFFER LINE E 3. END S OF ALL DISTRIB PIPESMOST BE P ENDS CAPPED 4 - TEENCH STONE TO BE 3/4 -11/2 WAS DNNE GRAVEL FREE OF FINES OR SH , D.o. H Φ 3 NOTE 5. TRENCH COURSE TO BE COVER PROVIDE 2 FILL WITH GEO-TENTILE MATERIA OR APPROVED EQUAL. 1232.96' + 5 . 1 -26'- 40".W 50.00 Ú. 5 HEPTIC SYSTEM ANALYSIS & NOTES I. NO KHOWN WELLS WITHIN 100'OF PROPOSED 5.5. P.A. OR WITHIN 200'IN LINE WITH DRAINAGE TO PROPOSED I A. 100 YEAR FLOOD PLAIN TOOL + FROM 5.5. D.A. 3 . NO WATERCOURSES, WETLAND' OR STREAMS WITHIN 100' OF 5.4, D.A. 4. ALTERED EXISTING RESIDENCE INTO OFFICE BUILDING FOR 5 OFFICES + RECEP. + CONF 5 PERSONS (1/OFF.) + I RECEPT + 2 IN CONF. RM = B PERSONS @ 159. P.D. = 120 G.P.D 5. SEPTIC TANK REQUIRED = 1,000 GAL, PRECAST CONC. 6. LEACHING FIELDS = 8-10 SOIL RATE - APP. PATE = 0.9 9/5F-(120 GPDA 0.90/5F) /2 5F/FT = 67 L. (150 GPO/0.90/5F)/2 = 833 L.F. 1+ 100% EXPANSION AREA PROVIDED CLIMATIC & GEOGRAPHIC DEGIGN' CRITERIA

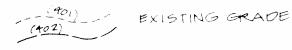
1-13)5"#	GROUND	MIND	SEISMIC	SUBJEC			FROM	DESIGN	ICE SHIEP	and the second sec
PORTS KHOCKOUTS	SNOW	SPEED	DESIGN	WEATER'S	FROST	TERMITE	DECAY	TEMP	UNDERLAYAT REQ'REM'T	H 522A
	LOSD	(M.P.H.)	CATEGORY		DEPTH				IL G V KEM 1	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
PTIC TANK	90 p. 6.F.	100-110	D-1	SEVERE	42"	MODERATE	SLIGHT	7°F	YE3	AMILIIM
FORT COVER	WATER SHEP	UPPER'L	ONG ISLA	and the second	and a second sec				レロビル - 94月	Contract of the local division of the local
4'- 6"	A STATE	RED ARCA		TH	OMAS F	ERRAR	A & GAN	PERCES	KER IE I LIDGE-M	23 -0/19/ +0/10 20 by 10/17
110			drawing		15- P. \$ 555	DILT DIC F	PLAN		scale A54 Jo 25	SHOWAN B/ 11,
		New J20 H		DORE L	AURENC	E STR	ng C(ASSOC nsult	ants	ing no

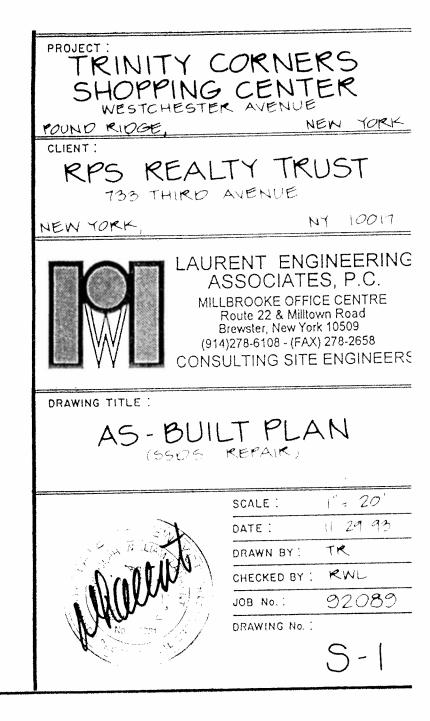




9456-1.9 55 WESTCHESTER AVE





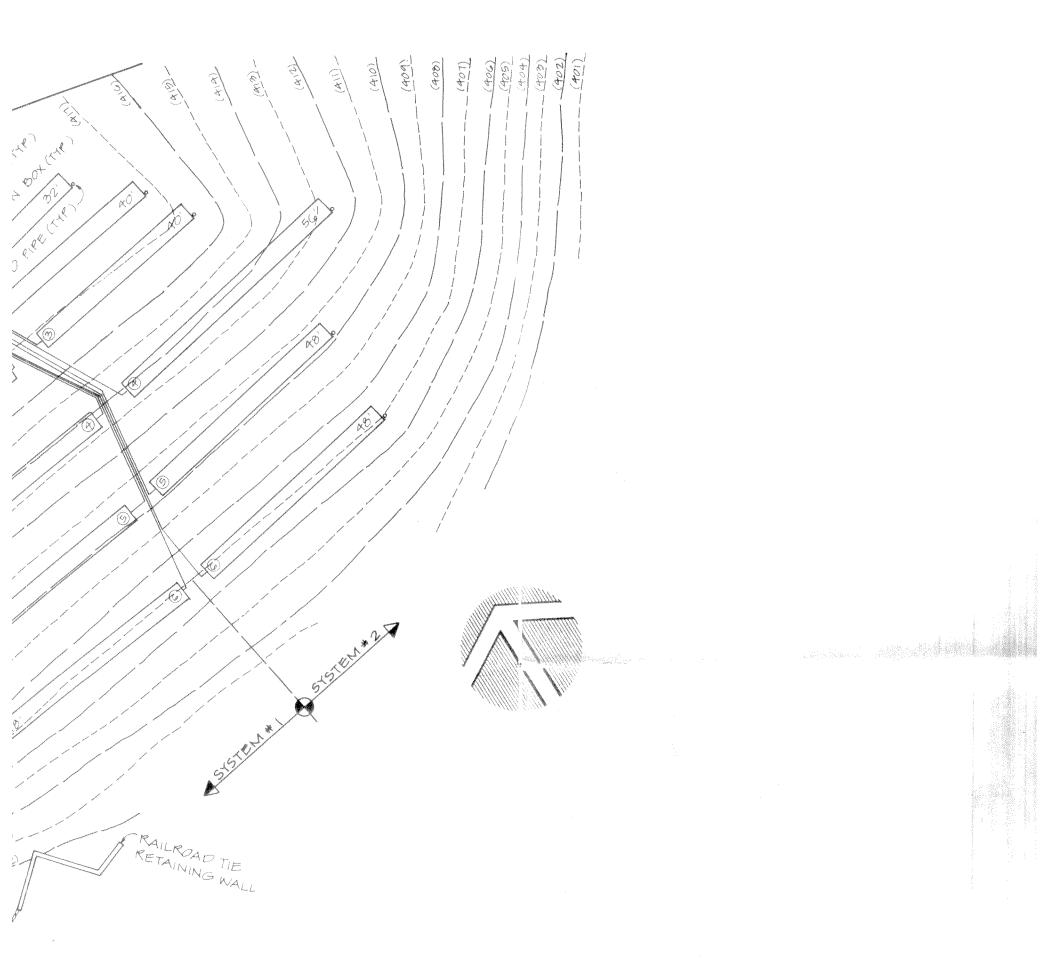


<u>TEM #2</u>

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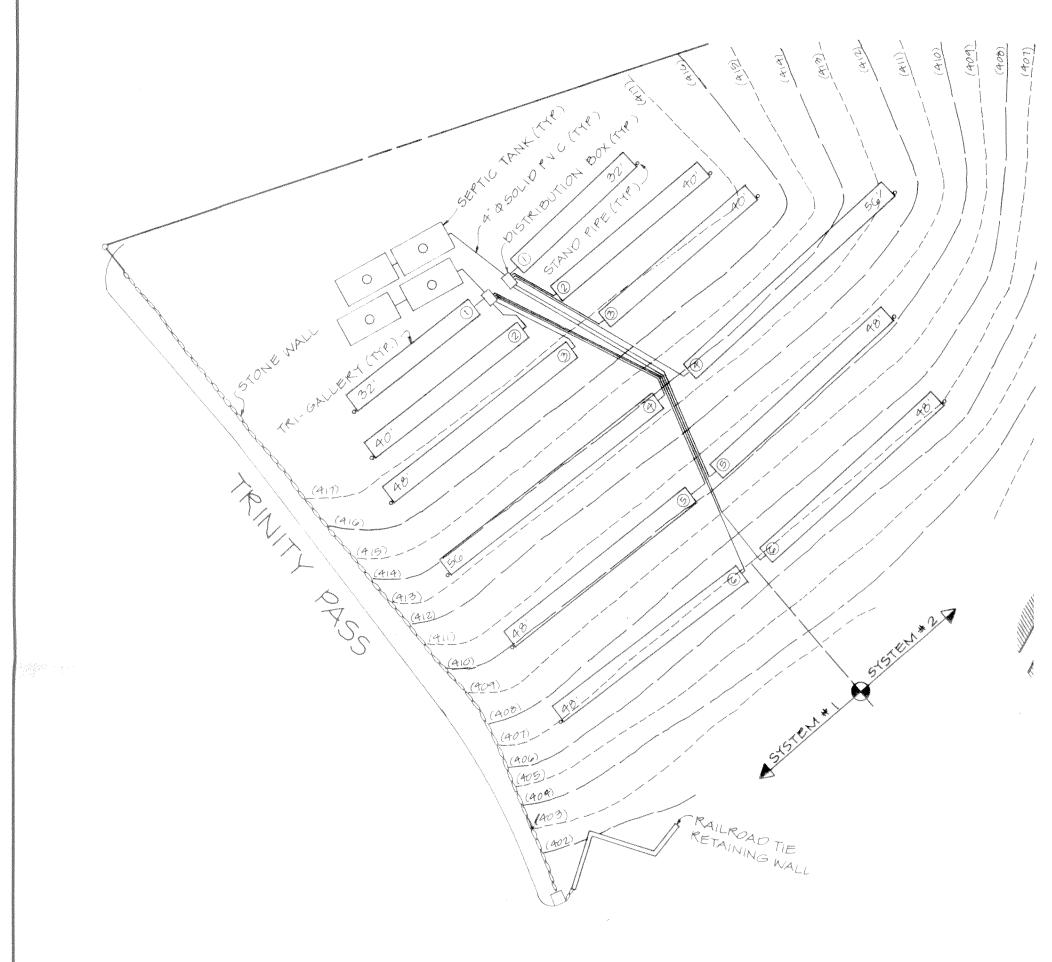
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416.33	415.66	_
416.13	415.46	
415.57	414.90	_
411.89	411.23	
408.93	408.26	
405.98	405.31	_



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IN	ουτ		ª STAND PIFE END	@ DIST. <u>Fipe End</u>	IN	OUT
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416.24	416.15	2nd SEPTIC TANK	astron	AND	416.49	416.39
416.29	415.73	DISTRIBUTION BOX	Alam	Loonerte	416.00	415.73
	1. L. L. A. L.	TRI-GALLERY #1	416.27	416.33	415.66	th
415.65		TRI-GALLERY #2	116.11	416.13	415.46	upentit
415.40		TRI-GALLERY #3	415.55	415.57	414.90	umar
414.96		TRI-GALLERY #4	411.86	411.89	411.23	North Contraction
411.37	,	TRI-GALLERY #5	408.96	408.93	408.26	unati
408.29	angia		405.92	405.98	405.31	vianté
405.33		TRI-GALLERY #6	- 7 KJ val 8 / dos			

SYSTEM #2



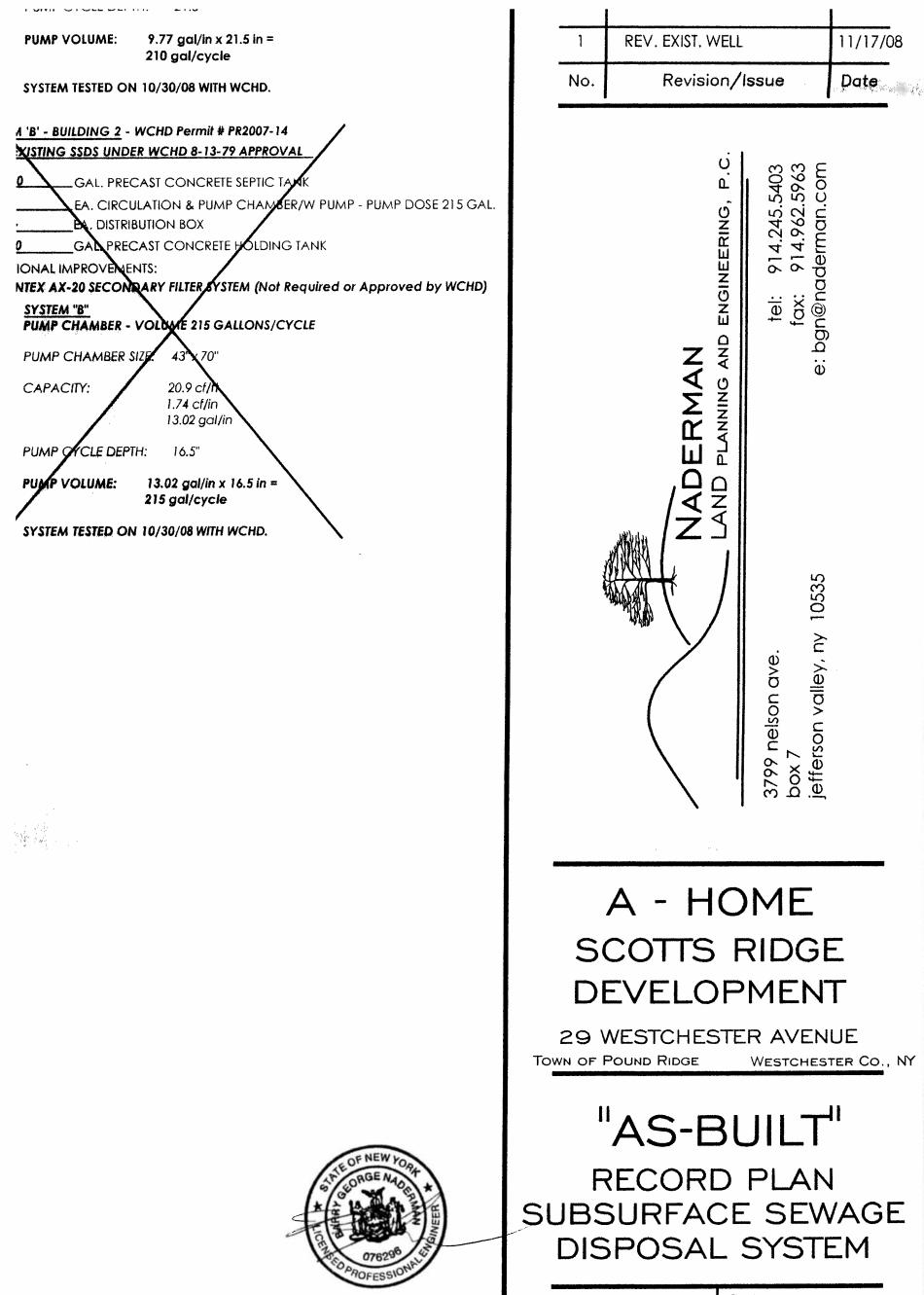
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	TC)F [.]	IN	/ERT	
	@ STAND PIFE END	@ DIST. <u>PIFE END</u>	IN	OUT	
1st SEFTIC TANK	and and a second s		416.45	416.27	1st SEPTIC TANK
2nd SEPTIC TANK		an une	416.24	416.15	2nd SEPTIC TANK
DISTRIBUTION BOX	-44 mg	JANN	416.00	415.73	DISTRIBUTION BOX
TRI-GALLERY #1	416.26	416.32	415.65	i name	TRI-GALLERY #1
TRI-GALLERY #2	416.15	416.07	415.40	*****	TRI-GALLERY #2
TRI-GALLERY #3	415.59	415.63	414.96	and a second	TRI-GALLERY #3
TRI-GALLERY #4	411.90	412.04	411.37	ruwe	TRI-GALLERY #4
TRI-GALLERY #5	408.94	408.96	408.29	water	TRI-GALLERY #5
TRI-GALLERY #6	405.91	406.00	405.33		TRI-GALLERY #6

9456-5 29 WESTCHESTER AVE

Westchester gov.com		nty Department of Health vironmental Quality	
WCDH File : PR 2007-13 New System "A"-Serving Bld Former Permit # PR2006-01 Separate Sewage System	Municipality: POUND RIDGE g. 1 Private Water Supply	🗹 Residential 🛛 Commercial	
CERTIFICATE OF CONSTRU	CTION COMPLIANCE:		
	١	Watershed Basin : STAMFORD	
Property Address: 29 WESTCHES	TER AVENUE Se	ection: 9 Block: 9456	
Owner Last Name: AHOME	First Name:	Lot: 5A R.S. Lot: -	
Owner's Address: 185 KISCO AVI	ENUE, MT. KISCO, NY 10549		
Separate Sewage System to Con	sist of:		
Septic Tank Size: 1500 Gallons Other Requirements: 1250 gal hold Building Type: Senior Housing	max FLOW 600gpp ing tank, recirculation & pump chamb	. Ft. X Trench Width: 72 Inches ber w/ 1/2 hp Pump - pump dose 210 gals/c 18" - 24" ROB Fill Within Primary Are Date Permit Issued: 8-23-2007	
Erosion Control (EC) Completed	Yes		d
Other Requirements: Advantex A	X-20 filter have not been required or	approved by the WCHD.	
Separate Sewage Contractor (SSC): Giovanni Battista Apollonio	WCDH Septic License #	£ 392
Water Suppiy: ✓ Private W	ater Supply	ce: Existing Well	nandom balandar salasan sakaran ta d
Well Driller (WD) Company Name:		NYDEC Reg. #	
(copies of which are attached), and in the Westchester County Department Date: <u>11</u> <u>7</u> <u>08</u> Certified by Any person occupying premises serve correction of any unsanitary conditions as soon as a public sanitary sewer be public water supply becomes available of Health, such revocation, modification	accordance with the standards, rules and of Health. d by the above system(s) shall promptly to resulting from such usage. Approval of comes available and the approval of the p s. Such approvals are subject to modification or change is necessary, said modification pistered Architect. With proper maintenant	ed as shown on the plans of the completed work d regulations, plans filed, and the permit issued by P.E. License #: 076296 take such action as may be necessary to secure t the separate severage system shall become null private water supply shall become null and void we ation or change when, in the judgment of the Com- ion or change shall be done under the supervision ince the systems can be expected to function satist	he and void hen a missione of a
Date: Re	commended By:		
Date: 11/24/08 A	oproved By: Full R		

Joshua Lipsman, M.D., J.D., M.P.H, Commissioner, Westchester County Department of Health

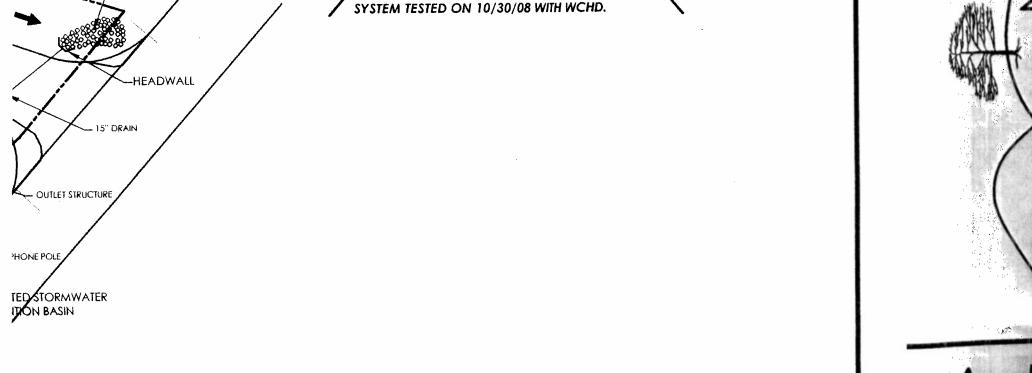


SUBSURFACE SEWAGE DISPOSAL SYSTEM

A" WCHD PERMIT # PR2007-13 HOHD PERMIT # PR2007-14

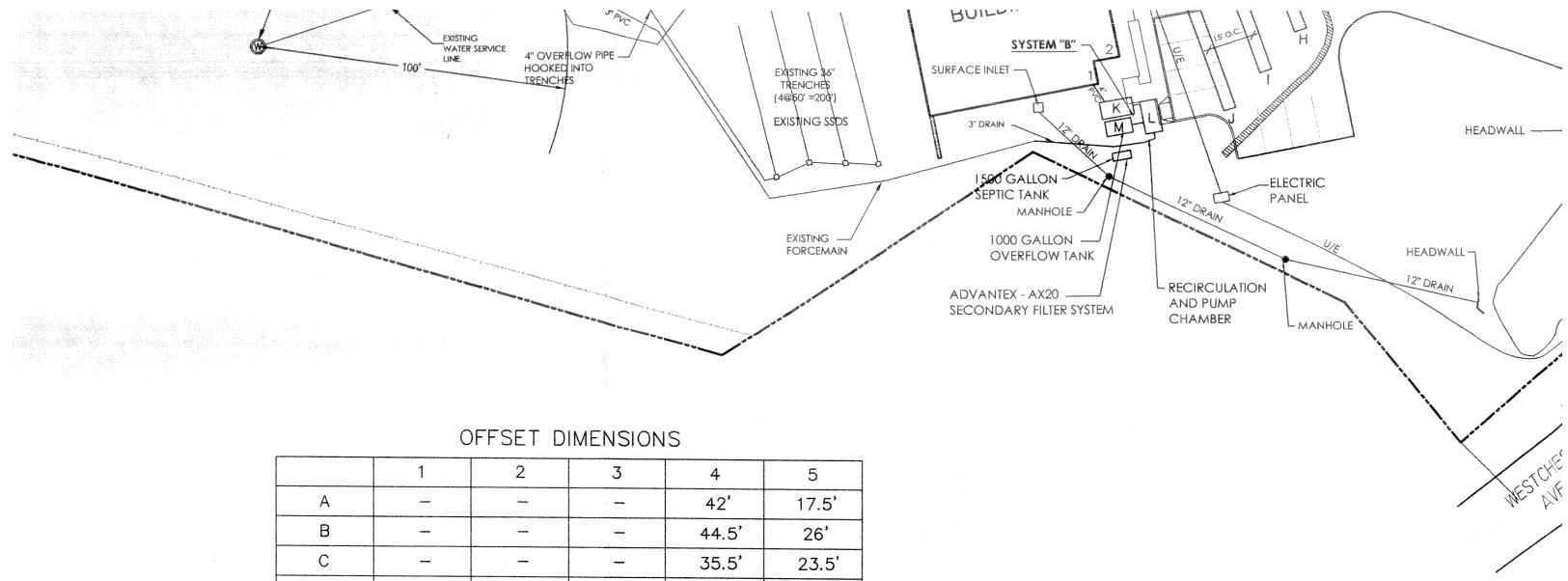
Project	5349	Sheet
Date	-07-08	RP-IA
Scale	I"= 30'	

	NOTES 1.) APPLICANT/ OWNER: <u>A - HOME</u>	
location based upon a survey	ADDRESS: 185 KISCO AVE., SUITE 4, MOUNT KISCO, NY 10549 PROPERTY LOCATION: 29 WESTCHESTER AVE., POUND RIDGE, NY 10576	UNAUTHORIZED ALTER
e upon field inspection and	TAX MAP DESIGNATION: SHEET: SEC. 9 BLK. 9456 LOT 5A	THIS DRAWING IS A VI OF THE NEW YORK ST/
	4.) THE DESIGN OF THE PROPOSED SUBSURFACE SEWAGE DISPOSAL AREA 'A' IS BASED ON A SOIL PERCOLATION RATE OF <u>15-20</u> MIN./INCH. AND A	OF THE NEW TORK 317
' of the new SSTS nor to remain the new SSTS.	PROP. 6 SENIOR RESIDENT SUITES/ BUILDING.	
' of the proposed will nor within 200' the general line of drainage from	MAX. 8 OCCUPANTS X 75 GPD/ OCCUPANT = 600 GPD DESIGN FLOW/ BLDG. 5.) THE SUBSURFACE SEWAGE DISPOSAL SYSTEM SHALL CONSIST OF THE FOLLOWING IMPROVEMENTS:	XAROO!
	SYSTEM 'A' - BUILDING 1 - WCHD Permit # PR2007-13	Rinnay P
with the Rules and Regulations for	216 L.F. 48" WIDE FLOW DIFFUSOR LEACHING CHAMBER	
rface Sewage Treatment Systems	GAL. PRECAST CONCRETE SEPTIC TANK	Westchester Are
	EA. CIRCULATION & PUMP CHAMBER/W PUMP - PUMP DOSE 210 GAL.	The second se
uction of the OWTS and certifies its plans.	Image:	5
	GAL. PRECAST CONCRETE HOLDING TANK ADDITIONAL IMPROVEMENTS:	
t Basin.	ADDITIONAL INTROVEMENTS. ADVANTEX AX-20 SECONDARY FILTER SYSTEM (Not Required or Approved by WCHD)	inter fr
X0 feet of the new SDS.	18" - 24" ROB FILL WITHIN PRIMARY AREA 'A'	
	<u>SYSTEM "A"</u> PUMP CHAMBER - VOLUME 210 GALLONS/CYCLE	Sten and State Sta
	PUMP CHAMBER SIZE: 37" x 61"	500 m
	CAPACITY: 15.67 cf/ft 1.3 cf/in 9.77 gal/in	
	PUMP CYCLE DEPTH: 21.5"	
	PUMP VOLUME: 9.77 gal/in x 21.5 in = 210 gal/cycle	REV. EXIST.
	SYSTEM TESTED ON 10/30/08 WITH WCHD.	No. Revis
	SYSTEM 'B' - BUILDING 2 - WCHD Permit # PR2007-14 EXISTING SSDS UNDER WCHD 8-13-79 APPROVAL	
	GAL. PRECAST CONCRETE SEPTIC TANK	
	EA. CIRCULATION & PUMP CHAMPER/W PUMP - PUMP DOSE 215 GAL.	
	E, DISTRIBUTION BOX	2 4 4
	GAL PRECAST CONCRETE HOLDING TANK	
	ADDITIONAL IMPROVEMENTS: ADVANTEX AX-20 SECONDARY FILTER SYSTEM (Not Required or Approved by WCHD)	
	SYSTEM "B" PUMP CHAMBER - VOLUME 215 GALLONS/CYCLE	
	PUMP CHAMBER SIZE. 43 70"	
LL	CAPACITY: 20.9 cf/n	
RIPRAP OUTFALL	1.74 cf/in 13.02 gal/in	.
	PUMP CYCLE DEPTH: 16.5"	
	PUMP VOLUME: 13.02 gal/in x 16.5 in = 215 gal/cycle	
		「「「「「「「「「」」」「「「」」「「「」」」「「」」「「」」「「」」」「「」」」「「」」」「「」」」」



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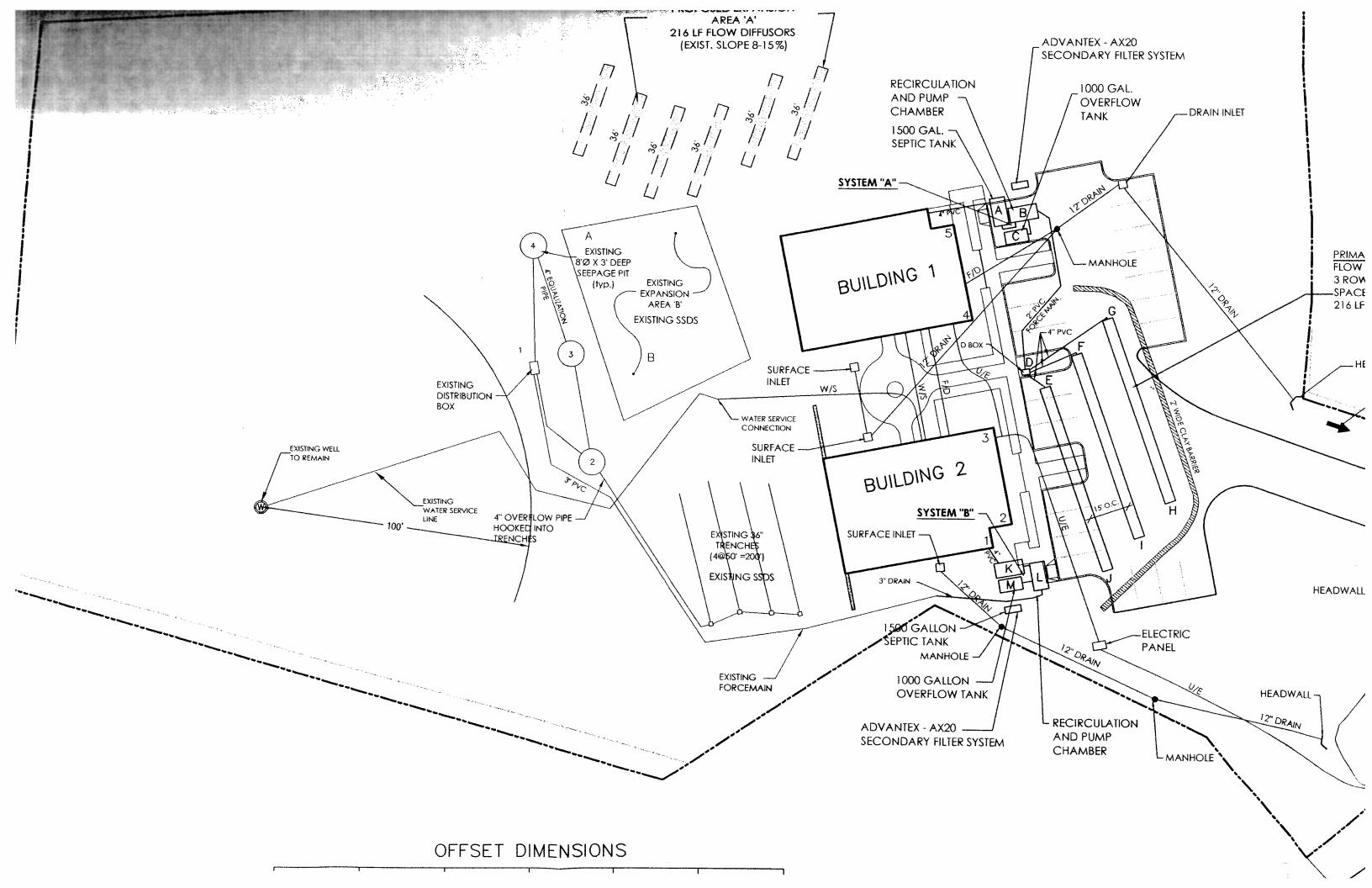
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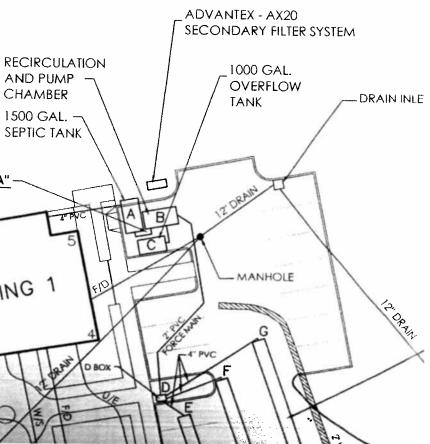
	1	2	3	4	5
A			1 method	42'	17.5'
В			-	44.5'	26'
С		-		35.5'	23.5'
D			24'	27.5	
E		53.5'	24.5'		949 - 1940 - 194
F	-	68'	40.5'		
G	www.	81.5'	56'		
Н		61.5'	74'		
l		49'	68'		
J		40'	67.5 '		
К	10'	17'			
L	20'	22'			44000000000000000000000000000000000000
М	15.5'	23'			







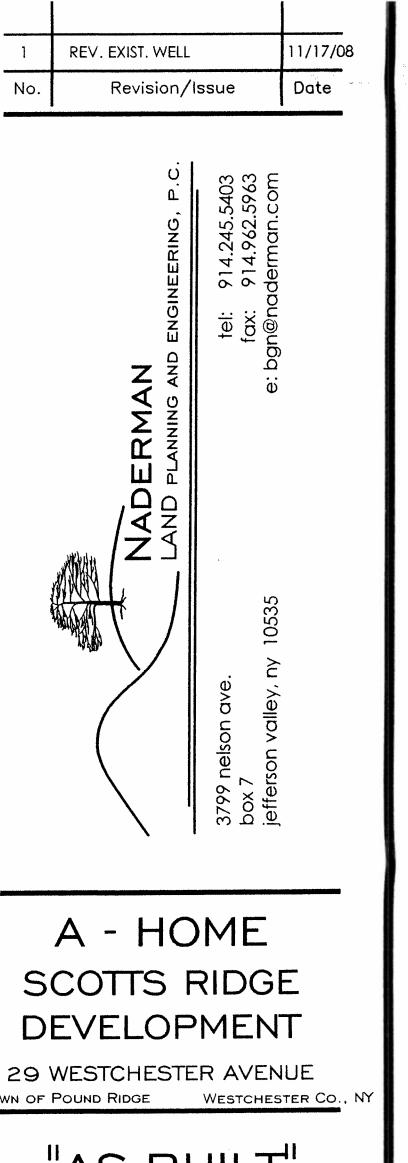
	Westchester			
ſ	WESTCHESTER COUNTY DEPARTMENT OF H Bureau of Environmental Quality	IEALTH		
	PERMIT NUMBER: Prz			
	Name: Jan Arale, A Home Municipality: Pome Description: New SETS T- Same Billy #	<u>shize</u>		
	# of Sheets: ONL (1)			
	Are hereby accepted a provisions 873, Article VIII, Sect	1 873.708.1		
	of the Westchester County construction Compliance issued this date.			
	Reviewed by:	Date		
		Date		
	Accepted by <u>cclfB</u>			
			مرور و برسوید و بر مروری ۵ ۵ مروری و در موری و او موری و او مروری	و و ومست و ی مسیح و ب بزدید و ی
	و هم ها بالاستان بي هم المحالية بي بي حاليه المحالية بي بي المحالية بي بي المحالية بي بي المحالية بي		PROPOSED EXPANSI	7
			216 LF FLOW DIFFUSO (EXIST. SLOPE 8-15)	
				7 17
		×		
				SYSTEM "A"
			A ,	
		82	EXISTING 3 X 3' DEEP EPAGE PIT	
			(typ.) EXISTING EXPANSION AREA 'B'	BUILDI
		1.	EXISTING SSDS	
			SUI INL	
		BOX	1/1-	



Westchester gov.com	Westchester County Department of Health Bureau of Environmental Quality
WCDH File: PR 2007-14 System "B" Existing - Serv See PR73-2 & PR2006-02	Municipality: POUND RIDGE
	Private Water Supply 🗹 Residential 🔲 Commercial
CERTIFICATE OF CONSTRUC	CTION COMPLIANCE: Watershed Basin : STAMFORD
Property Address: 29 WESTCHES	
Owner Last Name: AHOME	First Name: Lot: 5A R.S. Lot:
Owner's Address: 185 KISCO AVE	NUE, MT. KISCO, NY, 10549
Separate Sewage System to Cons	ist of:
Septic Tank Size: 1500 Gallons	max FLIN With: Trench Length: Lin. Ft. X Trench Width: Inches ***Exist. Pits & trenches/Ref/WCHD Permit PR73-2 holding tank, recirculation & pump chamber w/ 1/3 hp pump-pump dose 215 gal/cy
Building Type: Senior Housing	
Building Type. Certion Housing	# of Bedrooms Date Permit Issued: 8-23-2007 **6 Suites w/ Max.8 occupants
Erosion Control (EC) Completed	Yes DEC Waived
Other Requirements: Advantax AX	-20 filters have not been required or approved by the WCHD.
Separate Sewage Contractor (SSC)	: Giovanni Battista Apollonio WCDH Septic License # 392
Water Supply: ☑ Private Wa	ater Supply Public Water Source: Existing Well
Well Driller (WD) Company Name:	NYDEC Reg. #
(copies of which are attached), and in a the Westchester County Department of Date: <u>11/7/08</u> Certified by: Any person occupying premises served correction of any unsanitary conditions	oF NEW or NEW New premises were constructed as shown on the plans of the completed work inccordance with the standards, rules and regulations, plans filed, and the permit issued by f Health. P.E.Liense # 076296 I by the above system(s) shall promptly take such action as a grown of the secure the resulting from such usage. Approval of the secure several section as a grown on all become null and void omes available and the approval of the private
public water supply becomes available. of Health, such revocation, modification	Such approvals are subject to modification or change when in the odgment of the Commission or change is necessary, said modification or change shall be done under the supervision of a stered Architect. With proper maintenance the systems can be expected to function satisfactorily
Date: Rec	ommended By:
Date: $u/z + \sigma^2$ Ap	proved By: Eullex

Joshua Lipsman, M.D., J.D., M.P.H, Commissioner, Westchester County Department of Health

UMP YCLE DEPTH:	21.5"	
UMP VOLUME: 9.	.77 gal/ln x 21.5 in = 10 gal/cycle	1
YSTEM TESTED ON 10,	/30/08 WITH WCHD.	No
	HD Permit	
EA. CIRCULAT EA, DISTRIBUTIO GAL. PRECAST NAL IMPROVEMENTS:	T CONCRETE HOLDING TANK	
YSTEM "B"	LUME 215 GALLONS/CYCLE	
UMP CHAMBER SIZE:	43" x 70"	
APACITY:	20.9 cf/ft 1.74 cf/in 13.02 gal/in	
UMP CYCLE DEPTH:	16.5"	
	3.02 gal/in x 16.5 in = 15 gal/cycle	
YSTEM TESTED ON 10,	/30/08 WITH WCHD.	
		29 Town 0



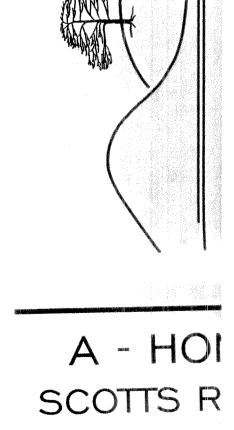


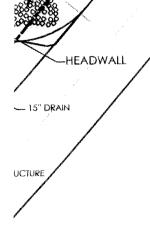
RECORD PLAN SUBSURFACE SEWAGE DISPOSAL SYSTEM

Project	5349	Sheet
Date	11-07-08	RP-1B
Scale	I "= 30'	

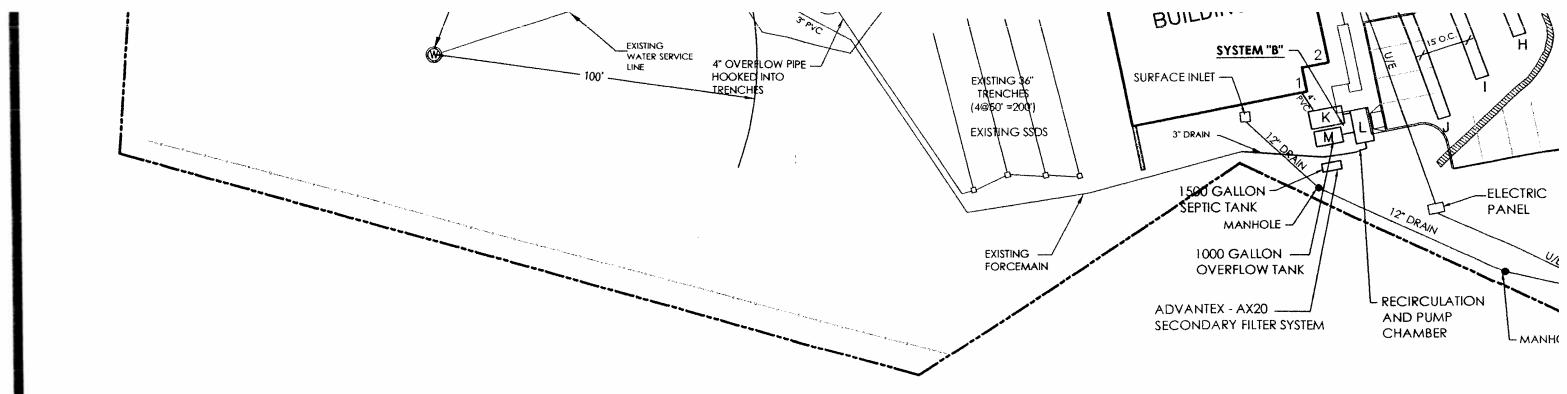
WCHD PERMIT # PR2007-13 WCHD PERMIT # PR2007-14

	NOTES 1.) APPLICANT/ OWNER: A - HOME	
ised upon a survey	ADDRESS: 185 KISCO AVE., SUITE 4, MOUNT KISCO, NY 10549 PROPERTY LOCATION: 29 WESTCHESTER AVE., POUND RIDGE, NY 10576	UNAUTHORIZED ALTERATIONS A
inspection and	TAX MAP DESIGNATION: SHEET: SEC. 9 BLK. 9456 LOT 5A	THIS DRAWING IS A VIOLATION OF THE NEW YORK STATE EDUC
 SSTS nor to remain 	 4.) THE DESIGN OF THE PROPOSED SUBSURFACE SEWAGE DISPOSAL AREA 'A' IS BASED ON A SOIL PERCOLATION RATE OF <u>15-20</u> MIN./INCH. AND A PROP.<u>6</u> SENIOR RESIDENT SUITES/ BUILDING. MAX. <u>8</u> OCCUPANTS X 75 GPD/ OCCUPANT = <u>600</u> GPD DESIGN FLOW/ BLDG. 	
oosed will nor within 200' Hine of drainage from	5.) THE SUBSURFACE SEWAGE DISPOSAL SYSTEM SHALL CONSIST OF THE FOLLOWING IMPROVEMENTS:	APHOO!
	SYSTEM 'A' - BUILDING 1 - WCHD Permit # PR2007-13	
es and Regulations for ge Treatment Systems	L.F. 48" WIDE FLOW DIFFUSOR LEACHING CHAMBER	
e OWTS and certifies its	 EA. CIRCULATION & PUMP CHAMBER/W PUMP - PUMP DOSE 210 GAL. I EA. DISTRIBUTION BOX 	The sector size dive
	1000 GAL, PRECAST CONCRETE HOLDING TANK ADDITIONAL IMPROVEMENTS:	
e new SDS.	ADVANTEX AX-20 SECONDARY FILTER SYSTEM (Not Required or Approved by WCHD) 18" - 24" ROB FILL WITHIN RIMARY AREA 'A'	Cover-Freidy Pass
	SYSTEM "A" PUMP CHAMBER - VOLUME 210 GALLONS/CYCLE	Binna state
	PUMP CHAMBER SIZE: 37" * 61"	300 m
	CAPACITY: 15.67 cf/t 1.3 cf/in 9.77 gal/in	
	PUMP CYCLE DEPTH: 21.5"	
	PUMP VOLUME: 9.77 gal/in x 21.5 in = 210 gal/cycle	1 REV. EXIST. WELL
	SYSTEM TESTED ON 10/30/08 WITH WCHD.	No. Revision/Iss
	<u>SYSTEM 'B' - BUILDING 2</u> - WCHD Permit # PR2007-14 EXISTING SSDS UNDER WCHD 8-13-79 APPROVAL	
	GAL. PRECAST CONCRETE SEPTIC TANK EA. CIRCULATION & PUMP CHAMBER/W PUMP - PUMP DOSE 215 GAL.	φ.
	EA, DISTRIBUTION BOX	ENGINEERING
	1000 GAL, PRECAST CONCRETE HOLDING TANK	С Ш
	ADDITIONAL IMPROVEMENTS:	Ш Z
	ADVANTEX AX-20 SECONDARY FILTER SYSTEM (Not Required or Approved by WCHD)	<u>o</u>
	<u>SYSTEM "B"</u> PUMP CHAMBER - VOLUME 215 GALLONS/CYCLE	
	PUMP CHAMBER SIZE: 43" x 70"	ZZ
	CAPACITY: 20.9 cf/ft 1.74 cf/in 13.02 gal/in	
RIPRAP OUTFALL - TO EXISTING CULVERT	PUMP CYCLE DEPTH: 16.5"	
	PUMP VOLUME: 13.02 gal/in x 16.5 in = 215 gal/cycle	A A A
	SYSTEM TESTED ON 10/30/08 WITH WCHD.	Z Z





'ATER

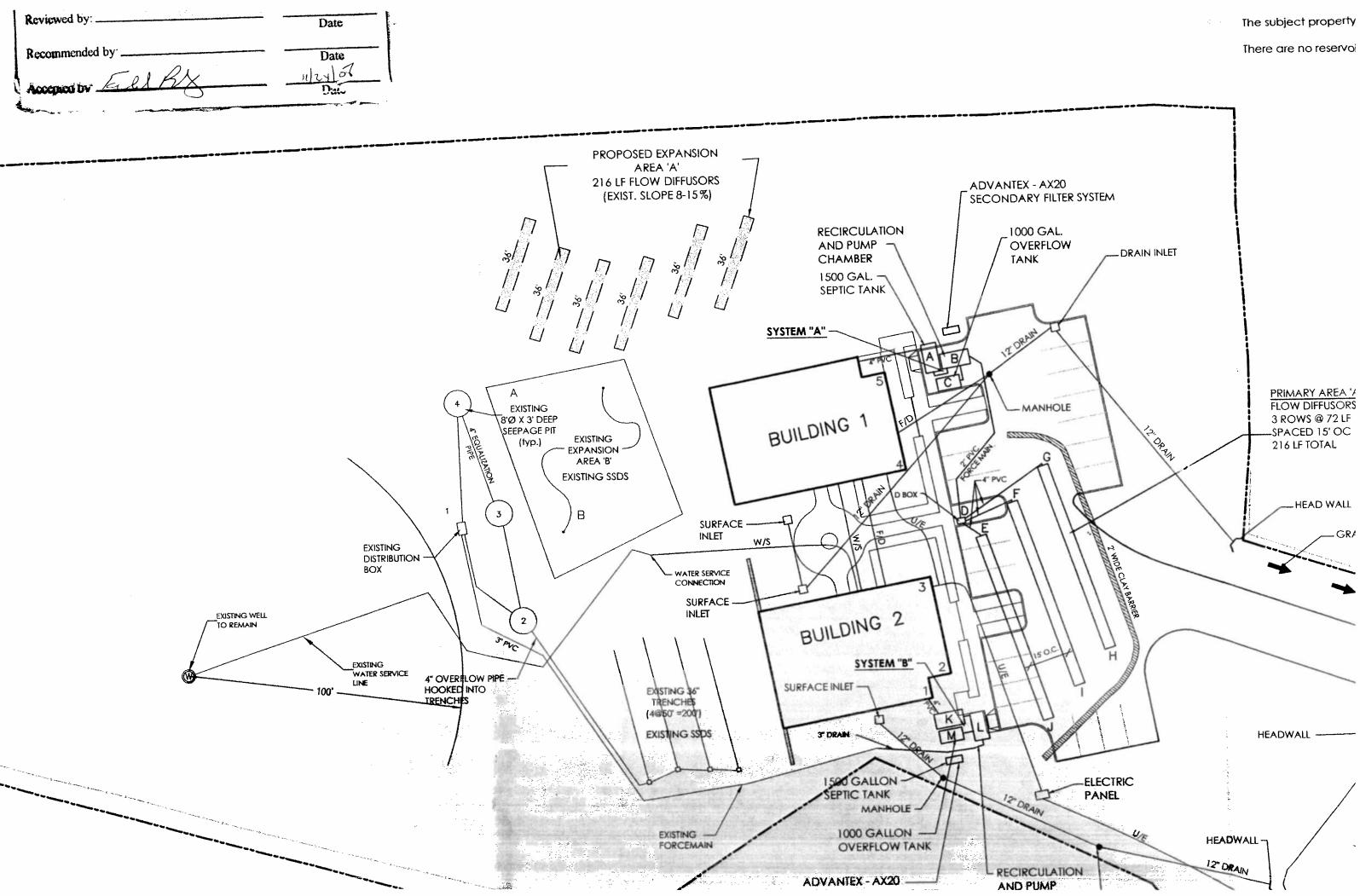


OFFSET DIMENSIONS

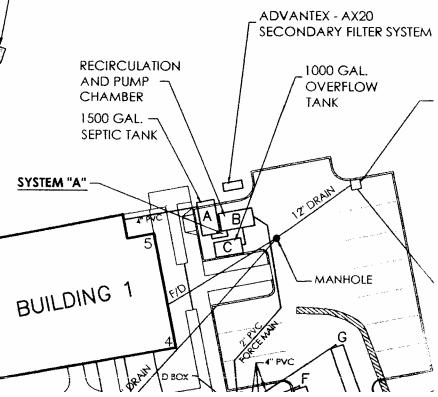
	1	2	3	4	5
A	-			42'	17.5'
В				44.5'	26'
С				35.5'	23.5'
D			24'	27.5	_
E		53.5'	24.5'		—
F		68'	40.5'	—	
G		81.5'	56'		
Н		61.5'	74'	_	
1		49'	68'		—
J		40'	67.5'		_
К	10'	17'			-
L	20'	22'			_
м	15.5'	23'		_	_







	Westchester gov.com WESTCHESTER COUNTY DEPARTMENT OF HEALTH Bureau of Environmental Quality	
	PERMIT NUMBER: PR2007-14 Name: Jin Arich, A Home Municipality: Porno Airby Description: SST3 Trape-unit To Sam Bir Hight (New Static tark + pump chamber) Max Flow 600 gpo # of Sheets: one (1) Are hereby accepted a growisions of Chapter 873, Article VIII, Sector and and the Westchester Champer of Static tark is and the Provisions of the Provisions of the Certificate of Coastruction Compliance issued this date. Reviewed by:	
1		PROPOSE Al 216 LF FLC (EXIST. S



WESTCHESTER COUNTY DEPARTMENT OF HEALTH Bureau of Environmental Quality 110 So. Bedford Road Mt. Kisco, NY 10549

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35" r.E.E.P

Locate	d at	(Street) <u>~~5</u> (Indi	cate near	est cross s	Sec	9 Block 943	56 Lot S
Munici	pali	ty2	0000	1206-	Ę	Water	shed 570M,	EonD
Presoa	k Da	ce: ///	3/00		Run	Date: //	WITH APPLIC	
FOLE #			CLOCK	TIME		7	PA	MARY
Hole Number	 Rur No.	 Start	Stop	Elapse Time Min.	Depth From Gr Start Inches	d Surface Stop	Water Level Drop In Inches	Rate Min/
	1	3:23	3:53	30	37	1 2.9	2	15
	2	3:55	4:25	30	27	2874	-	17./
	3	4:26	4:56	30	27	283/4	1 ³ /4 1 ³ /4	17-1
	4	 						
	5	*						
21	1	3:25	+	30	26	284	24	/3.3
		3:57		30	26	28	2	15-0
	3	4:28	4:58	30	26	28	2	15.0
	4			*				
	5			a design of the second				-
3	1		3:47	17	27/2	305	3	5-7
90-00004494 90-00-00-00-00-00-00-00-00-00-00-00-00-0		3:50	4:13	23	27/4	1	3.44	7-/
	3	4:14	4:36	22	27/2	302	3	7.3
	4	and the second sec	1			to transmission of the second se	2 2 2 2 2 2	
- vervenanske ver	5	40 millionaurige of	er stormandens					

are obtained at each percolation test hole. All data to be submitted for review.

2) Depth measurements to be made from top of hole. DO NOT REPORT INCREMENTS OF LESS THAN ONE INCH.

6 "	DLD PKG LOT SUBBRSE GANVEL CANVELLY LODM FILL VENY ROCKY	SUBBASE GRAVEL GRAVELY	OLO PILG SUBBASE GAMEL GAMELY SAMO LAMEE BOULDEAS	SANDY LOAM
6" - 12" - 18" - 24" - 30" - 36" - 42" - 48" - 54" -	SUBBASE GRAVEL GRAVELLY LOAM MILL VERY	SUBBASE GRAVEL GRAVELY	SUBBASE GANVEL GRAVELY GRAVELLY SAMO	SUBBINSIS CHNIEL SANDY LOAM FINE SIMOS
12"	VERY	GRAVELLY	GRAVELLY SAMO	SANDY LOAM
12"	Very		- Stores	FINE Smos
24" 30" 36" 42" 48" 54"				FINE Smos
24" 30" 36" 42" 48" 54"				Stres S
30" 36" 42" 48" 54"				Stres S
36" 42" 48" 54"				Stres S
36" 42" 48" 54"				
42"				
48"				
54"				LMGE STONES
54"				
60"		V		
		MOTTLING		
•				
66"	pock		FINE DENSE	
72"	pock		SAND	
78"		GROUNDWATER SEEPMEE	MOTTLING.	
		1	/	
84"		V		V
INDICATE LE INDICATE LE DEEPTESTS N	LVEL FOR WHICH WA	NUND WATER IS ENCO TER LEVEL RISES I - NAOEANAN P. Y - WCHO	FTER BEING ENCOUR	TTERED 78" STS 11/19/02
Soil Rate L	Jsed 16-20 Min	DESIGN /1" Drop: S.I). Usable Area Pro	ovided 9, 6005.F.
No. of Bedr (3-7 Absorption WITHIN P,	COOMS 6 Septic BORM UNITS Area Prov. by 32 RIMPMY PREA	Tank Capacity <u>/,</u> 5 <i>48thX18thF2</i> C.L.F.X 200 <i>1,500 G.M. 1404</i>	Gals. Masonr DW DIFFUSCA_S th trench. Other ING FARK, put	<u>ry X</u> Metal r <u>/8"-</u> 24" 20B P.C.WEMBER W/
			ture	P.C.W.R.M.BER. W/ D.O.S.E. 314 S.M.S.
		SEAL		
<u> 361</u>	FERSON VML	7. M.Y. 10535		
Hestchester	County Health De	epartment	1900000	<u></u>

4/98

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WESTCHESTER COUNTY DEPARTMENT OF HEALTH Bureau of Environmental Quality 110 So. Bedford Road Mt. Kisco, NY 10549

DESIGN DATA SHEET - SEPARATE SEWERAGE SYSTEM FILE NO.____ Owner HCG DRYWALL, INC. Address 10 DUNWOODIE ST., SCRASDME Located at (Street) westered STER ME Sec. 9 Block 9455 Lot (Indicate nearest cross St.) 10583

Municipality Pouro RIDGE Watershed 57RMF620

SOIL PERCOLATION TEST DATA REQUIRED TO BE SUBMITTED WITH APPLICATION 1 1

OLE #			CLOCK	TIME			121/02- EXPA	NSION AT
Hole Number		 Start		Elapse Time Min.	Depth t	to Water 1 Surface Stop	ERCOLATION Water Level Drop In Inches	Soil Rate Min/In Drop
4	1	10:36	11:24	28	76	79/4	3/2/	8.6
	2	11:25	11:52	27	26	29	3	9.0
	3	11:53	10:00	29	26	29	3	9.6
	4							
	5							
	1							
	2							
P	3							
an Maria	4					u de la constante de la consta		
and distantial and	5							*******
States According to	1	*****						
	2							
	3						·	konnen an
	4		·			4 Yang (1997)		000
	5	40	r 	۰ ۱				
	i	1		1	1			

2) Depth measurements to be made from top of hole. DO NOT REPORT INCREMENTS OF LESS THAN ONE INCH.

36 " OEEP

DEDTU			MITTED WITH APPLI ERED IN TEST HOLE	S EVAN
DEPTH	HOLE NO. 5	HOLE NO. 6	HOLE NO. 7	HOLE NO
G.L.	LIGHT WOODS	LIGHT WOODS	461H W0003	LIGHT WOODS
6 "	7000012	TOPSOIL	7015012	TOPSOIL
12"	5 ANOY LOAM 5000 51275	SANDY LORM SOMB SILTS	SAMU LAND	
18"		·	1	
24"	5ANDS/GRAVEL	3 ATOS/GRAVEL		5 Arros/ GRAVELS
30"		Ţ.	HEDIUM SANDS GAAVELLY	
36"		GANVELLY		
42"	VERY POLKY			
48"	GRAVELLY			
54"				
60"			VERY	
66"				
72"				
78"				
84"				V
	NDWATER ENCOUNTERED LEVEL AT WHICH GROU LEVEL FOR WHICH WA	UND WATER IS ENCOU	ETED DETNO DUCCT	ERED / /
DEEPTESTS	w/ ED OFLINES	- WICHAMIS P.E.		s <u>11/19/02</u>
DEEPTESTS	$\frac{\omega}{\varepsilon} = \frac{\delta \varepsilon}{\delta \varepsilon} \frac{\delta \varepsilon}$	- WRUEAAAAN, D-E. - WGNO DESIGN /1" Drop: S.D.	. Usable Area Prov	ided 9, 6.00 5.F.
Soil Rate	$\frac{U}{E0} \frac{\partial E^{2}}{\partial E^{2}} \frac{\partial E^{2}}{\partial E^{2}}$ $\frac{U \text{ Sed}}{\partial E^{2}} \frac{\partial E^{2}}{\partial E^{2}} Min_{f}$ $\frac{d \text{ rooms } 6}{\partial E^{2}} \frac{\text{ Septic}}{\partial E^{2}}$ $\frac{\partial E^{2}}{\partial E^{2}} \frac{\partial E^{2}$	- WRUERAAAN, D.E. - WGAO DESIGN /1" Drop: S.D. Tank Capacity /, Sc 48"×18" 500	. Usable Area Prov ²⁰ Gals. Masonry w <i>DIFFUSCA_S</i>	ided 9,6005.F. Metal
Soil Rate	$\frac{\omega}{\varepsilon} = \frac{\delta \varepsilon}{\delta \varepsilon} \frac{\delta \varepsilon}$	- WRUERAAAN, D.E. - WGAO DESIGN /1" Drop: S.D. Tank Capacity /, Sc 48"×18" 500	. Usable Area Prov ²⁰ Gals. Masonry w <i>DIFFUSCA_S</i>	ided 9,6005.F. Metal
NDICALE DEEPTESTS Soil Rate No. of Be (3-2 Absorption WITHIN Name <u>Span</u> Address <u>37</u>	$\frac{w/EO}{EO} \frac{DELAND}{DELAND} \frac{w}{EO} \frac{DELAND}{DELSON} \frac{w}{EO} \frac{DELSON}{DUS}$ $\frac{w/EO}{EC} \frac{DELSON}{DUS} \frac{DELSON}{DUS} \frac{DELSON}{DUS}$	- WREERFINGS DE - WCHO DESIGN /1" Drop: S.D. Tank Capacity / Sc 48"×18" FLOU 2 L.F. X24 - WOLDI 2 MIN / Signat SCX 7 SEAL	. Usable Area Prov ²⁰ Gals. Masonry w <i>DIFFUSCA_S</i>	ided 9,6005.F. Metal
Soil Rate No. of Be (3-2 Absorption WITHIN Name <u>Spa</u>	$\frac{W}{E0} \frac{\partial E^{2}}{\partial E^{2}} \frac{\partial E^{2}}{\partial E^$	- WREERFINGS DE - WCHO DESIGN /1" Drop: S.D. Tank Capacity / Sc 48"×18" FLOU 2 L.F. X24 - WOLDI 2 MIN / Signat SCX 7 SEAL	. Usable Area Prov ²⁰ Gals. Masonry w <i>DIFFUSCA_S</i>	ided 9,6005.F. Metal
NDICALE DEEPTESTS Soil Rate No. of Be (3-2 Absorption WITHIN Name Address 37	$\frac{w/EO}{EO} \frac{DELAND}{DELAND} \frac{w}{EO} \frac{DELAND}{DELSON} \frac{w}{EO} \frac{DELSON}{DUS}$ $\frac{w/EO}{EC} \frac{DELSON}{DUS} \frac{DELSON}{DUS} \frac{DELSON}{DUS}$	- WREERFINGS DE - WCHO DESIGN /1" Drop: S.D. Tank Capacity / Sc 48"×18" FLOU 2 L.F. X24 - WOLDI 2 M.M. J. Signat SCX 7 SEAL - J. J. S.J.	. Usable Area Prov ²⁰ Gals. Masonry w <i>DIFFUSCA_S</i>	ided 9,6005.F. Metal

4/98

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9456-55 35 WESTCHESTER AVE

Private Water Supply TOWN OF BUNG RIDGE Separate Sewerage System_ 73 CERTIFICATE OF CONSTRUCTION COMPLIANCE WCDH File N Located at WESTCHESTER AVE Owner EMIL DELENSER 9 9950 parate Severage System built by SAF SEPAC SYSTEMS INC. 750 Consisti 1 HP POMP IN POMPPIT. ALARM IN BUILDING Other reg Water Supply rivate Supply Drilled By BORISCHUR amEURA 600 GPD DENTIAL ilding cert that the system(s) as listed serving the above prorules and regulations, plans filed occupying premises served by the abo ary to secure the correction of any unsanitary conditions resulting from the separate sewerage st ry sewer becomes available and the approval of the private water supply shall hen a public water supr cation or change when, in the judgment of the Commissioner of Health, such licensed Professional Engineer or Registered Architect. 1974 16 Westchester County Department of Health

WELL COMPLETION REPORT

is report is to be completed by well driller and submitted to Health Department, together with aboratory report of analysis of water sample indicating water is of satisfactory bacterial ality, before certificate of construction compliance is issued.

	Well construction *RULES & REGULATION	n to be in accor ONS RELATING TO	dance with Bul INDIVIDUAL WAT	letin SD-62 TR SUPPLIES*	Q. T
XATION: MUNICIPAL		un (12)	TION 9	BLOCK 9456	LOT 5
ILL OWNER: MAE	Emil Osland	()	Contract Conversion Contraction Conversion Conversion		
Name	me yarmjer	Street Addres	R HUE YOU	ind Kidge NY City and Town	allen minister en verste første som ander at det en en som sakt minister for at de statet for en som som som s
IL DRILLER: R	oris Church		<i>~</i> 5		6
Name	The Ludde	20 Corbo Peri Street Addres	- Stamforg	City and Tow	
				CITY AND ION	Δ
CASING DETAILS	YIELD T	RST I	WATER LEVEL	I CODFEN	DETAILS
nath.	* Bailed		asure from lan	d surfâce)	
33	Feet or	6 Hours Sta	tion 5	: Foet [:] Make:	
	17		n Bailed	roet Make;	'Slot
ameter: 6 In	nches Mield:	15 G.P.M. for		Feet' Length	Ft_'Size
nd: Keary Duty He	el i	8 \$		Diameter	In.'
TAL DEPTH OF WELL	290 PEE		9999		
	annound for the second se	L			
		WELL LO	G		
	cemented, soft,	medium. coarse)	, granite, etc	. Include size of	gravel (diamet
1 Pt. to 15 Pt.		ller		Naamandaattikeen manekinnä taisin aatta attaksi Näädinen muyentöön piraatsee	assan tang baga mang barang barang bar ^a n Marida at Pandar da Sananda at Kasa
5 Pt. to 290 Pt.	gronite			n fan Sen en de Sen an en en de Besterne en fan en fan de Sen en de Sen	Brochoolselongsbandprocestington-ongoodsbandsbowgdandfodradongponoury
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Ft.to Ft.			nan an	nan waa maa maa ka ahaa ka ahaa ka ahaa ka ahaa aha	nn fan transfelining fan en til att for fan en f
Ft.to Ft.		annen menstaan gebraak en server en geboert kenne het geboert de kenne kenne geboert en de kenne geboert geboer			nazieron en en anter moner australitaria anter a fan anter
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te Well Was Comple					

	u 5			
	WELL PIT ANI) PUMP EQUIPMENT D	ETAILS	
Finished Well: Check	Pit with 4	-inch Gravity Dra	in to Grade	4 ²²
	Pit with 4	-inch Gravity Dra	in to Basement	
	Pitless Ad	lapter - Casing Mi	n. 12 inches above	grade
	Other: Des	scribe		
Pump: Make Belke	Ley Type va	ubreisible	Capacity 2H.P.	G.P.M. 10
Storage Tank: Type	gal.	Capacity	<u>62</u> .Ga	1.(42 Gal. Min.)
	DIAGRAM SHOWING	LOCATION OF WELL		
	sewage disposa Also indicate direction with	tion of house, wel al system with dis direction of slop a distances to all sposal systems wit	tances. es. and m wells i O	

I certify that the individual water supply indicated above was installed as per the rules and regulations of Bulletin SD.62 of the Westchester County Department of Health.

NEW

COUNTY OF WESTCHESTER DEPARTMENT OF	HEALTH - Divisio	n of Envir	onmental Sanitation				
DESIGN DATA SHEET - SEPARATE SEWAGE		FILE NO					
Over EMIL DOLENSEK	Address TRINIT	Y PASS	POUND RIDGE N.Y.				
Located At (Street) WESTCHEST	rer Ave	Sec. 9	Block9456Lot P/0 5				
(Indicate hearest cross street)							
Municipality POUND RIDGE	Watershed	TAMEON	Res.				

j.

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SOIL PERCOLATION TEST DATA REQUIRED TO BE SUBMITTED WITH APPLICATION

Start 7:50	Stop	Elapse Time Min.	PERCOLA Depth to From Grou Start Inches	Water ind Surface	Water Level in Inches	Soil Rate
	10:10			4.P	Drop in Inches	Min/in.drop
0111		20	4'-0"	4'-3'12"	31/2"	5.7
	10:32	21	4'-0"	4'-33/8"	33/8	6.2
			<.		Diministration of the second	Na Marina Santa
100	10/12	12	4'-0"	4-314	3 1/4	3.7
13	10:26	13	4'-0"	4'-31/8	348	4.1
			nin kan kan bili kan			nan de la constant d
103	10:22	19	4'-0"	4'-3"	3	6.3
123	10:43	20	4'-0"	4'-3"	3	6.6
				140 m		
				an an air an		
1949-993 1940-973-974						

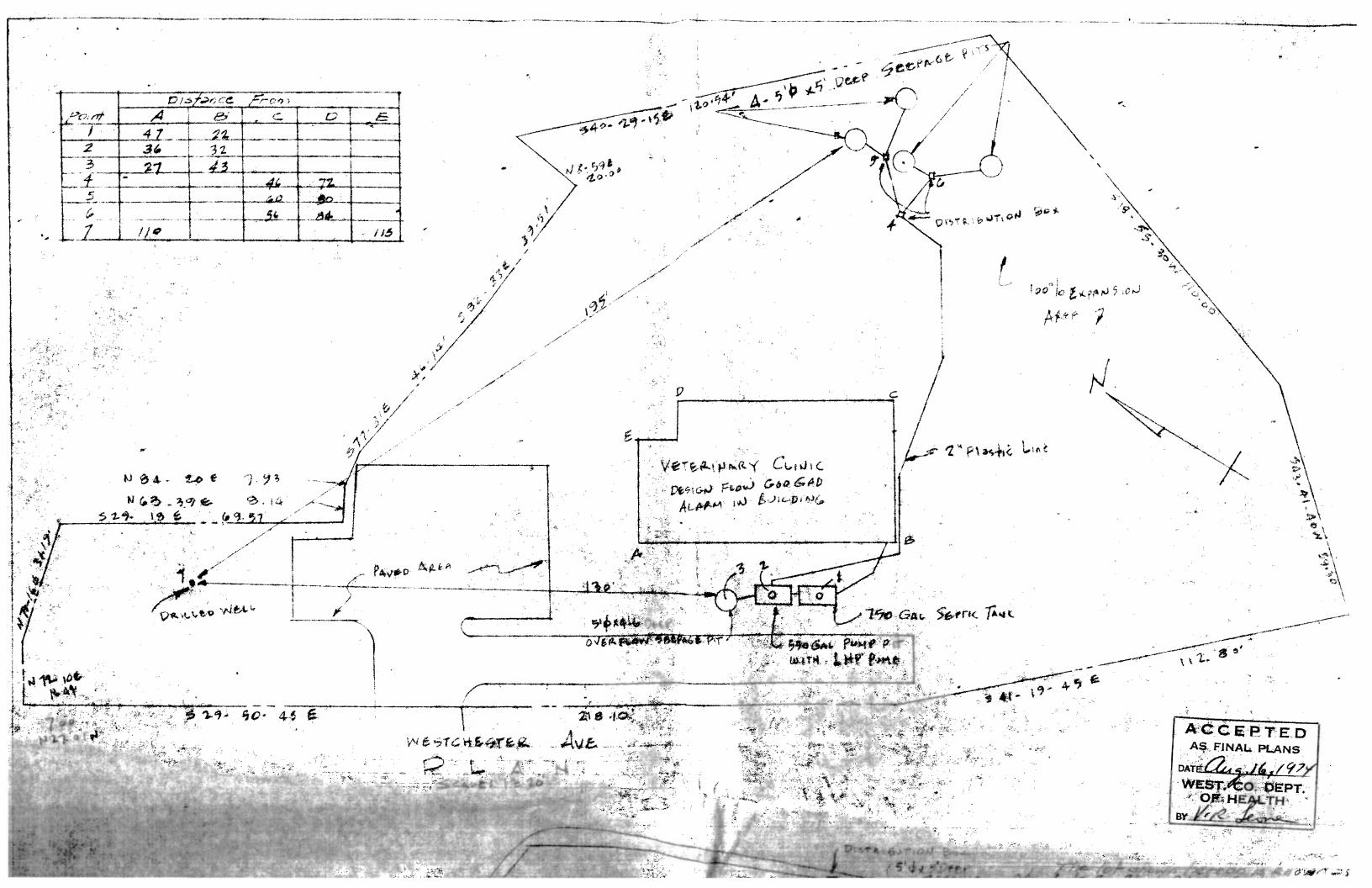
 Tests to be repeated at same depth until approximately equal soil rates are obtained at each percolation test hole. All data to be submitted for review.
 Depth measurements to be made from top of hole .

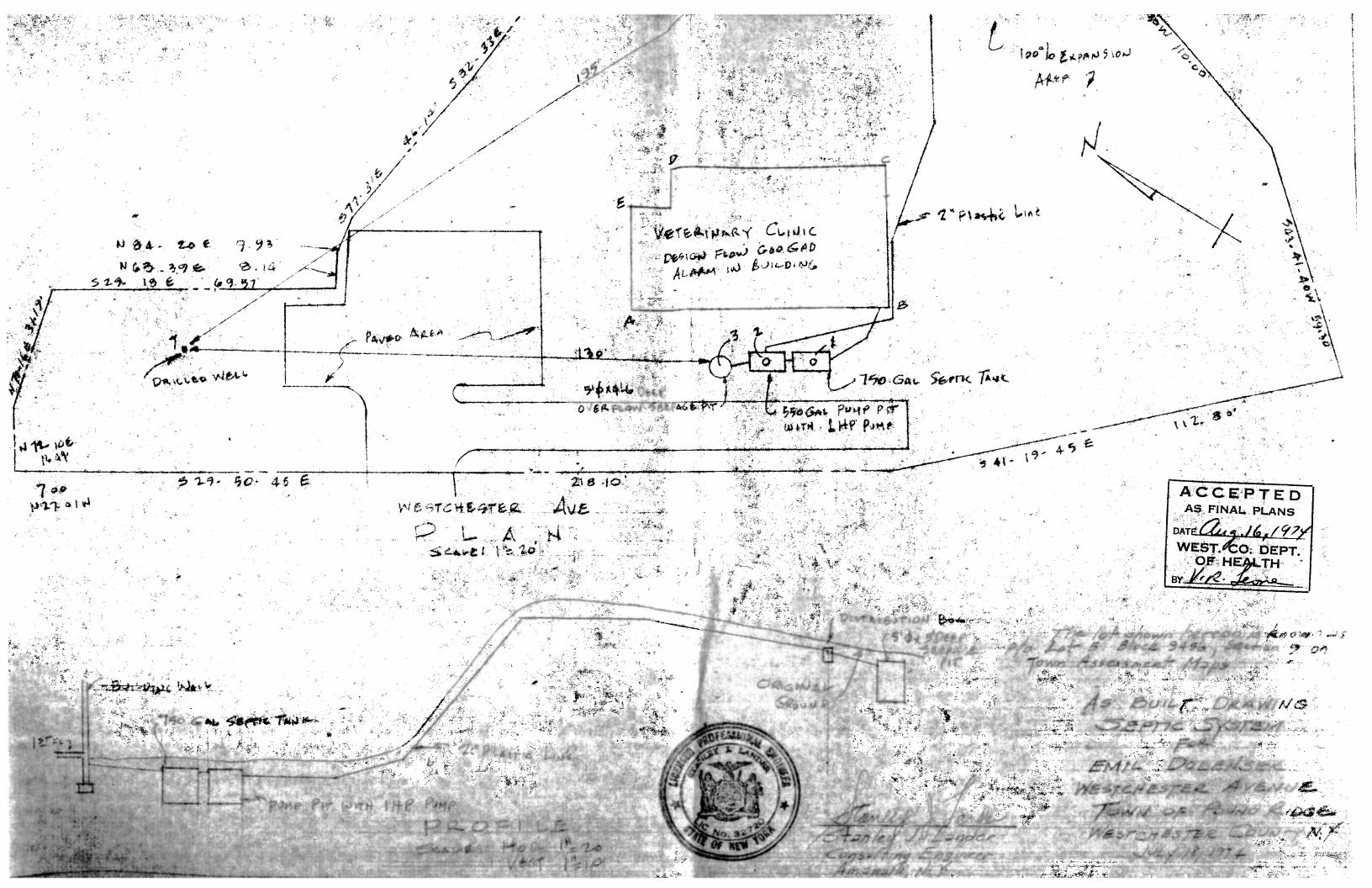
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	HOLE NO. K	HOLE NO. K2 HOLE	NO. P3 HOLE NO.	DEEP HOLE
G.L.	OPSOIL -	Topsoil	Topsoil	TOPSOIL
6¤	ŧ 8	£ 1	n se	£ ;
8-100 Autom	NO CLAY MIX	SAND CLAY MIX	SAND GLAY MIX	SAND CLAY MIX
	N NI	PADD CLAY ITTX	CARD CLAY PILL	HI II
18"	en an		;/	g g
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30" _				
36" <u>) A</u>	ND ZIME JON	E JAND SOME JTONE	- AND JOME STONE	JANO DOME STR
42" _	<i>ŧ</i>]	e .		
48 n	<i>tt</i>			
54" _	Ë ĝ		<i>i i</i>	Êţ
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8411		50 (453-48)-694-694-694-694-644-64-64-64-64-64-64-64-64-64-64-64-6	aunduren felden auf die Samater von Kalter der Verlagen der Kalter der Kalter der Kalter der Kalter der Kalter	and the second
	TE LEVEL TO W	HICH GROUND WATER : HICH WATER LEVEL R 1. LANDER	ISES AFTER BEING H DATE	NO WATER ENCOUNTERED 5-3-73
TESTS	and the design of the second		CALL CYT PART	
TESTS Soil I No. of Absor	FLOW Good Bedrooms otion Area Pro	Min/l" Drop:	acity 750 Ga	a Provided 5000 · ls. Masonry √ Meta width trench. Oth
Soil I No. of Absor	FLOW Good Bedrooms ption Area Pro $5^{\circ}q \times 5^{\circ}$	Min/l" Drop: Septic Tank Cap vided By L.F DEEP SEEPAGE	S.D. Usable Area acity 750 Gai .x21, H 36H	ls. Masonry V Meta
Soil I No. of Absory Absory Name	FLOW Good Bedrooms ption Area Pro $5' \not q \times 5'$ STANLE	Min/1" Drop: Septic Tank Cap vided By L.F DEEP SEEPAGE	S.D. Usable Area acity 750 Gai .x214" 36" 	ls. Masonry V Meta
Soil I No. of Absor	FLOW Good F Bedrooms ption Area Pro $5' \not q \times 5'$ STANLE ss BC AMAWALK	Min/l" Drop: Septic Tank Cap vided By L.F DEEP SEEPAGE	S.D. Usable Area acity 750 Gai .x21, H 36 H	ls. Masonry V Meta
TESTS Soil I No. of Absor <u>4</u> Name Addre	FLOW Good P Bedrooms ption Area Pro STANLE STANLE SS: BC AMAWALK 21	Min/1" Drop: Septic Tank Cap vided By L.F DEEP SEEPAGE (I. LANDER) X 267 (N. Y. 10501	S.D. Usable Area acity 750 Gai .x24" 36" 	Is. Masonry Meta width trench. Oth Karley J. Jen
TESTS Soil I No of Absory <u>4</u> Name Addre	FLOW Good Fedrooms ption Area Pro $5' \not + 5'$ STANLE ss BC AMAWALK 22 hester County	Min/l" Drop: Septic Tank Cap vided By L.F DEEP SEEPAGE (I. LANDER) X 267 X, N. Y. 10501 45-2645	S.D. Usable Area acity 750 Gai .x24 36" 	Is. Masonry Meta width trench. Oth Karley J. Jen





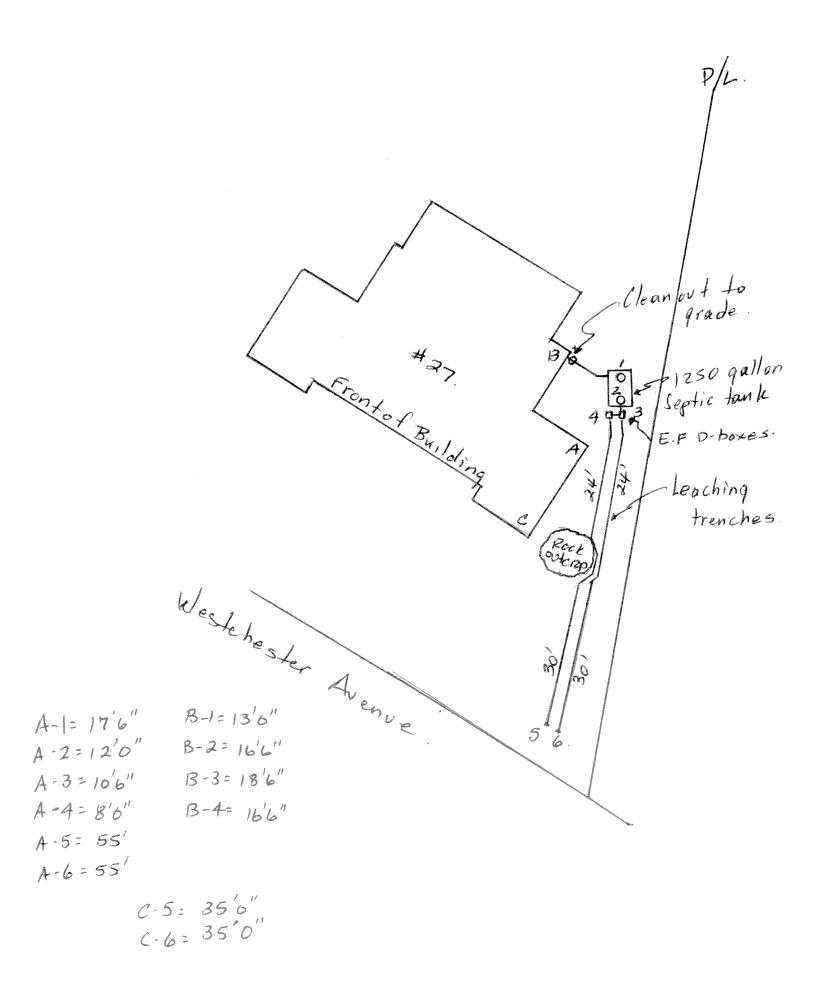
9456-6 27 WESTCHESTER AVE



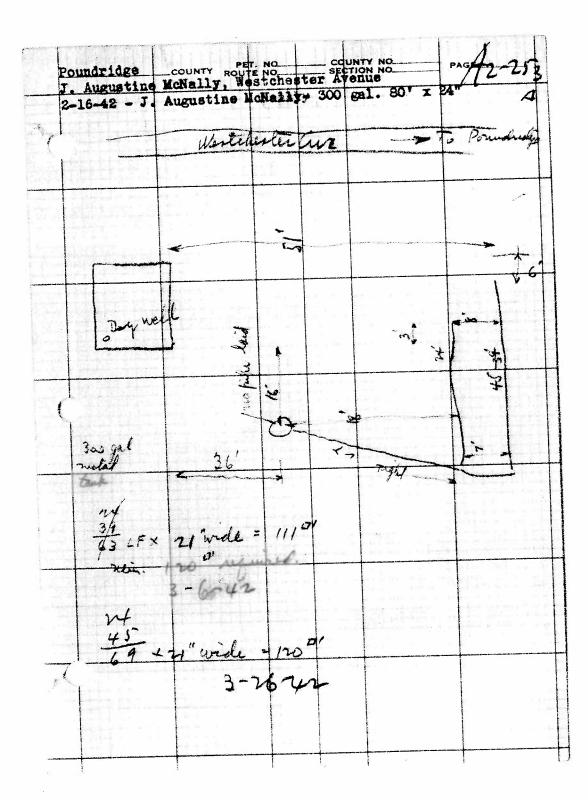
ONSITE WASTEWATER TREATMENT SYSTEM (OWTS) REPAIR AND REMEDIATION DATA FORM

Municipality:_		SALES MILLION DATA FOR	VI	
Property Mailir	ng Address (No. & Street): 27 W.	estingety the		
- IOWN/ Village:	Toldad Ridas		anna an	
				Richal + Maris
Owner Mailing	Address (No. & Street) (if different):	CD Stup 10050	717 2014 0 000	- Cher Hour
Town/ Village:	New York	State: 111	TOT S FIVE 24	"Floor
Property Use:	[] Single Family [] Multi-Family [] Inc	lustrial & Commercial	Zip: 10017	
	M Other - Describe: Apt Attaci	neel Apt/Hillia	len	
OWTS Reme	disting T			
		WCDH F	ile #:	
wastes or offens	all mean installation, replacement, or exp e, or impending failure, resulting in, or the sive material on to the surface of the grou all not include repairs, as defined above,	at may result in, the discharge of si	ent system components to corr wage or domestic wastes or tr tercourse or water body.	ect ade
	1	OR		
OWTS Repai	r 🗹 Complete the following in	formation.		
<i>Repair</i> shall me treatment system	an the repair, maintenance, and replacem n components.	ent in kind and in situ; of broken, d	amaged, or worn onsite wastev	vater
Number of Bedr	ooms / Number of Bathroom	as: Z Water Supp		
	Please note below only compo	nents that have been to a set of the set of	ly Type: Public L Well 🛛	١
Repaired R	eplaced	ients that have been repaired	or replaced.	N
	House Sewer or other Solid Pi	mal at		
	Sentic Tank#1 Siza(gallana)	157.5	AW BUILDING AND LOCATION	
	Septic Tank#2: Size (gallons):		WORK PERFORMED ON BACK THIS FORM	
	Junction/Distribution Box(es)		······	
	Septic Tank#1 Size(gallons): Septic Tank#2: Size (gallons): Junction/Distribution Box(es) Sewage Pump(s) or other Dosi Absorption Trench Length	28' ft. X Trench Width $4'$		
	Seepage Pit(s)	20 n. X Hench width T		
	Galley(s)			
Concession Concession	Gravelless Trench(es)			
	Image: Constraint of the system Image: Constraint of the system <td></td> <td></td> <td></td>			
	Other System Component(s) -	ystem Describe:		
U	Entire System Replaced			
Contractor's Nan	1e (print): Davis Strand	****	4 声 4 笔 4 笔 4 卷 2 含 8 含 8 名 4 8 4 8 4 4 4 4 4 4 4 4 4 4 4 4 4 4	
	aturé: Paul SkiADA	Date Repair/Remedi	ation Completed: $3/9/1$	2,
Connactor s Sign	lature: Taul Skuch	License No.: 36	3.	
Upon completion				
	Westchester Count	y Department of Health- BEQ		
	Hið North E Mr. k	Bedford Road, Rm# 100 Kisco, NY 10549		
	Attn: Pan	ricia Tomello-Adams	-stite	

Repair File #:<u>REP_1012 - 8</u> (WCDH Staff only)



Location:	Westchester Avenue	e: 2-16-42 3/7/14
Section:	Block:	Lot:
<u>Owner:</u> j	, Augustine Mc Nally	
Builder:	Samo	
House:	1 bedroom 1 bathgoom	
Soil test m	ade: no	Rate:
Tank capaci	ty: 300 gal.	Material: Masonry
Absorption:	80' x 24"	
Approval is	and 2 and 1	Sketch-Book 2-2



COUNTY BOARD OF HEALTH

EDWIN G. RAMSDELL, M.D., PRESIDENT NELSON A. ROCKEFELLER, VICE-PRES. CHARLES C. SWEET, M.D. MISS RUTH TAYLOR MISS JANE H. TODD FREDERICK E. VAUGHAN, M.D. RALPH A. McCLELLAND J. RUSSELL FOSHAY, M.D.

County of Westchester

DEPARTMENT OF HEALTH County Office Building White Plains, N. Y. GEORGE H. RAMSEY, M.D. COMMISSIONER

> W. A. HOLLA, M.D. FIRST DEPUTY

E. H. MARSH, M.D. A. D. LANGMUIR, M.D. DEPUTIES

March 27, 1942

Jewes Pondailje

Mr. J. Augustine MeMally Box 244 New Canaan, Connecticut

FINAL APPROVAL OF SEWAGE DISPOSAL SYSTEM

Dear Sir:

You are hereby notified that the sewage disposal system consisting of

a 300 gallon masonry septic tank and 69 linear feet of 21 inches wide absorption

tronch

to serve the bungalow of J. Augustine McMally, Vestchester Avenue, Town of

Poundridge, New York (maximum eccupancy 4 persons)

has been completed in general accordance with the requirements of this department and the permit issued **February 16, 1942**.

Very truly yours,

R. M. McLaughlin Director Division of Sanitation

HeleI c/c Stamford Water Company

THE OWNER OR HIS AGENT MUST RECEIVE THIS NOTICE OF APPROVAL OR A COPY THEREOF.

42-253A

WESTCHESTER COUNTY DEPARTMENT OF HEALTH

GEORGE H. RAMSEY, M. D., Commissioner

White Plains, N. Y.

PERMIT TO PROVIDE A SEWAGE DISPOSAL SYSTEM

Application having been duly made to the County Commissioner of Health as required by

Article II of the Sanitary Code of the Westchester County Health District, permission is hereby

given to J. Augustine Edually, Box 244, New Cansan, Connecticut

for the construction or provision of a sewage disposal system consisting of a 500 gallon masonry septic tank and 60 linear feet of 26 inches wide absorption trench

SUBJECT TO SOIL TEST

to serve the bungalow of J. Augustine McMally, Westchester Avenue, Town of Poundridge,

New York (maximum occupancy 4 persons)

subject to the following conditions: NOTE: Well should be 100' distant minimum from septie

- 1. That this department shall receive due notification and be afforded an opportunity to inspect the system before any portion is backfilled or covered.
- II. That this system shall not be used until the written final approval thereof shall have been obtained from the Department of Health.
- III. That such sewage disposal system shall be constructed in complete conformity with the application data and plans as approved or with approved amendments thereto. Any changes in this system must be approved.
- IV. That such system shall receive only the sewage or wastes from the structures or premises covered by this permit.
- V. That such system shall be so maintained and operated as not to expose sewage or sludge, or create a condition of nuisance.
- VI. That this permit shall not be construed to invalidate any rule or regulation enforceable by any local authority having jurisdiction.
- VII. That all duly enacted rules and regulations for the protection of water supplies shall be complied with.
- VIII. That a connection to the public sewer shall be made as soon as such is available.
- IX. That whenever it is determined by the Commissioner of Health that additional or more adequate sewage disposal facilities are necessary, such facilities shall be provided, plans for which shall first be submitted to and receive the approval of the Department of Health.
- X. That whenever the sludge and scum shall so accumulate in any settling tank as to occupy together at any point more than one-fourth of the distance between the bottom and the flow line, they shall be removed.
- XI. That whenever sludge or scum is removed from any settling tank or any part of the system, it shall be done in such a manner as to cause no nuisance and the material disposed of by burial in some remote place at least 250 feet from any house, road, well, spring, stream or other body of water, and covered with not less than 6 inches of earth in such a manner that it will not flow or be washed by rain or melted snow or other means over the surface of the ground or into any well, stream, spring or other body of water.
- XII. That this permit shall be revocable at any time or subject to modification or change when in the judgment of the Commissioner of Health such revocation, modification or change shall become necessary.

Peb. 16, 1942

HALO: I

Date: Copy to: Stamford Water Co.

COMMISSIONER

THE OWNER OR HIS AGENT MUST RECEIVE THIS PERMIT OR A COPY THEREOF.

VISION OF SANITATION R. M. McLaughlin, Director W. M. Scott J. D. Barrett H. M. Gray Sanitary Engineers	County of Westchester DEPARTMENT OF HEALTH GEORGE H. RAMSEY, M.D., COMMISSIONER County Office Building White Plains, N. Y.	File Pourdrudg Permit Inspected by
APPLICATIO	ON FOR SEWAGE DISPO	
To the Commissioner of Healt		Date
Application is hereby m	ade for a permit to construct a sewage syst	tem to serve
an a	Number, type and use of buildings to be served	te the second
concerning which the followin		
1. Owner	Mail Address	en that the later den
Note: Owner must receive permit an	d approval Check here if extra copies are requested	
2. Property location	(Street)	(Village, Town, City)
3. Tax Map Location: Sectio	n Block Lot Sub	
	acement. Proposed Future Building	
5. Lot area	No. of rooms Bedrooms	Bathrooms
	Special Fixtures Maximu	
6. Source of water supply		
Watershed on which sys	tem is located	16 5 f - 6
	rcourse Owner's wells	
	of persons 4 x 75 gals. =	
8. Settling treatment, Septic	tank: liquid capacity 7.0.0 7.0.6	material Lakoa as to The
inside dimensions: lengt Note: Liquid capacity of tank shall	h width effective be not less than volume of waste per day, with a minimum of	depth diam.
9. Soil: clay, loam, sand, bo good, fair, poor.	oulders, rock; surface: flat, sloping, steep; (Check terms that apply)	ground water and surface drainage:
Note: Except in clay soil, a rate of	1 gal. per sq. ft. of bottom area per day shall be used unless	a higher rate is established by soil test.
~	waste (No. 7) Absorption rate from table	sq. ft. bottom area.
11. Absorption treatment, T	renches: / / inches wide;	linear feet of distributing tile;
	ards, to depth of inches below	
ě ,	outside dimensions	
	ne material	
Absorption area: trench	leaching pits total	
		Title Original
	(By owner, b	builder, of officer of sewage disposal firm, or contractor)

Sketch required on reverse side or on attached sheet showing plan with general relation of dwelling and property boundaries, wells and streams to system and arrangement of absorption facilities, together with all other pertinent data, including details of grease trap, manholes, diversion gates, siphon, curtain drains, special structures and unusual features. Failure to secure permit before construction or final written approval of the system before using is a violation of the County Sanitary Code and is a misdemeanor.

Pound Ridge Waste Water Task Force

Appendix D: Flow Estimate Details

Based upon data from June 10, 2016

		300103	Corner Full Occupane			Building			Usage Rate		1/2	
Block	Lot	Zone	Property Address	Use	Acreage	Square Footage	Usage Number	Usage Measure	(gallons/day/ unit)	Wastewater Generation (gallons per day)	Allowable Flow (DOH)	
9454	36	R-2A	89 Westchester Ave	community facility	0.530	1,296	1,296	sq. ft.	0.10	130		
9454	5	PB-A	87 Westchesterchester Ave	retail	1.131	1,444	1,444	sq. ft.	0.24	347		
9454	6	PB-A	85 Westchester Ave	restaurant	0.415	4,122	50	seats	35.00	1,750		
9454	6	PB-A	85 Westchester Ave	office	0.473		1,360	sq. ft.	0.10	2		
9454	7	PB-A	83 Westchester Ave	retail	0.473	9,161	6,138	sq. ft.	0.24	737		
9454	7	PB-A	83, A, & B Westchester Ave	apartments			2	apts.	300.00	600		
9454	7	PB-A	83 C & D Westchester Ave	office			2,290	sq. ft.	0.10	57		
9454	8	PB-A	79 Westchester Ave	office	0.345	1,872	1,872	sq. ft.	0.10	187		
9454	9	PB-A	77 Westchester Ave	auto repair	0.342	4,864	2	bays	750.00	1,500		
9454	9	PB-A	77A Westchester Ave	apartments			1	apts.	300.00	300		
9454	35	PB-A	NA	Vacant	0.356	0	0	NA	NA	0		
9454	10	PB-A	73 Westchester Ave	office	0.670	5,600	5,600	sq. ft.	0.24	1,344		
9454	11	PB-A	71 Westchester Ave	resaurant	0.631	3,878	25	seats	35.00	875		
9454	11	PB-A	71 Westchester Ave	retail			3,878	sq. ft.	0.24	931		
9454	11	PB-A	71 Westchester Ave	office			3,878	sq. ft.	0.10	388		
9454	12	PB-A	69 Westchester Ave	resaurant	0.493	12,285	40	seats	35.00	1,400		
9454	12	PB-A	69 Westchester Ave	retail			12,285	sq. ft.	0.24	2,211		
9454	13	PB-A	67 Westchester Ave	apartments	0.147	3,368	2	apts.	300.00	600		
9454	13	PB-A	67 Westchester Ave	retail			1,684	sq. ft.	0.24	404		
9454	14	PB-A	4 Trinity Pass Rd.	office	0.181	1,012	1,012	sq. ft.	0.10	101		
9454	15	PB-A	65 Westchester Ave	retail	0.185	65	1,174	sq. ft.	0.24	282		
9454	15	PB-A	65A,B Westchester Ave	apartments	0.185		2	apts.	300.00	600		
9320	56	PB-A	Westchester Ave	parking w/2 shed	5.084	0		NA	NA	0		
9320	58	PB-A	80 Westchester Ave	community facility		7,076	7,076	sq. ft.	0.10	708		
9320	59	PB-A	78 Westchester Ave	retail	0.207	2,979	2,234	sq. ft.	0.24	536		
9320	59	PB-A	78 Westchester Ave	office			745	sq. ft.	0.10	74		
9320	60	PB-A	76 Westchester Ave	restaurant	0.207	8,910	60	seats	35.00	2,100		
9320	60	PB-A	76 Westchester Ave	office			1,782	sq. ft.	0.10	178		
9320	60	PB-A	76 Westchester Ave	apartments			4	apts.	300.00	1,200		
9320	61	PB-A	74 Westchester Ave	restaurant	0.207	7,970	50	seats	35.00	1,750		
9320	61	PB-A	74 Westchester Ave	retail			1,993	sq. ft.	0.24	478		
9320	61	PB-A	74 A, B, C, & D Westchester Ave	apartments			4	apts.	300.00	1,200		
9320	62	PB-A	72 Westchester Ave	retail	0.207	4,750	2,375	sq. ft.	0.24	570		
9320	62	PB-A	72 A & B Westchester Ave	apartments			2	apts.	300.00	600		
9320	63	PB-A	70 Westchester Ave	apartments	0.207	3,120	2	apts.	300.00	600		
9320	63	PB-A	70 Westchester Ave	retail			1,560	sq. ft.	0.24	374		
9320	64	PB-A	68 Westchester Ave	retail	0.418	6,923	3,462	sq. ft.	0.24	831		
9320	64	PB-A	68 A, B, C, & D Westchester Ave				4	apts.	300.00	1,200		
9320	65	PB-A	66 Westchester Ave	auto repair	0.642	2,130	2	bays	750.00	1,500		
				PB-A Subtotal	14.185	92,825	NA	NA	NA	28,645		

Appen	dix D	Scotts Co	rner Full Occupancy W	astewater G	eneration	Estimate (P	B-B and	PB-C) a	ind Total		2/2
									Usage Rate		
						Building Square	Usage	Usage	(gallons/day/	Wastewater Generation	Allowable
Block	Lot	Zone	Property Address	Use	Acreage	Footage	Number	Measure	unit)	(gallons per day)	Flow (DOH)
9455	20	PB-B	32 Westchester Ave	retail	0.656	3,800	4,441	sq. ft.	0.24	1,066	
9455	20	PB_B	32 Westchester Ave	apartment		641	1	apts.	300.00	300	
9455	21	PB-B	34 Westchester Ave	apartment	0.652	3,929	1	apts.	300.00	300	
9455	21	PB-B	34 Westchester Ave	retail			1,965	sq. ft.	0.24	471	
9455	27	PB-B, R-1A	38 Westchester Ave	office	0.717	1,760	1,760	sq. ft.	0.10	176	
9455	28	PB-B	40, 40A Westchester Ave	retail	0.495	3,870	3,870	sq. ft.	0.24	929	
9455	25	PB-B	54 Westchester Ave	restaurant	1.632	5,355	25	seats	35.00	875	
9455	25	PB-B	54 Westchester Ave	retail			1,607	sq. ft.	0.24	386	
9455	25	PB-B	54 Westchester Ave	apartment			1	apts.	300.00	300	
9455	24	PB-B	56, 60 Westchester Ave	apartment	1.698	10,388	5	apts.	300.00	1,500	
										4.070	
9455	24	PB-B	56, 60 Westchester Ave	retail	1.698		7,791	sq. ft.	0.24	1,870	
9455	4	PB-B	39 Westchester Ave	residential	2.196	0		NA	NA	0	
9456	1.9	PB-B	55, 57 Westchester Ave	retail	7.71	54,138	54,139	sq. ft.	0.24	12,993	
				PB-B Subtotal	17.45	83,881	NA	NA	NA	21,166	
9455	10	PB-C	22, 24 Westchester Ave	office	2.005	4,781	4,781	sq. ft.	0.10	478	
9455	13	PB-C	26 Westchester Ave	apartment	0.781	2,197	1	apts.	300.00	300	
9455	13	PB-C	26 Westchester Ave	office			1,648	sq. ft.	0.10	165	
9455	14	PB-C	30 Westchester Ave	residential	1.002	1,708	1,708	NA	NA	0	
9456	8	PB-C	21 Westchester Ave	residential	0.656	2,342	2,342	NA	NA	0	
9456	7	PB-C	23, 23 A, B Westchester Ave	retail	1.537	3,062	3,062	sq. ft.	0.24	735	
9456	6	PB-C	27 Westchester Ave	apartment	0.693	3,036	1	apts.	300.00	300	
9456	6	PB-C	27 Westchester Ave	retail			1,518	sq. ft.	0.24	364	
9456	5	PB-C	29 Westchester Ave	residential	3.195	11,018	12	apts.	300.00	3,600	
9456	55	PB-C	35 Westchester Ave	retail	0.764	3,425	3,425	sq. ft.	0.24	822	
				PB-C Subtotal	10.633	31,569	NA	NA	NA	6,764	
				PB Total	32.525	178,532				50,633	
Waterwat	er Gener	ation Rates fr	om New York City Department o	of Environmental P	rotection						
retail	0.24 gall	ons per day pe	er square foot								
			er square foot			1					
	<u> </u>	1 / 1	seat (about 60 square feet per s	eat)		1					
	-		r person/3 persons per apartme		data for affe	ted blocks)					
	-		or first bay and 500 gallons per o								
<u> </u>			455-27 are for PB-B section only	, ,	1	1					

POUND RIDGE WASTEWATER TASK FORCE

Appendix E: Photos of current conditions

Photos indicate wells that exist near Westchester Ave. and septic systems behind the buildings on Westchester Ave. under the parking lots and in one case extending into the woods, and high water table during an excavation.













